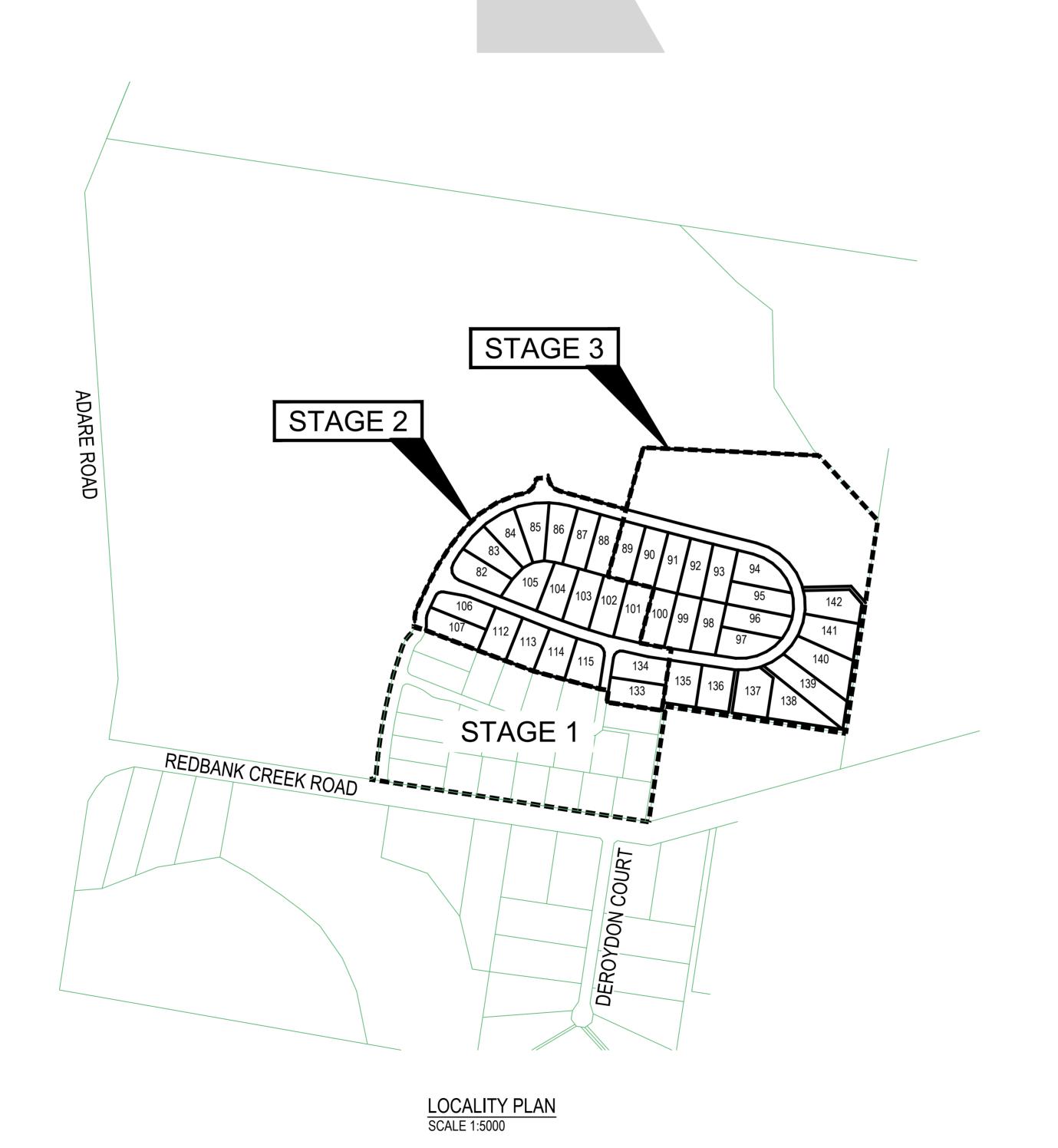
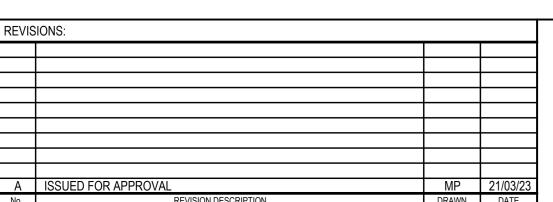
# PROPOSED SUBDIVISION

PARKLAKE, ADARE - STAGES 2 AND 3 174 ADARE ROAD, ADARE, QLD 4343



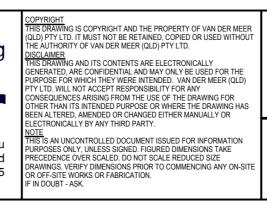
CIVIL ENGINEERING DRAWINGS				
SHEET NUMBE	SHEET TITLE	REVISION		
C2000	COVER SHEET	A		
C2001	STANDARD NOTES	А		
C2002	SITE PLAN - SHEET 1	А		
C2002	SITE PLAN - SHEET 2	А		
C2100	EROSION AND SEDIMENT CONTROL PLAN - SHEET 1	А		
C2101	EROSION AND SEDIMENT CONTROL PLAN - SHEET 2	А		
C2190	EROSION AND SEDIMENT CONTROL DETAILS	A		
C2191	EROSION AND SEDIMENT CONTROL NOTES	A		
C2200	BULK EARTHWORKS PLAN - SHEET 1	A		
C2201	BULK EARTHWORKS PLAN - SHEET 2	А		
C2290	BULK EARTHWORKS DETAILS	А		
C2300	PAVEMENT PLAN - SHEET 1	А		
C2301	PAVEMENT PLAN - SHEET 2	А		
C2400	ROADWORKS PLAN - SHEET 1	А		
C2401	ROADWORKS PLAN - SHEET 2	A		
C2430	SIGNS AND PAVEMENT MARKING PLAN - SHEET 1	A		
C2431	SIGNS AND PAVEMENT MARKING PLAN - SHEET 2	A		
C2450	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 1	A		
C2451	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 2	A		
C2452	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 3	A		
C2453	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 4	A		
C2454	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 5	A		
C2455	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 1	A		
C2456	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 2	A		
C2457	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 3	A		
C2458	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 4	A		
C2459	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 5	A		
C2460	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 6	A		
C2461	ROAD LONGITUDINAL AND CROSS SECTIONS - AXFORD WAY	A		
C2480	INTERSECTION DETAILS	A		
C2490	ROADWORKS DETAILS	A		
C2500	DRAINAGE CATCHMENT PLAN	A		
C2501	DRAINAGE PLAN - SHEET 1	A		
C2502	DRAINAGE PLAN - SHEET 2	A		
C2510	DRAINAGE BIO RETENTION BASIN DETAILS - SHEET 1	A		
C2510 C2511	DRAINAGE BIO RETENTION BASIN DETAILS - SHEET 2			
C2550		A A		
C2560	DRAINAGE LONGITUDINAL SECTIONS - SHEET 1  DRAINAGE CALCULATIONS	A		
C2600	WATER RETICULATION COVER SHEET	A		
C2600	WATER RETICULATION COVER SHEET  WATER RETICULATION PLAN - SHEET 1	A		
C2602	WATER RETICULATION PLAN - SHEET 2	A		
C2700	BUSHFIRE TRAIL PLAN	A		
C2700	BUSHFIRE TRAIL 1 LONGITUDINAL SECTION - SHEET 1	A		
C2701	BUSHFIRE TRAIL 1 LONGITUDINAL SECTION - SHEET 1	A		
C2702	BUSHFIRE TRAIL 1 LONGITUDINAL SECTION - SHEET 3	A		
C2703	BUSHFIRE TRAIL 1 CROSS SECTIONS - SHEET 1	A		
C2704 C2705	BUSHFIRE TRAIL 1 CROSS SECTIONS - SHEET 1	A		
C2706	BUSHFIRE TRAIL 1 CROSS SECTIONS - SHEET 3	A		
C2707	BUSHFIRE TRAIL 2 LONGITUDINAL AND CROSS SECTIONS	A		
02101	DOGNINE TO ME E EGNOTIODIWAE / MED ORGOOD OF OTTOMO	A		











OSED SUBDIVISION E ROAD, ADARE, QLD 4343	ORIGINAL ISSUE FOR APPROVAL				
AND 3	PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 1953 NER: 30532	
SHEET	DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1	
SHELL	JOB No. BR22216	_	C2000	REVISION	

#### **GENERAL NOTES**

- G1. ALL LEVELS SHALL BE OBTAINED FROM ESTABLISHED BMS OR SSM.
- G2. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK.
- G3. ALL WORKS ARE TO BE UNDERTAKEN IN ACCORDANCE WITH COUNCIL'S SPECIFICATIONS AND THE DIRECTIONS OF THE SUPERINTENDENT.
- G4. DIMENSIONS MUST NOT BE SCALED FROM DRAWINGS.
- G5. CONTRACTOR TO ENSURE THAT ALL ROADWORKS ARE SMOOTHLY TRANSITIONED TO EXISTING LEVELS FREE FROM ABRUPT CHANGES
- G6. THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR. FURTHER, THE LOCATION OF RECOVERY MARKS SHOULD BE VERIFIED AND CONFIRMED BY THE CONTRACTOR AND ANY DISCREPANCIES SHOULD BE CLARIFIED IN WRITING WITH THE SUPERINTENDENT PRIOR TO THE COMMENCEMENT
- G7. AT COMPLETION OF WORKS ALL ADJOINING DISTURBED AREAS ARE TO BE REINSTATED TO THE "AS FOUND" CONDITION.
- G8. THE CONTRACTOR SHALL ENSURE ALL AREAS DRAIN WITH A MINIMUM FALL OF 1% (1:100) GRADE TO OUTLETS UNLESS INDICATED OTHERWISE. NO WORKS SHALL CAUSE PONDING OF STORMWATER ON UPSTREAM PROPERTIES OR CONCENTRATE RUNOFF ONTO DOWNSTREAM PROPERTIES.
- G9. THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED LANDSCAPE, ARCHITECTURAL, ELECTRICAL, RETICULATION, WATER AND SEWER DRAWINGS AND SPECIFICATIONS AND OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED.
- G10. THE CONTRACTOR SHALL ENSURE THAT ALL PAVEMENTS GRADE EVENLY BETWEEN NOMINATED RL'S ON PLAN AND NO POND OF WATER OCCURS.
- G11. ALL DIMENSIONS ARE IN METERS UNLESS STATED OTHERWISE. ALL LEVELS ARE EXPRESSED IN METERS.
- G12. DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A STABLE CONDITION AND ENSURING NO PART SHALL BE OVERSTRESSED UNDER CONSTRUCTION ACTIVITIES.
- G13. WORKMANSHIP AND MATERIALS ARE TO BE IN ACCORDANCE WITH THE RELEVANT CURRENT S.A.A. CODES INCLUDING ALL AMENDMENTS. AND THE LOCAL STATUTORY AUTHORITIES. EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- G14 THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE ENGINEER BUT IS NOT AN AUTHORIZATION FOR AN EXTRA. ANY EXTRAS INVOLVED MUST BE TAKEN UP WITH THE SUPERINTENDENT BEFORE THE WORK COMMENCES.
- G15 THE CONTRACTOR IS TO EMPLOY A QUALIFIED GEOTECHNICAL ENGINEER AS REQUIRED FOR ALL GEOTECHNICAL ASPECTS OF THE BUILDING WORKS. REFER TO FOUNDATION, GROUNDWORKS AND RETENTION/SHORING NOTES. REFER ALSO TO THE GEOTECHNICAL REPORT FOR THIS PROJECT.
- G16 ORIGINAL SURVEY WAS COMPLETED BY BPLANNED & SURVEYED PH.1300 275 266

#### SUBGRADE PREPARATION

- RW1. REMOVE ALL VEGETATION. TOPSOIL AND DELETERIOUS MATERIAL FROM AREA OF PROPOSED BUILDING PLATFORM AND PAVEMENTS.
- RW2. PROOF ROLL EXPOSED SUB GRADE TO ACHIEVE A MINIMUM COMPACTION OF 98% STANDARD MAXIMUM DRY DENSITY (SMDD), DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1.
- RW3. REMOVE ANY SOFT, HEAVING, WET OR UNSTABLE AREAS IDENTIFIED DURING PROOF ROLLING AND REPLACE USING SELECT IMPORTED FILL COMPACTED IN LAYERS NOT EXCEEDING 200mm MEASURED LOOSE TO ACHIEVE A MINIMUM 98% STANDARD MAXIMUM DRY DENSITY.
- RW4. NOTE THAT THE SITE IS UNDERLAIN BY EXISTING SERVICES AND COMPACTION UTILISING VIBRATION MAY NOT BE SUITABLE IN THE VICINITY OF UNDERGROUND SERVICES.
- RW5. ANY FILL REQUIRED TO RAISE LEVELS TO BULK EARTHWORKS TO WITHIN 50mm OF NOMINATED LEVELS IS TO BE APPROVED GRANULAR MATERIAL COMPACTED IN LAYERS NOT EXCEEDING 300mm MEASURED LOOSE TO 98% STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SOMC).
- RW6. THE CONTRACTOR IS TO PROVIDE CERTIFICATION TO THE EFFECT THAT EARTHWORKS COMPACTION TO 98% STANDARD MAXIMUM DRY DENSITY, (AS 1289 E1.1, E4.1) HAS BEEN ACHIEVED, UNLESS OTHERWISE AGREED IN WRITING BY SITE SUPERINTENDENT.
- RW7. THE CONTRACTOR IS TO PROVIDE TO THE SITE SUPERINTENDENT A SURVEY CONFIRMATION FROM A REGISTERED SURVEYOR, CONFIRMING BULK EARTHWORKS LEVELS AS WITHIN +/-50mm OF LEVELS NOMINATED.
- RW8. SUBGRADE REPLACEMENT MATERIAL IS TO CONSIST OF CLEAN, UNCONTAMINATED, WELL-GRADED MATERIAL WITH A MAXIMUM PARTICLE SIZE OF 75mm, WITH 80% LESS THAN 20mm, AND A SOAKED C.B.R. GREATER THAN 10% AND A PLASTICITY INDEX LESS THAN 12.
- RW9. BACK FILLING FOR SERVICE TRENCHES AND REMOVED SERVICES OR PITS OR FOUNDATIONS IS TO USE APPROVED WELL-GRADED GRANULAR MATERIAL WITH MINIMUM VOIDS, (EITHER SELECT INSITU OR IMPORTED FILL), COMPACTION AS SPECIFIED ABOVE.
- RW10. ALL EARTHWORKS TO BE UNDERTAKEN IN ACCORDANCE WITH AS3798-1996: GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS.

#### **GENERAL EARTHWORKS**

- E1. THE SITE OF THE WORKS SHALL BE PREPARED BY STRIPPING ALL EXISTING TOPSOIL, FILL AND VEGETATION.
- E2. COMPACT SUBGRADE TO 98% OF THE STANDARD MAXIMUM DRY DENSITY WHEN TESTED IN ACCORDANCE WITH AUSTRALIAN STANDARD AS 1289 TESTS E.1.1. OR E.1.2. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED TO DETECT ANY SOFT OR WET AREAS WHICH SHOULD BE LOCALLY EXCAVATED AND BACK FILLED WITH SELECTED MATERIAL. THE BACK FILLING MATERIAL SHALL BE IMPORTED GRANULAR FILL OF LOW PLASTICITY, PREFERABLY CRUSHED SANDSTONE, AND TO BE PLACED IN LAYERS NOT EXCEEDING 300mm LOOSE THICKNESS AND COMPACTED TO 98% OF STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT. SITE WORKS ARE TO BE BATTERED TO ADJACENT PROPERTY LEVELS.
- E3. NO STORMWATER IS TO POND ON ADJOINING PROPERTIES. THE SITE SHALL BE GRADED AND DRAINED SO THAT STORMWATER WILL BE DIRECTED AWAY FROM THE BUILDING PLATFORM. STORMWATER DRAINAGE SHALL BE PROVIDED AND MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION. ALL STORMWATER RUNOFF SHALL BE GRADED AWAY FROM THE DWELLING AND DISPOSED OF VIA SURFACE CATCHDRAINS AND STORMWATER COLLECTION PITS.
- E4. ENSURE ALL RETAINING WALLS ARE CONSTRUCTED WITH ADEQUATE SUBSOIL DRAINAGE.

#### GROUND WORKS AND EXCAVATION

- GW1. ALL GROUND WORKS AND EXCAVATION SHALL BE IN ACCORDANCE WITH GEOTECHNICAL REPORT: 93323.00.R.01.Rev01.
- GW2. SEPARATE AND REMOVE ALL TOPSOIL, NON SOIL MATERIAL, CONCRETE, VEGETATION, BRICKBATS, TIMBER, ROOT AFFECTED SOIL AND EXISTING FILL. STORE TOPSOIL IF REQUIRED.
- GW3. ALL EXCAVATIONS SHALL BE FINISHED CLEAN AND HORIZONTAL AND SHALL NOT UNDERMINE FOOTINGS. WALLS etc...
- GW4. PROOF ROLL WITH AN 8 TONNE ROLLER, REPLACE ANY SOFT MATERIAL WITH APPROVED FILL AND RE-COMPACT. GEOTECHNICAL ENGINEER TO
- GW5. THE FILL IS TO BE PLACED AND COMPACTED IN LAYERS OF MAXIMUM LOOSE THICKNESS 300mm.
- GW6. TOP LAYER OF PAVED AREAS TO BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY, GEOTECHNICAL ENGINEER TO VERIFY.
- GW7. ALL PERMANENT EMBANKMENTS TO BE COMPACTED IN 200 mm LAYERS AS PER NOTE GW6 AND AT A MAXIMUM SLOPE OF 1 VERTICAL TO 2.5 HORIZONTAL UNLESS NOTED OTHERWISE. SHOULD DRAINAGE BE REQUIRED THEN SUBMIT DETAILS TO THE ENGINEER.
- GW8. ALL GROUND WORKS SHALL BE TESTED BY AN APPROVED GEOTECHNICAL ENGINEER TO A LEVEL 1 STANDARD IN ACCORDANCE WITH AS 3798 - 1996.
- GW9. ALL EXCAVATIONS TO BE INSPECTED AT REGULAR INTERVALS BY A GEOTECHNICAL ENGINEER.
- GW10. REFER TO ARCHITECTURAL DRAWINGS TO CONFIRM SETOUT OF BUILDINGS, CARPARKS ETC.
- GW11. THE LEVELS SHOWN ARE ONLY RELEVANT TO THE PLAN UPON WHICH THEY ARE SHOWN.
- GW12. ALL CONTOURS AND LEVELS USED TO PRODUCE EARTHWORK DETAILS HAVE BEEN BASED ON SURVEYOR AND ARCHITECTS SURVEY INFORMATION
- GW13. ALL FINISHED FLOOR LEVELS ARE TO BE CONFIRMED BY ARCHITECT.
- GW14. ALL EXISTING SERVICES ARE TO BE CAPPED OFF PRIOR TO ANY WORKS.
- GW15. A PRE-CONSTRUCTION MEETING SHALL BE HELD BETWEEN THE CONTRACTOR, THE GEOTECHNICAL ENGINEER, AND THE EARTHWORKS CONTRACTOR TO UNDERSTAND POTENTIAL DIFFICULTIES AND TO ORGANISE TESTING PROCEDURES. THE CONTRACTOR SHALL CONFIRM TO THE ENGINEER THAT THE MEETING HAS BEEN HELD.

#### DRAINAGE NOTES

- D1. PIT LEVELS SHOWN ON STORMWATER DRAINAGE PLANS ARE FOR INFORMATION. EXACT PIT LEVELS TO BE ADJUSTED TO SUIT FALLS IN PAVEMENT/LANDSCAPED AREA.
- D2. PITS GREATER THAN 1.2m DEEP TO BE FITTED WITH STEP IRONS.
- D3. DRAINAGE PIPES SHALL BE BACKFILLED WITH COMPACTED CLEAN SHARP SAND TO 200 ABOVE PIPE OBVERT. ADDITIONAL BACKFILL UNDER ROADS SHALL CONSIST OF CLASS 2 F.C.R. MATERIAL COMPACTED IN 200mm LAYERS TO 98% SMDD. UNDER LANDSCAPED AREAS ADDITIONAL BACKFILL SHALL CONSIST OF GRANULAR MATERIAL COMPACTED IN 200mm LAYERS A 3m LENGTH OF 100 Ø SLOTTED AGRICULTURAL LINE SURROUNDED BY GEOTECH STOCKING SHALL BE PROVIDED ON THE UPSTREAM SIDE OF ALL
- D4. CONCRETE STORMWATER PIPES TO BE CLASS '3' UNDER ROADS AND CLASS '2' IN NON-TRAFFICED AREAS. ALL PIPES GREATER THAN 300Ø ARE TO BE RUBBER RING JOINTS U.N.O.
- D5. CONCRETE PITS GREATER THAN 1.0m DEEP TO BE REINFORCED WITH N12-200 EACH WAY CENTRED, MIN. 300 LAP, CONCRETE - F'c 25MPa
- D6. 150Ø, 225Ø AND 300Ø uPVC PIPES TO BE SEWER GRADE PIPE UNDER TRAFFICABLE PAVEMENT. MIN. 400 COVER UNDER NON-TRAFFICABLE **PAVEMENT**
- D7. PIT COVERS AND GRATED DRAINS IN TRAFFICABLE PAVEMENT TO BE AS 3996 CLASS D "HEAVY DUTY" AND IN NON-TRAFFICABLE AREAS TO BE AS 3996 CLASS C "LIGHT DUTY".

#### **UTILITY SERVICES**

- S1. CONDUITS TO BE PROVIDED FOR WATER AND ENERGY AUTHORITIES, TELSTRA AND OTHER SERVICES AS REQUIRED.
- S2. THE LOCATIONS OF UNDERGROUND SERVICES SHOWN ON THESE DRAWING'S HAVE BEEN PLOTTED FROM SURVEY AND AUTHORITY INFORMATION. THE SERVICE INFORMATION HAS BEEN PREPARED ONLY TO SHOW THE APPROXIMATE POSITIONS OF ANY KNOWN SERVICES AND MAY NOT BE AS CONSTRUCTED OR ACCURATE.
- VAN DER MEER CANNOT GUARANTEE THAT THE SERVICES INFORMATION SHOWN ON THESE DRAWINGS, ACCURATELY INDICATES THE PRESENCE OR ABSENCE OF SERVICES OR THEIR LOCATION AND WILL ACCEPT NO LIABILITY FOR INACCURACIES IN THE SERVICES INFORMATION SHOWN ARISING FROM ANY CAUSE WHATSOEVER.
- S4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.
- CONTRACTORS SHALL TAKE DUE CARE WHEN EXCAVATING ON SITE INCLUDING HAND EXCAVATION WHERE NECESSARY.
- CONTRACTORS ARE TO CONTACT THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF EXCAVATION OR FUTURE WORKS.
- CONTRACTORS ARE TO UNDERTAKE A SERVICES SEARCH PRIOR TO COMMENCEMENT OF WORKS ON SITE. SEARCH RESULTS ARE TO BE KEPT ON SITE AT ALL TIMES.

#### TELSTRA - DUTY OF CARE NOTE:

TELSTRA'S PLANS SHOW ONLY THE PRESENCE OF CABLES AND PLANT, THEY ONLY SHOW THEIR POSITION RELATIVE TO ROAD BOUNDARIES, PROPERTY FENCES ETC. AT THE TIME OF INSTALLATION AND TELSTRA DOES NOT WARRANT OR UPHOLD THAT SUCH PLANS ARE ACCURATE THEREAFTER DUE TO CHANGES THAT MAY OCCUR OVER TIME. DO NOT ASSUME DEPTH OR ALIGNMENT OF CABLES OR PLANT AS THESE VARY SIGNIFICANTLY.

THE CONTRACTOR HAS A DUTY OF CARE WHEN EXCAVATING NEAR TELSTRA CABLES AND PLANT. BEFORE USING MACHINE EXCAVATORS TELSTRA PLANT MUST FIRST BE PHYSICALLY EXPOSED BY SOFT DIG POT HOLING TO IDENTIFY IT'S LOCATION. TELSTRA WILL SEEK COMPENSATION FOR DAMAGES CAUSED TO IT'S PROPERTY AND LOSSES CAUSED TO TELSTRA AND IT'S CUSTOMERS.

**ELECTRICAL AND GAS NETWORK:** A MINIMUM OF 30 DAYS PRIOR TO COMMENCEMENT OF EXCAVATION WORKS THE SUBCONTRACTOR MUST CONTACT DIAL BEFORE YOU DIG.

#### RETAINING WALL GENERAL

- GR1. BASE MATERIAL SHALL BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY (SMDD) WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SMOC) DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. MINIMUM ALLOWABLE BEARING PRESSURE OF 150 kPa. GEOTECHNICAL ENGINEER EMPLOYED BY CONTRACTOR TO INSPECT AND CONFIRM.
- GR2. DRAINAGE MATERIAL WITHIN AND IMMEDIATELY BEHIND THE WALL SHALL BE 12-20mm CLEAN AGGREGATE. DRAINAGE MATERIAL TO EXTEND A MINIMUM 300mm BEHIND WALL. COMPACT DRAINAGE MATERIAL.
  - ALTERNATIVELY, USE NO FINES CONCRETE, AS FOLLOWS:
- CONCRETE STRENGTH N15. 210kg/m3 PORTLAND CEMENT
- MAXIMUM AGGREGATE SIZE 20 mm.
- W/C RATIO 0.45 TO 0.55. DENSITY 1600 TO 2000 kg/m3
- GR3. INFILL SOIL SHALL BE CLASS 1 CONTROLLED FILL TO AS4678, OR AS SPECIFIED ON THE DRAWINGS, UNSUITABLE SOILS, SUCH AS HEAVY CLAYS OR ORGANIC SOILS WITH HIGH PLASTICITY, SHALL NOT BE USED IN THE REINFORCED SOIL MASS.
- GR4. SPREAD BACKFILL IN UNIFORM LIFTS OF 200mm UNCOMPACTED THICKNESS, COMPACT TO MINIMUM 95% OF SMDD. COMPACTION WITHIN 1.0 m BEHIND THE WALL SHALL BE ACCOMPLISHED BY USING A HAND-OPERATED PLATE COMPACTOR AND SHALL BEGIN BY RUNNING THE PLATE DIRECTLY ON THE BLOCK. THEN COMPACTING IN PARALLEL PATHS, PROGRESSIVELY AWAY FROM THE WALL FACE.
- GR5. WHERE ROADWAYS OR BUILDING STRUCTURES ARE LOCATED ABOVE THE REINFORCED ZONE, COMPACT TO 98% SMDD WITHIN 2% OF SOMC DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. COMPACTION TESTING SHALL BE TAKEN AT 1.2m BEHIND THE WALL.

#### **PAVEMENT**

- F1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.
- F2. PAVEMENT MATERIAL SHALL CONSIST OF APPROVED OR RIPPED SANDSTONE, NATURAL GRAVEL OR FINE CRUSH ROCK AS PER COUNCIL SPECIFICATION.
- PAVEMENT MATERIALS SHALL BE SPREAD IN LAYERS NOT EXCEEDING 150mm AND NOT LESS THEN 75mm COMPACTED THICKNESS. PAVEMENT MATERIALS SHALL BE SIZED AND OF A STANDARD OUTLINED IN AS1141.
- CRUSHED OR RIPPED SANDSTONE SHALL BE MINUS 75mm NOMINAL SIZE DERIVED FROM SOUND, CLEAN SANDSTONE FREE FROM OVERBURDEN, CLAY SEAMS, SHALE AND OTHER DELETERIOUS MATERIAL.
- PAVEMENT MATERIALS SHALL BE COMPACTED BY SUITABLE MEANS TO SATISFY THE FOLLOWING MINIMUM SPECIFICATIONS (AS PER AS1289.52)

**DESCRIPTION** MODIFIED DENSITY RATIO SUB-BASE 98% MDD BASE COURSE 98% MDD ASPHALTIC CONCRETE 97% MDD

AND SUBJECT TO COUNCIL'S CONSTRUCTION SPECIFICATION.

- TESTING FOR EACH LAYER SHALL BE UNDERTAKEN BY A N.A.T.A. REGISTERED LABORATORY IN ACCORDANCE WITH AS1289, AT NOT MORE THAN 50m INTERVALS AND A MINIMUM OF TWO PER LAYER. FURTHER FREQUENCY OF TESTING SHALL BE NO LESS THAN THAT REQUIRED BY AS3978-1996.
- A MINIMUM GRAVEL LAYER 150-200mm ABOVE THE GEOGRID SHALL BE ADHERED TO AS PER SUPPLIER (GLOBAL SYNTHETICS) REQUIREMENTS AND SPECIFICATIONS.

#### AS CONSTRUCTED

#### PRIVATE WORKS (SITE CIVIL WORKS)

THE CONTRACTOR SHALL PROVIDE THE FOLLOWING AS-CONSTRUCTED DOCUMENTATION TO VAN DER

MEER PRIOR TO PRACTICAL COMPLETION OF CIVIL WORKS: PW.01 A COMPLETE SURVEY OF COMPLETED SURFACE INCLUDING SURFACE LEVELS OF ALL

- STRUCTURES INCLUDING BUT NOT LIMITED TO:
- BIO-RETENTION AREAS, INCLUDING BASE AND TOP OF FILTER TRENCH MATERIAL AND AREA INSTALLED, IF APPLICABLE.
- WATER SERVICE CONNECTION INCLUDING FITTINGS AND METERS.
- SEWERAGE PROPERTY CONNECTIONS INCLUDING MANHOLES.
- SEWER PUMP STATIONS, IF APPLICABLE

STORMWATER MANHOLES AND PITS.

- PW.02 COPIES OF NATA TEST CERTIFICATE RESULTS IN RESPECT OF:
- THE COMPACTION OF FILL INCLUDING COMPACTION OF TRENCH BACKFILL. EARTHWORKS CERTIFICATION FROM GEOTECHNICAL RPEQ INCLUDING LEVEL 1 CERTIFICATION WHERE REQUIRED.
- THE SUB-GRADE CBR.
- THE SUB-GRADE COMPACTION.
- THE LOWER SUBBASE (CBR 15) MATERIAL QUALITY.
- THE LOWER SUBBASE (CBR 15) COMPACTION.
- THE SUB-BASE COURSE (CBR 45) MATERIAL QUALITY.
- THE SUB-BASE COURSE (CBR 45) COMPACTION. THE BASE COURSE (CBR 80) MATERIAL QUALITY.
- THE BASE COURSE (CBR 80) COMPACTION.
- THE PRIME OR PRIMER SEAL SPRAY AND APPLICATION RATES.
- THE AC CORE TESTS ANY CONCRETE TESTING REQUIRED.
- CCTV VIDEO FOR UNDERGROUND STORMWATER INFRASTRUCTURE WORK.
- PW.03 DURING CONSTRUCTION, DIGITAL PHOTOGRAPHS MUST: BE TAKEN OF COMPLEX CONSTRUCTIONS OR INSTALLATIONS WHICH WILL BE BELOW GROUND
- LEVEL OR NOT VISIBLE AFTER CONSTRUCTION COMPLETION OR AS REQUESTED ON SITE.
- BE TAKEN PRIOR TO BACKFILLING. INCLUDE A CHAINAGE OR EXACT LOCATION REFERENCE IN THE TITLE OF THE DIGITAL PHOTO
- BE DATE STAMPED.

#### COUNCIL WORKS

CW.01 TO PROVIDE AS CONSTRUCTED INFORMATION AS PER LOCAL COUNCIL SUBMISSION GUIDELINES REQUIREMENTS.

#### REINFORCED CONCRETE BLOCKWORK

- M1. CONCRETE BLOCKS SHALL BE BORAL 'CORE FILL BLOCKS', DOUBLE-U TYPE, OR SIMILAR APPROVED.
- M2. MINIMUM DURABILITY REQUIREMENTS:

LOCATION	SALT ATTACK RESISTANCE GRADE OF MASONRY UNIT	MORTAR CLASS	DURABILITY CLASS OF WALL TIES AND BUILT-IN COMPONENTS
INTERIOR MASONRY	GENERAL PURPOSE	M3	R3
EXTERIOR MASONRY ABOVE DAMP PROOF COURSE	GENERAL PURPOSE	M3	R3
BELOW DAMP PROOF COURSE OR IN CONTACT WITH GROUND	EXPOSURE	M4	R4

#### M3. MINIMUM STRENGTH REQUIREMENTS:

ELEMENT	STRENGTH OF MASONRY UNIT	MORTAR CLASS#
CONCRETE BLOCKWORK (REINF)	fuc = 15 MPa	M3

# UNLESS A HIGHER CLASSIFICATION IS REQUIRED FOR DURABILITY (REFER NOTE M2).

- M4. LAY BOTTOM COURSE OF BLOCKS ON FULL MORTAR BED. ALL PERPENDS SHALL BE FILLED WITH MORTAR, EXCEPT WEEPHOLES.
- M5. ALL CORES SHALL BE GROUTED UNLESS NOTED OTHERWISE. GROUT FOR CORE FILLING SHALL BE IN ACCORDANCE WITH AS3600, WITH THE FOLLOWING
- PROPERTIES:
- STRENGTH GRADE S20 MAX. AGGREGATE SIZE 10mm

WALLS AS FOLLOWS

SLUMP 230mm ± 25mm MIN. CEMENT CONTENT 300kg/m3 M7. PROVIDE VERTICAL CONTROL JOINTS IN MASONRY

WALL TYPE	JOINT WIDTH	MAX JOINT SPACING
CONCRETE BLOCKWORK (REINF)	15mm	12m

- M7. AT CORNERS, CONTROL JOINTS SHALL BE WITHIN HALF THE SPECIFIED JOINTS SPACING FROM THE CORNER. JOINTS SHALL BE SEALED WITH AN APPROVED FLEXIBLE SEALANT.
- PROVIDE JOINTS TO MATCH JOINTS IN SUPPORTING SLABS M8. PROVIDE CLEANOUT OPENINGS AT THE BASE OF ALL REINFORCED CORES AND REMOVE ALL MORTAR PROTRUSIONS BEFORE GROUTING. ADDITIONAL CLEANOUT OPENINGS SHALL
- BE PROVIDED ABOVE EACH HORIZONTAL POUR BREAK M9. MAXIMUM HEIGHT OF POUR FOR GROUTING SHALL NOT EXCEED 3.6m FOR 190 LOCKWORK, AND 0.8m FOR 140 BLOCKWORK. STOP POUR 50mm BELOW TOP OF BLOCK TO PROVIDE KEY FOR SUBSEQUENT POUR.

M10. GROUT SHALL BE THOROUGHLY COMPACTED IN THE CORES BY RODDING OR MECHANICAL

#### CONCRETE

- C1. ALL WORKMANSHIP AND MATERIALS SHALL BE IN ACCORDANCE WITH AS 3600 AND OTHER RELEVANT AUSTRALIAN STANDARDS.
- C2. CONCRETE SHALL BE SUPPLIED BY AN APPROVED MANUFACTURER IN ACCORDANCE WITH AS1379.
- C3. CONCRETE SHALL HAVE THE FOLLOWING PARAMETERS:

ELEMENT	SLUMP (mm)	AGGREGATE	f'c (MPa)	OTHER REQ
EXTERNAL VEHICLE SLAB	+ 80	20	N32	(1)

DENOTES SLUMP AT PLANT

OF THE ENGINEER.

- DENOTES MAXIMUM BASE SHRINKAGE STRAIN 600 x 10 -6 AT 56 DAYS (TO AS 1012 PART 13)
- SIZES OF CONCRETE ELEMENTS DO NOT INCLUDE THICKNESS OF APPLIED
- C5. BEAM DEPTHS ARE WRITTEN FIRST AND INCLUDE SLAB THICKNESS, IF ANY.
- HOLES, CHASES OR EMBEDMENT ITEMS, INCLUDING PIPES AND CONDUITS SHALL NOT BE PLACED IN CONCRETE MEMBERS WITHOUT PRIOR APPROVAL
- CONDUITS, PIPES AND LIKE SHALL NOT BE PLACED WITHIN THE CONCRETE COVER, NOR DISPLACE THE REINFORCEMENT LAYERS.
- CONSTRUCTION JOINTS (CJ) SHALL BE PROPERLY FORMED AND USED ONLY WHERE SHOWN OR SPECIFICALLY APPROVED BY THE ENGINEER. ALL
- CONSTRUCTION JOINTS SHALL BE THOROUGHLY SCABBLED. THE MAXIMUM HEIGHT OF POUR FOR CONCRETE ELEMENTS SHALL BE 3m

UNLESS METHOD OF PLACEMENT HAS BEEN APPROVED BY THE ENGINEER.

C10. CONCRETE SHALL BE THOROUGHLY COMPACTED IN THE FORMS BY MEANS OF MECHANICAL VIBRATION.

COLUMNS SHALL NOT BE POURED WITH THE SLAB OVER.

- C11. WHEN THE SHADE TEMPERATURE EXCEEDS 35°C, THE EXPOSED SURFACE OF CONCRETE SHALL BE SPRAYED WITH A FINE FILM OF APPROVED ALIPHATIC ALCOHOL DURING CONCRETE PLACEMENT AND FINISHING IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ENSURING ADEQUATE SUPPLY OF ALIPHATIC ALCOHOL ON SITE BEFORE COMMENCING CONCRETE WORK.
- C12. CURING OF CONCRETE SHALL COMMENCE WITHIN 2 HOURS OF FINISHING OPERATIONS AND SHALL BE MAINTAINED FOR A MINIMUM OF 7 DAYS USING AN APPROVED PROPRIETARY CURING COMPOUND IN ACCORDANCE WITH AS 3799 AND COMPATIBLE WITH THE PROPOSED FINISH OR CONTINUOUS PONDING WITH POTABLE WATER. THE CONTRACTOR TO SUBMIT PROPOSED CURING PROCEDURE FOR APPROVAL OF THE ENGINEER.
- C13. ALL CONCRETE DELIVERED TO SITE SHALL BE SUBJECT TO PROJECT ASSESSMENT IN ACCORDANCE WITH AS 1379.
- C14. THE CONTRACTOR SHALL NOMINATE A CONCRETE DELIVERY SUPERVISOR WHO SHALL BE A SUITABLE EXPERIENCED PERSON FOR THE APPROVAL OF THE ENGINEER, TO MONITOR THE DELIVERY AND PLACING OF THE CONCRETE FOR EACH POUR ON THE PROJECT. IN ADDITION, THE MANUFACTURER SHALL SAMPLE AND TEST FOR DRYING SHRINKAGE EACH TYPE OF CONCRETE SUPPLIED. AT LEAST EVERY MONTH DURING THE COURSE OF THE PROJECT OR FOR EVERY 1000 CUBIC METRES PLACED. NATA TEST CERTIFICATES SHALL BE FORWARDED TO THE ENGINEER. THE RESULTS OF THESE TESTS SHALL ALSO BE KEPT ON SITE.

#### C15. CONCRETE SAMPLES AND TESTS

ARRANGE FOR A NATA REGISTERED TESTING LABORATORY TO TAKE SAMPLES OF AND TEST CONCRETE FOR COMPRESSION, FLEXURAL TENSILE STRENGTH (SLABS ON GROUND ONLY) AND SLUMP.

COMPRESSION TEST SAMPLES SHALL CONSIST OF 3 STANDARD CYLINDERS (4 STANDARD CYLINDERS FOR POST-TENSIONED CONCRETE), TESTED FOR COMPRESSIVE STRENGTH AS FOLLOWS:

ONE (1) CYLINDER AT 3 DAYS FOR POST-TENSIONED CONCRETE ONLY. ONE (1) CYLINDER AT 7 DAYS. TWO (2) CYLINDERS AT 28 DAYS.

- 2 SAMPLES

1 SAMPLE PER TRUCK AT TIME OF POURING.

THE MINIMUM NUMBER OF DAILY SAMPLES SHALL BE AS FOLLOWS:

IN COLUMNS/WALLS: 1 SAMPLE PER TRUCK

6 TO 10 TRUCKS PER DAY - 3 SAMPLES

10 TO 20 TRUCKS PER DAY - 4 SAMPLES

ALL OTHER CONCRETE OF ANY ONE TYPE AS FOLLOWS: 1 TRUCK PER DAY - 1 SAMPLE

FOR EACH ADDITIONAL 10 TRUCKS PER DAY, 1 SAMPLE.

C16. REFER TO TYPICAL STRIPPING AND PROPPING DETAIL.

2 TO 5 TRUCKS PER DAY

**REVISIONS:** A ISSUED FOR APPROVAL

REVISION DESCRIPTION





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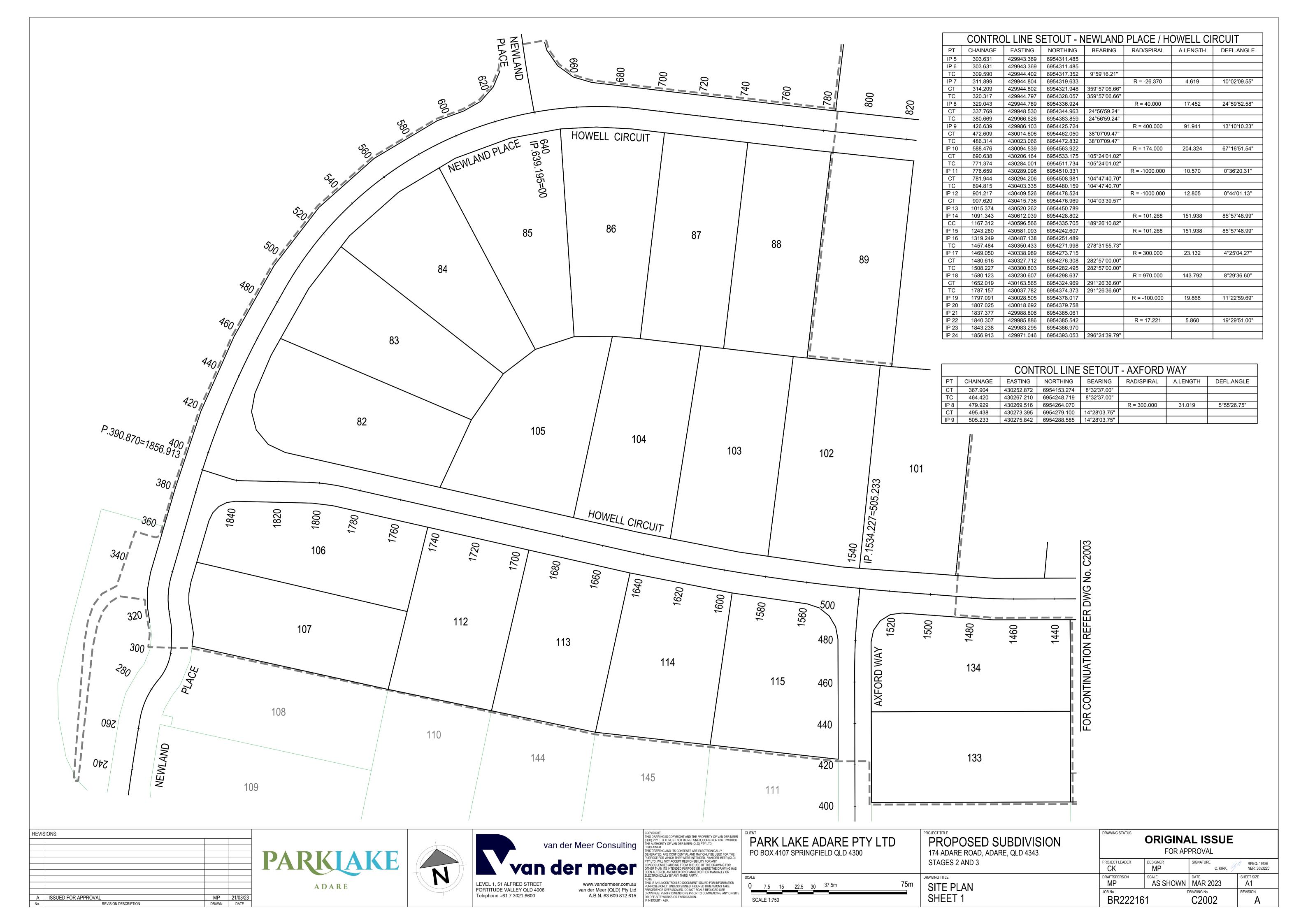
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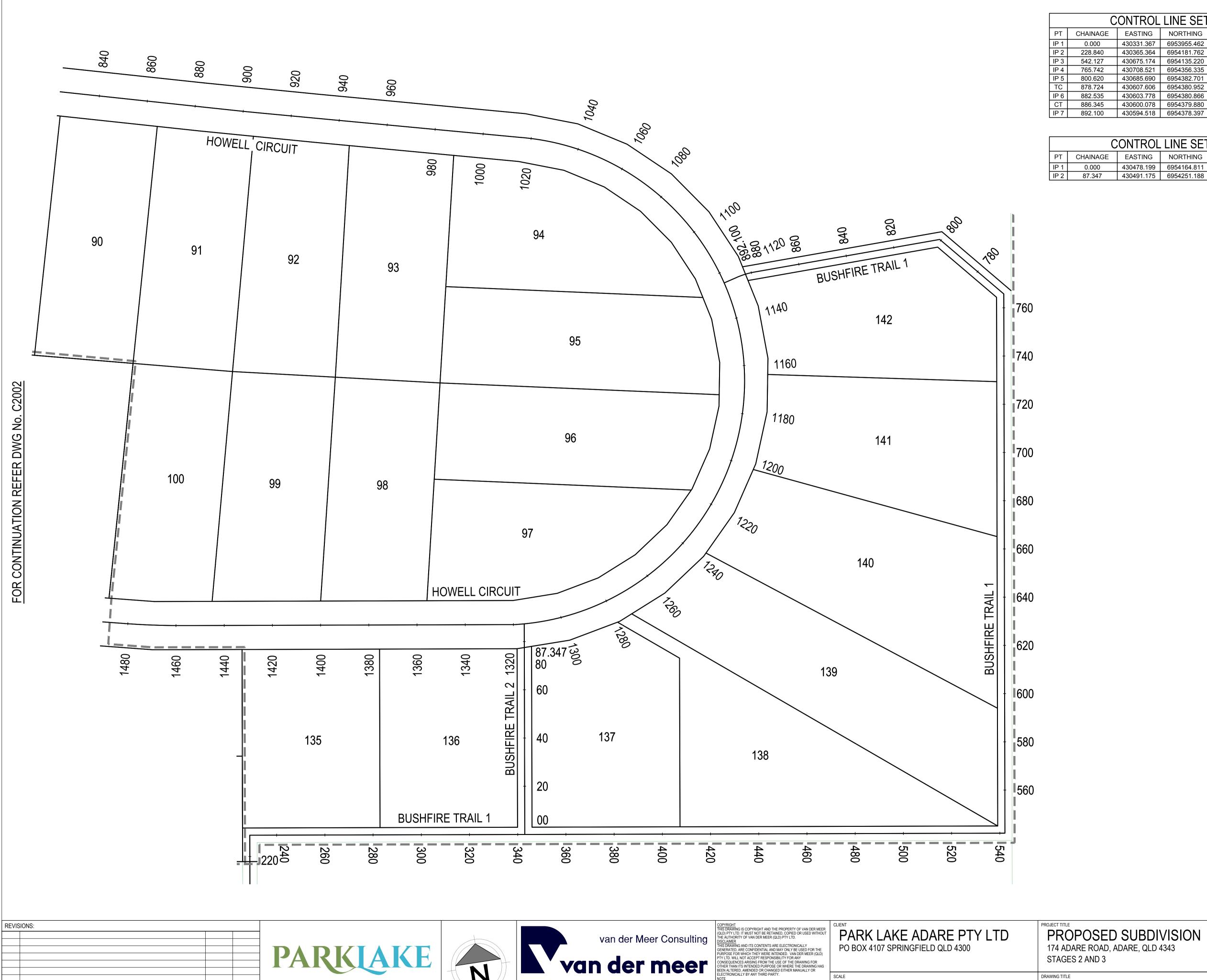
PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343 STAGES 2 AND 3

**ORIGINAL ISSUE** FOR APPROVAL PROJECT LEADER MP C KIRK DRAFTSPERSON SHEET SIZE MP AS SHOWN | MAR 2023 A1 STANDARD NOTES EVISION BR222161 C2001

RPEQ: 19536

NER: 3053220





LEVEL 1, 51 ALFRED STREET FORTITUDE VALLEY QLD 4006

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ADARE

REVISION DESCRIPTION

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0 7.5 15 22.5 30 37.5m

SCALE 1:750

CONTROL LINE SETOUT - BUSHFIRE TRAIL 1							
PT	CHAINAGE	EASTING	NORTHING	BEARING	RAD/SPIRAL	A.LENGTH	DEFL.ANGLE
IP 1	0.000	430331.367	6953955.462	8°32'36.49"			
IP 2	228.840	430365.364	6954181.762				
IP 3	542.127	430675.174	6954135.220				
IP 4	765.742	430708.521	6954356.335				
IP 5	800.620	430685.690	6954382.701				
TC	878.724	430607.606	6954380.952	268°42'59.30"			
IP 6	882.535	430603.778	6954380.866		R = -32.000	7.621	13°38'44.65"
СТ	886.345	430600.078	6954379.880	255°04'14.65"			
IP 7	892.100	430594.518	6954378.397	255°04'14.65"			

	C	ONTROL	LINE SET	OUT - BU	SHFIRE TRA	AIL 2	
P	T CHAINAGE	EASTING	NORTHING	BEARING	RAD/SPIRAL	A.LENGTH	DEFL.ANGLE
ΙP	1 0.000	430478.199	6954164.811	8°32'37.00"			
ПР	2 87 347	430491 175	6954251 188	8°32'37 00"			

**ORIGINAL ISSUE** 

FOR APPROVAL

AS SHOWN MAR 2023

C2003

C. KIRK

RPEQ: 19536 NER: 3053220

SHEET SIZE

A1

REVISION

DESIGNER MP

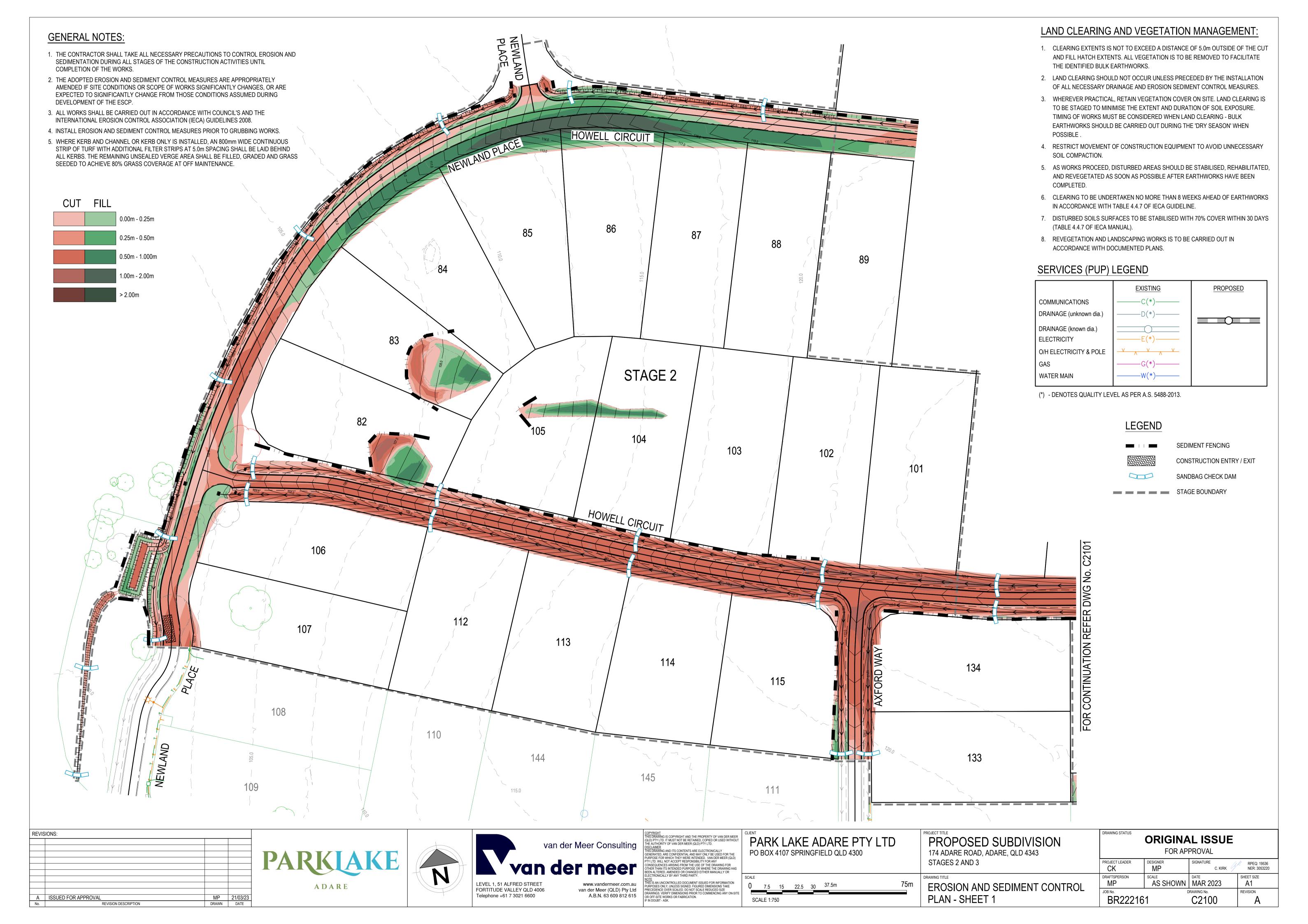
DRAFTSPERSON MP

BR222161

DRAWING TITLE

SITE PLAN

SHEET 2





#### LAND CLEARING AND VEGETATION MANAGEMENT:

- 1. CLEARING EXTENTS IS NOT TO EXCEED A DISTANCE OF 5.0m OUTSIDE OF THE CUT AND FILL HATCH EXTENTS. ALL VEGETATION IS TO BE REMOVED TO FACILITATE THE IDENTIFIED BULK EARTHWORKS.
- 2. LAND CLEARING SHOULD NOT OCCUR UNLESS PRECEDED BY THE INSTALLATION OF ALL NECESSARY DRAINAGE AND EROSION SEDIMENT CONTROL MEASURES.
- 3. WHEREVER PRACTICAL, RETAIN VEGETATION COVER ON SITE. LAND CLEARING IS TO BE STAGED TO MINIMISE THE EXTENT AND DURATION OF SOIL EXPOSURE. TIMING OF WORKS MUST BE CONSIDERED WHEN LAND CLEARING - BULK EARTHWORKS SHOULD BE CARRIED OUT DURING THE 'DRY SEASON' WHEN POSSIBLE.
- 4. RESTRICT MOVEMENT OF CONSTRUCTION EQUIPMENT TO AVOID UNNECESSARY SOIL COMPACTION.
- 5. AS WORKS PROCEED, DISTURBED AREAS SHOULD BE STABILISED, REHABILITATED, AND REVEGETATED AS SOON AS POSSIBLE AFTER EARTHWORKS HAVE BEEN COMPLETED.
- 6. CLEARING TO BE UNDERTAKEN NO MORE THAN 8 WEEKS AHEAD OF EARTHWORKS IN ACCORDANCE WITH TABLE 4.4.7 OF IECA GUIDELINE.
- 7. DISTURBED SOILS SURFACES TO BE STABILISED WITH 70% COVER WITHIN 30 DAYS (TABLE 4.4.7 OF IECA MANUAL).
- 8. REVEGETATION AND LANDSCAPING WORKS IS TO BE CARRIED OUT IN ACCORDANCE WITH DOCUMENTED PLANS.

#### SERVICES (PUP) LEGEND

	<u>EXISTING</u>	PROPOSED
COMMUNICATIONS	C(*)	
DRAINAGE (unknown dia.)	D(*)	
DRAINAGE (known dia.)		
ELECTRICITY	———E(*)———	
O/H ELECTRICITY & POLE		
GAS	G(*)	
WATER MAIN	W(*)	

(\*) - DENOTES QUALITY LEVEL AS PER A.S. 5488-2013.



SEDIMENT FENCING CONSTRUCTION ENTRY / EXIT

SANDBAG CHECK DAM

**ORIGINAL ISSUE** 

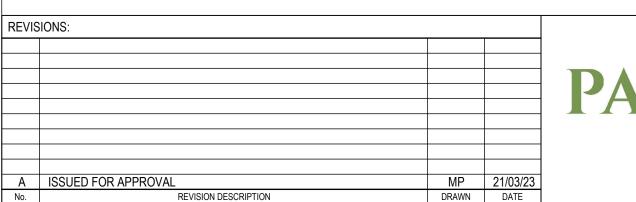
RPEQ: 19536 NER: 3053220

SHEET SIZE

A1

REVISION

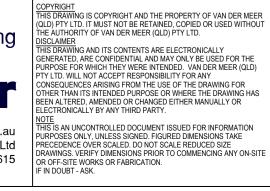
STAGE BOUNDARY





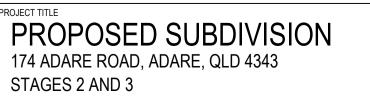




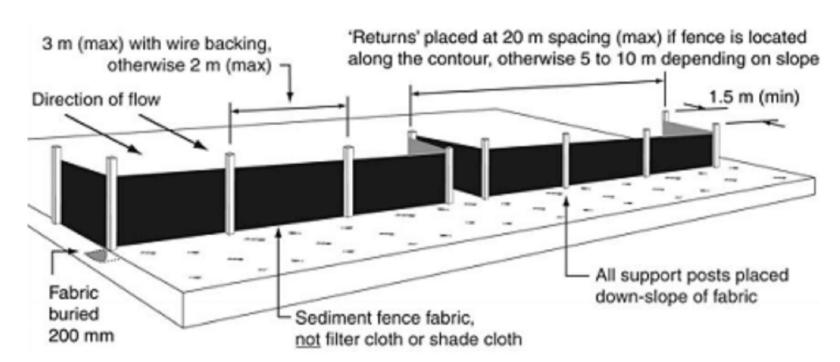




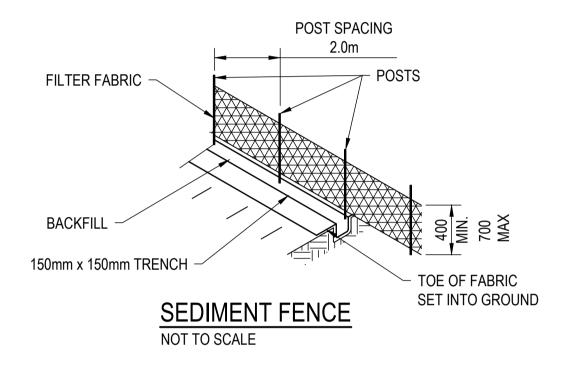
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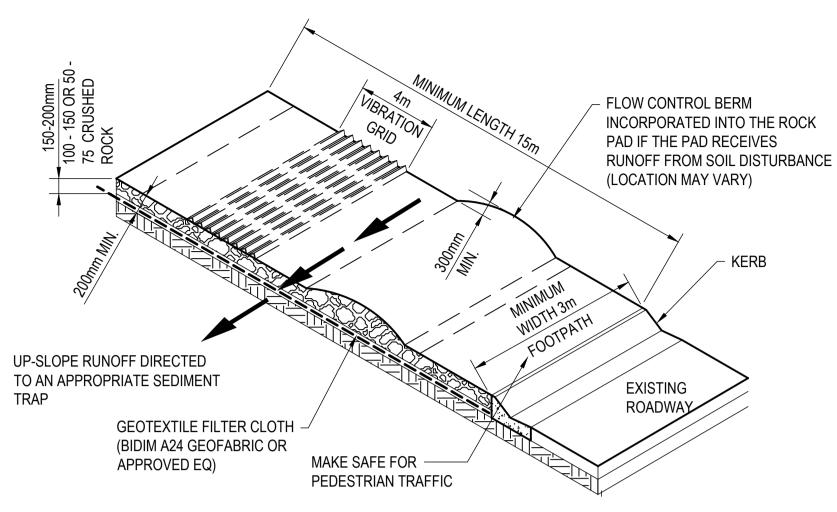


174 ADARE ROAD, ADARE, QLD 4343		FOR AP	PROVAL
STAGES 2 AND 3	PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK
EROSION AND SEDIMENT CONTROL	DRAFTSPERSON MP	AS SHOWN	MAR 2023
PLAN - SHEET 1	JOB No. BR22216	_	C2101

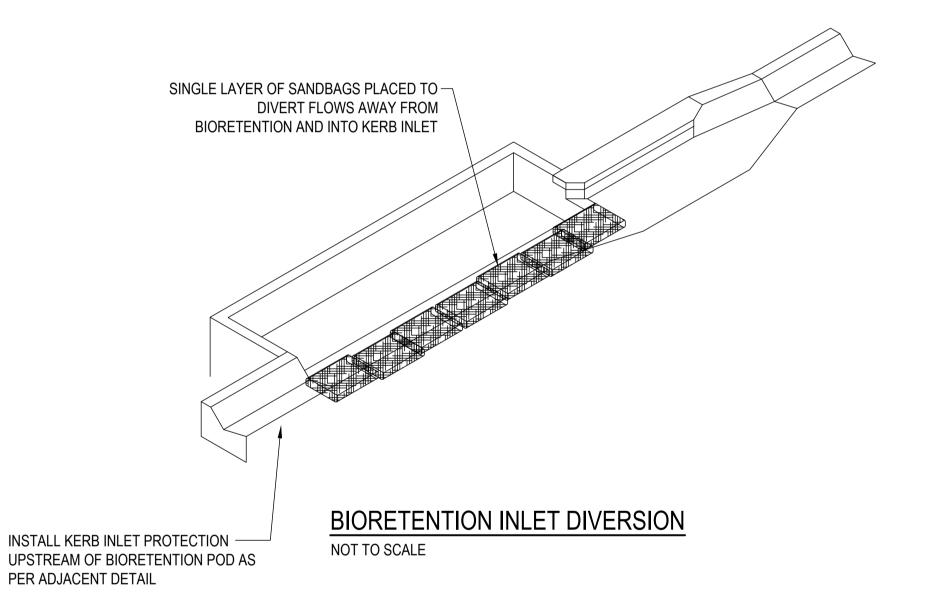


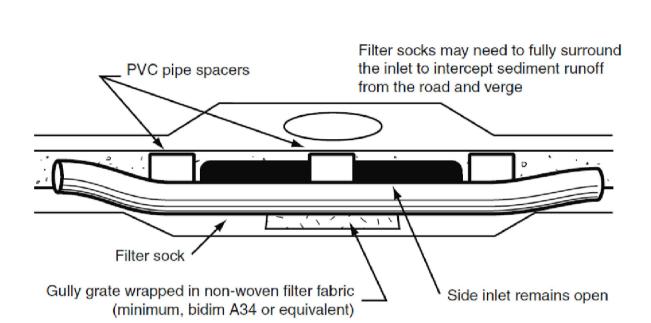
# TYPICAL INSTALLATION OF A SEDIMENT FENCE NOT TO SCALE





# CONSTRUCTION ENTRY/EXIT POINT DETAIL NOT TO SCALE





ON-GRADE KERB INLET SEDIMENT TRAP

4.0 m (min)

1 m (max) on public roads

Filter socks overlap

NOT TO SCALE

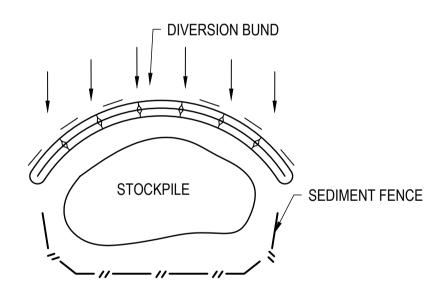
onto kerb 🔨

Sediment

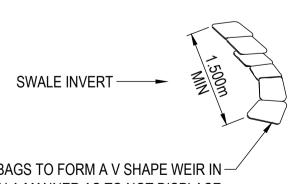
pond

Run-off

#### SAG INLET SEDIMENT TRAP NOT TO SCALE

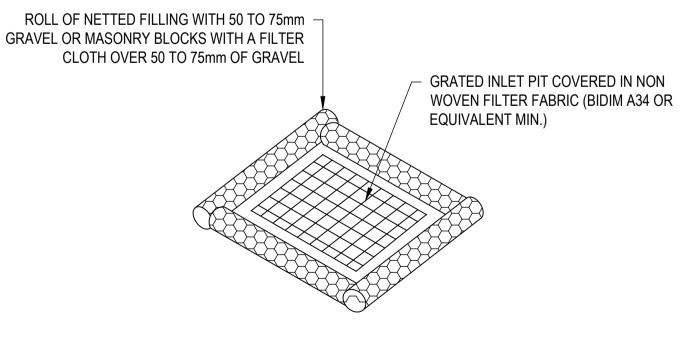


STOCKPILE SEDIMENT CONTROL NOT TO SCALE



SINGLE LAYER OF SANDBAGS TO FORM A V SHAPE WEIR IN-CHANNEL. PLACE IS SUCH A MANNER AS TO NOT DISPLACE STORMWATER RUNOFF OUTSIDE OF SWALE I.E. CENTRE OF WEIR MUST BE LOWER THAN SURROUNDING SURFACE

TYPICAL SAND BAG CHECK DAM NOT TO SCALE



FIELD INLET SEDIMENT TRAP

NOT TO SCALE

REVISIONS:

REVISION DESCRIPTION





DN75, 300 THICK DUMPED ROCK SCOUR -

PROTECTION OVER GEOFABRIC TO

**EMERGENCY SPILLWAY** 

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-DN75, 300 THICK DUMPED ROCK SCOUR

PROTECTION OVER GEOFABRIC TO

EMERGENCY SPILLWAY

PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343 STAGES 2 AND 3
DRAWING TITLE

**EROSION AND SEDIMENT** 

CONTROL DETAILS

DRAWING STATUS			
PROJECT LEADER CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220
DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1
JOB No. BR22216		C2190	REVISION

SEDIMENT BASIN EMERGENCY SPIL

SEDIMENT BASIN EMERGENCY SPILLWAY DETAIL NOT TO SCALE

TOP OF BUND 1.5m

#### SEDIMENT FENCE

#### **MATERIALS**

- 1. FABRIC: POLYPROPYLENE, POLYAMIDE, NYLON, POLYESTER OR POLYETHYLENE WOVEN OR NON-WOVEN FABRIC, AT LEAST 700mm IN WIDTH AND A MINIMUM UNIT WEIGHT OF 140GSM.
- 2. SUPPORT POSTS/STAKES AND STEEL STAR PICKETS SUITABLE FOR ATTACHING FABRIC.

#### INSTALLATION

- 1. WHERE POSSIBLE INSTALL SEDIMENT FENCE AT LEAST 2m FROM THE TOE OF ANY FILLING OPERATIONS THAT MAY RESULT IN SHIFTING SOIL/FILL DAMAGING THE FENCE.
- 2. ENSURE THE EXTREME ENDS OF THE FENCE ARE TURNED UP THE SLOPE AT LEAST 1.5m OR AS NECESSARY TO MINIMISE WATER BYPASSING AROUND THE FENCE.
- 3. ENSURE THE SEDIMENT FENCE IS INSTALLED IN A MANNER THAT AVOIDS THE CONCENTRATION OF FLOW ALONG THE FENCE AND THE UNDESIRABLE DISCHARGE OF WATER AROUND THE ENDS OF THE FENCE.
- 4. IF THE SEDIMENT FENCE IS TO BE INSTALLED ALONG THE EDGE OF THE EXISTING TREES, ENSURE CARE IS TAKEN TO PROTECT THE TREES AND THEIR ROOT SYSTEMS DURING INSTALLATION OF THE FENCE.
- 5. UNLESS DIRECTED BY THE SITE SUPERVISOR OR THE APPROVED PLANS, EXCAVATE A 200mm WIDE BY 200mm DEEP TRENCH ALONG THE PROPOSED FENCE LINE, PLACING THE EXCAVATED MATERIAL ON THE UP-SLOPE SIDE OF THE TRENCH.
- 6. ALONG THE LOWER SIDE OF THE TRENCH, APPROPRIATELY SECURE THE STAKES INTO THE GROUND SPACED NO GREATER THAN 3m IF SUPPORTED BY A TOP SUPPORT WIRE OR WEIR MESH BACKING, OTHERWISE NO GREATER THAN 2m.
- 7. WHEREVER POSSIBLE, CONSTRUCT THE SEDIMENT FENCE FROM A CONTINUOUS ROLL OF FABRIC. TO JOIN FABRIC ATTACH EACH END OF TWO OVERLAPPING STAKES WITH THE FABRIC FOLDING AROUND THE ASSOCIATED STAKE ONE TURN AND WITH TWO STAKES TIED TOGETHER WITH THE WIRE METHOD OR OVERLAP THE FABRIC TO THE NEXT ADJACENT SUPPORT POST.
- 8. SECURELY ATTACH THE FABRIC TO THE SUPPORT POSTS USING 25 X 12.5mm STAPLES, OR TIE WIRE AT MAXIMUM 150mm SPACING.
- 9. SECURELY ATTACH THE FABRIC TO THE SUPPORT WIRE/MESH (IF ANY) AT A MAXIMUM SPACING
- 10. ENSURE THE COMPLETED SEDIMENT FENCE IS AT LEAST 450mm, BUT NOT MORE THAN 700mm HIGH. IF A SPILL THROUGH WEIR IS INSTALLED, ENSURE THE CREST OF THE WEIR IS AT LEAST 300mm ABOVE GROUND LEVEL.
- 11. BACKFILL THE TRENCH AND TAMP THE FILL TO FIRMLY ANCHOR THE BOTTOM OF THE FABRIC AND MESH TO PREVENT WATER FROM FLOWING UNDER THE FENCE.
- 12. IF IT IS NOT POSSIBLE TO ANCHOR THE FABRIC IN AN EXCAVATED TRENCH, THEN USE A CONTINUOUS LAYER OF SAND OR AGGREGATE TO HOLD THE FABRIC FIRMLY ON THE GROUND.

#### **MAINTENANCE**

- 1. INSPECT THE SEDIMENT FENCE AT LEAST WEEKLY AND AFTER ANY SIGNIFICANT RAIN. MAKE NECESSARY REPAIRS IMMEDIATELY.
- 2. REPAIR ANY TORN SECTIONS WITH A CONTINUOUS PIECE OF FABRIC FROM POST TO POST.
- 3. WHEN MAKING REPAIRS, ALWAYS RESTORE THE SYSTEM TO ITS ORIGINAL CONFIGURATION UNLESS AN AMENDED LAYOUT IS REQUIRED OR SPECIFIED.
- 4. IF THE FENCE IS SAGGING BETWEEN STAKES, INSTALL ADDITIONAL SUPPORT POSTS.
- 5. REMOVE ACCUMULATED SEDIMENT IF THE SEDIMENT DEPOSIT EXCEEDS A DEPTH OF 1/3 THE
- 6. DISPOSE OF SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

#### SEDIMENT BASINS

#### **GENERAL**

- 1. SEDIMENT BASIN TO BE LOCATED ABOVE THE 5YR FLOOD LINE. INSTALL SEDIMENT BASINS FOR ALL CATCHMENTS ACROSS THE PROJECT AREA.
- 2. MATERIALS USED IN THE CONSTRUCTION OF SEDIMENT BASINS SHOULD NOT HAVE AN EMERSON NUMBER OF 3 OR ABOVE (I.E. DISPERSIVE SOILS SUCH AS THE SUBSOILS THAT CAN BE ENCOUNTERED AT THE SITE CANNOT BE USED TO CONSTRUCT SEDIMENT BASINS).
- 3. A "FULL OF SEDIMENT" MARKER MUST BE PLACED IN THE SEDIMENT BASIN TO SHOW THE DESIGN DEPTH OF THE SOIL/STORAGE ZONE VOLUME AND TO INDICATE WHEN REMOVAL OF THE SEDIMENT IS TO BE CARRIED OUT
- 4. CONSTRUCTED SEDIMENT BASINS TO BE FULLY OPERATIONAL THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL THE BASINS CATCHMENT AREA ACHIEVES 70% GROUND COVER ON ALL SOIL SURFACES.
- 5. FLOCCULATION REQUIREMENTS TO BE IN ACCORDANCE WITH TABLE B17 OF THE IECA GUIDELINES. IN GENERAL 32kg OF GYPSUM TO BE ADDED TO 100m3 OF STORED WATER.

#### **MAINTENANCE**

- 1. INSPECT THE SEDIMENT BASIN DURING THE FOLLOWING PERIODS AS STATED WITHIN PAGE B.52 OF THE IECA GUIDELINES:
- 1.1. DURING CONSTRUCTION TO DETERMINE WHETHER MACHINERY, FALLING TREES OR CONSTRUCTION ACTIVITY HAS DAMAGED ANY COMPONENT OF THE SEDIMENT BASIN. IF DAMAGE HAS OCCURRED, REPAIR IT.
- 1.2. AFTER EACH RUNOFF EVENT. INSPECT THE EROSION DAMAGE AT FLOW ENTRY AND EXIT POINTS. IF DAMAGE HAS OCCURRED, MAKE THE NECESSARY REPAIRS.
- 1.3. AT LEAST WEEKLY DURING THE NOMINATED WET SEASON (IF ANY) OTHERWISE AT LEAST FORTNIGHTLY.
- 1.4. PRIOR TO, AND IMMEDIATELY AFTER, PERIODS OF "STOP WORK" OR SITE "SHUTDOWN"
- 2. CLEAN OUT ACCUMULATED SEDIMENT WHEN IT REACHES THE MARKER BOARD/POST, AND RESTORE THE ORIGINAL STORAGE VOLUME. PLACE SEDIMENT IN A DISPOSAL AREA OR, IF APPROPRIATE, MIX WITH DRY SOIL ON THE SITE.
- 3. DO NOT DISPOSE OF SEDIMENT IN A MANNER THAT WILL CREATE AN EROSION OR POLLUTION HAZARD.
- 4. CHECK ALL VISIBLE PIPE CONNECTIONS FOR LEAKS, AND REPAIR AS NECESSARY.
- 5. CHECK FILL MATERIAL IN THE DAM FOR EXCESSIVE SETTLEMENT, SLUMPING OF THE SLOPES OR PIPING BETWEEN THE CONDUIT AND THE EMBANKMENT; MAKE ALL NECESSARY
- 6. REMOVE ALL TRASH AND OTHER DEBRIS FROM THE BASIN AND RISER.
- 7. SUBMERGED INFLOW PIPES MUST BE INSPECTED AND DE-SILTED (AS REQUIRED) AFTER EACH INFLOW EVENT.

## REMOVAL OR CONVERSION OF SEDIMENT BASIN

- 1. WHEN GRADING AND CONSTRUCTION IN THE DRAINAGE AREA ABOVE A TEMPORARY SEDIMENT BASIN IS COMPLETED AND THE DISTURBED AREAS ARE ADEQUATELY STABILISED, THE BASIN MUST BE REMOVED OR OTHERWISE INCORPORATED INTO THE PERMANENT STORMWATER DRAINAGE SYSTEM. IN EITHER CASE, SEDIMENT SHOULD BE CLEARED AND PROPERLY DISPOSED OF AND THE BASIN AREA STABILISED
- 2. BEFORE STARTING ANY MAINTENANCE WORK ON THE BASIN OR SPILLWAY, INSTALL ALL NECESSARY SHORT-TERM SEDIMENT CONTROL MEASURES DOWNSTREAM OF THE SEDIMENT
- 3. ALL WATER AND SEDIMENT MUST BE REMOVED FROM THE BASIN PRIOR TO THE DAM'S REMOVAL. DISPOSE OF SEDIMENT AND WATER IN A MANNER THAT WILL NOT CREATE AN **EROSION OR POLLUTION HAZARD**
- 4. BRING THE DISTURBED AREA TO A PROPER GRADE, THEN SMOOTH, COMPACT AND STABILISE OR REVEGETATE AS REQUIRED TO ESTABLISH A STABLE LAND SURFACE.

## MATERIAL STOCKPILING:

- 1. THE CONSTRUCTION CONTRACTOR IS TO ADHERE TO THE FOLLOWING SOIL AND STOCKPILE MANAGEMENT PRACTISES. STOCKPILES OF ERODIBLE MATERIAL THAT HAS THE POTENTIAL TO CAUSE ENVIRONMENTAL HARM IF DISPLACED MUST BE:
- 2. APPROPRIATELY PROTECTED FROM WIND, RAIN, CONCENTRATED SURFACE FLOW AND EXCESSIVE UP-SLOPE STORMWATER SURFACE FLOWS.
- 3. LOCATED AT LEAST 2m FROM ANY HAZARDOUS AREA, RETAINED VEGETATION, OR CONCENTRATED DRAINAGE LINE.
- 4. LOCATED UP-SLOPE OF AN APPROPRIATE SEDIMENT CONTROL SYSTEM.
- 5. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 28 DAYS.
- 6. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 10 DAYS DURING THOSE MONTHS THAT HAVE A HIGH EROSION RISK.
- 7. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 5 DAYS DURING THOSE MONTHS THAT HAVE A HIGH EROSION RISK.
- 8. A SUITABLE FLOW DIVERSION SYSTEM MUST BE ESTABLISHED IMMEDIATELY UP-SLOPE OF A STOCKPILE OF ERODIBLE MATERIAL THAT HAS THE POTENTIAL TO CAUSE ENVIRONMENTAL HARM IF DISPLACED, IF THE UP-SLOPE CATCHMENT AREA DRAINING TO THE STOCKPILE EXCEEDS 1500m2.

#### STABILISED ENTRY/EXIT NOTES

#### MATERIALS

- ROCK: WELL GRADED, HARD, ANGULAR, EROSION RESISTANT ROCK, NOMINAL DIAMETER OF 50mm TO 75mm (SMALL DISTURBANCES) OR 100 TO 150mm (LARGE DISTURBANCES). ALL REASONABLE MEASURES MUST BE TAKEN TO OBTAIN ROCK OF NEAR UNIFORM SIZE.
- FOOTPATH STABILISING AGGREGATE: 25 TO 50mm GRAVEL OR AGGREGATE.
- GEOTEXTILE FABRIC: HEAVY-DUTY, NEEDLE-PUNCHED, NON-WOVEN FILTER CLOTH ('BIDIM' A24 OR EQUIVALENT).

- 1. REFER TO APPROVED PLANS FOR LOCATION AND DIMENSIONAL DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION, DIMENSIONS, OR METHOD OF INSTALLATION, CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER
- 2. CLEAR THE LOCATION OF THE VIBRATION GRID, REMOVING STUMPS, ROOTS AND OTHER VEGETATION TO PROVIDE A FIRM FOUNDATION SO THAT THE ROCK IS NOT PRESSED INTO SOFT GROUND. CLEAR SUFFICIENT WIDTH TO ALLOW PASSAGE OF LARGE VEHICLES, BUT CLEAR ONLY THAT NECESSARY FOR THE EXIT. DO NOT CLEAR ADJACENT AREAS UNTIL THE REQUIRED EROSION AND SEDIMENT CONTROL DEVICES ARE IN PLACE
- 3. IF THE EXPOSED SOIL IS SOFT, PLASTIC OR CLAYEY, PLACE A SUB-BASE OF CRUSHED ROCK OR A LAYER OF HEAVY-DUTY FILTER CLOTH TO PROVIDE A FIRM FOUNDATION.
- 4. ENSURE THAT THE INSTALLATION OF THE VIBRATION GRID INCLUDES ADEQUATE SEDIMENT STORAGE VOLUME UNDER THE GRID. WHERE NECESSARY, INSTALL SUITABLE PRECAST SEDIMENT COLLECTION CHAMBERS
- 5. PLACE A ROCK PAD/RAMP FORMING A MINIMUM 200mm THICK LAYER OF CLEAN, OPEN-VOID ROCK OVER THE ROADWAY BETWEEN THE VIBRATION GRID AND THE SEALED STREET TO PREVENT TYRES FROM PICKING UP MORE SOIL AFTER THEY HAVE BEEN CLEANED.
- 6. IF THE ASSOCIATED CONSTRUCTION SITE IS UP-SLOPE OF THE ROCK PAD, THUS CAUSING STORMWATER RUNOFF TO FLOW TOWARDS THE ROCK PAD, THEN FORM A MINIMUM 300mm HIGH FLOW CONTROL BERM ACROSS THE ROCK PAD TO DIVERT SUCH RUNOFF TO A SUITABLE SEDIMENT TRAP
- 7. THE TOTAL LENGTH OF THE VIBRATION GRIP AND ROCK RAMPS SHOULD BE AT LEAST 15m WHERE PRACTICABLE, AND AS WIDE AS THE FULL WIDTH OF THE ENTRY OR EXIT AND AT LEAST 3m. THE ROCK RAMP SHOULD COMMENCE AT THE EDGE OF THE OFF-SITE SEALED ROAD OR PAVEMENT
- 8. FLARE THE END OF THE ROCK PAD WHERE IT MEETS THE PAVEMENT SO THAT THE WHEELS OF TURNING VEHICLES DO NOT TRAVEL OVER UNPROTECTED SOIL.

- 1. INSPECT VIBRATION GRID PRIOR TO FORECAST RAIN, DAILY DURING EXTENDED PERIODS OF RAINFALL, AFTER SIGNIFICANT RUNOFF-PRODUCING RAINFALL. OR OTHERWISE AT FORTNIGHTLY INTERVALS
- 2. IF SAND, SOIL, SEDIMENT OR MUD IS TRACKED OR WASHED ONTO THE ADJACENT SEALED ROADWAY, THEN SUCH MATERIAL MUST BE PHYSICALLY REMOVED, FIRST USING A SQUARE-EDGED SHOVEL, AND THEN A STIFF-BRISTLED BROOM, AND THEN BY A MECHANICAL VACUUM UNIT, IF AVAILABLE.
- 3. IF NECESSARY FOR SAFETY REASONS, THE ROADWAY SHALL ONLY BE WASHED CLEAN AFTER ALL REASONABLE EFFORTS HAVE BEEN TAKEN TO SHOVEL AND SWEEP THE MATERIAL FROM THE ROADWAY.
- 4. WHEN THE VOIDS BETWEEN THE ROCK BECOMES FILLED WITH MATERIAL AND THE EFFECTIVENESS OF THE ROCK RAMPS ARE REDUCED TO A POINT WHERE SEDIMENT IS BEING TRACKED OFF THE SITE, A NEW 100mm LAYER OF ROCK MUST BE ADDED AND/OR THE ROCK PAD MUST BE EXTENDED.
- 5. ENSURE ANY ASSOCIATED DRAINAGE CONTROL MEASURES ARE MAINTAINED IN ACCORDANCE WITH THEIR DESIRED OPERATIONAL CONDITION.
- 6. DISPOSE OF SEDIMENT AND DEBRIS IN A MANNER THAT WILL NOT CREATE AN EROSION OR POLLUTION HAZARD.

#### **CHECK DAM SEDIMENT TRAPS**

#### INSTALLATION

- 1. REFER TO APPROVED PLANS FOR LOCATION AND INSTALLATION DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION OR METHOD OF INSTALLATION CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.
- 2. PRIOR TO PLACEMENT OF THE SEDIMENT TRAP, ENSURE THE DRAINAGE CHANNEL IS DEEP ENOUGH TO PREVENT WATER BEING UNSAFELY DIVERTED OUT OF THE DRAIN ONCE THE CHECK DAMS ARE INSTALLED.
- 3. LOCATE EACH CHECK DAM SEDIMENT TRAP AS DIRECTED WITHIN THE APPROVED PLANS, OR OTHERWISE AT SUCH A SPACING TO ACHIEVE THE REQUIRED SEDIMENT TRAPPING OUTCOMES.
- 4. IF THE CHECK DAMS ARE ALSO BEING USED TO CONTROL EROSION WITHIN THE DRAINAGE CHANNEL, THEN LOCATE EACH SUCCESSIVE CHECK DAM SUCH THAT THE CREST OF THE IMMEDIATE DOWNSTREAM DAM IS LEVEL WITH THE CHANNEL INVERT AT THE IMMEDIATE UPSTREAM CHECK DAM.
- 5. ENSURE SAND BAGS EXTEND UP THE CHANNEL BANKS (WHERE PRACTICAL) TO A LEVEL AT LEAST 100mm ABOVE THE CREST LEVEL OF THE CHECK DAM.

#### **MAINTENANCE**

- 1. INSPECT EACH CHECK DAM AND THE DRAINAGE CHANNEL AT LEAST WEEKLY AND AFTER RUNOFF-PRODUCING RAINFALL.
- 2. CORRECT ALL DAMAGE IMMEDIATELY. IF SIGNIFICANT EROSION OCCURS BETWEEN ANY OF THE CHECK DAMS, THEN CHECK THE SPACING OF THE DAMS AND WHERE NECESSARY INSTALL INTERMEDIATE CHECK DAMS OR A SUITABLE CHANNEL LINER.
- 3. CHECK FOR DISPLACEMENT OF THE CHECK DAMS.
- 4. CHECK FOR SOIL SCOUR AROUND THE ENDS OF EACH CHECK DAM. IF SUCH EROSION IS OCCURRING, CONSIDER EXTENDING THE WIDTH OF THE CHECK DAM TO AVOID SUCH PROBLEMS.
- IF SEVERE SOIL EROSION OCCURS EITHER UNDER OR AROUND THE CHECK DAMS. THEN SEEK EXPERT ADVICE ON AN ALTERNATIVE TREATMENT MEASURE.
- 6. DE-SILT SEDIMENT TRAP IF THE SEDIMENT LEVEL EXCEEDS 1/3 THE CREST HEIGHT.
- 7. DISPOSE OF COLLECTED SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

#### KERB INLET SEDIMENT TRAPS

FROM ICEA (INTERNATIONAL EROSION SEDIMENT ASSOCIATION) AUSTRALASIA STANDARD DRAWING ESC-03 (DEC 09).

#### **MATERIALS**

- 1. SOCKS: MINIMUM 200mm DIAMETER SYNTHETIC OR BIODEGRADABLE TUBES MANUFACTURED FROM NON-WOVEN OR COMPOSITE FABRIC SUITABLE FOR THE 'FILTRATION' OF COARSE SEDIMENTS.
- 2. FILL MATERIAL: STRAW, CANE MULCH, COMPOSITE MATERIAL (AS4454), COARSE SAND, OR CLEAN AGGREGATE.
- 3. STAKES: MINIMUM 25 x 25mm TIMBER.

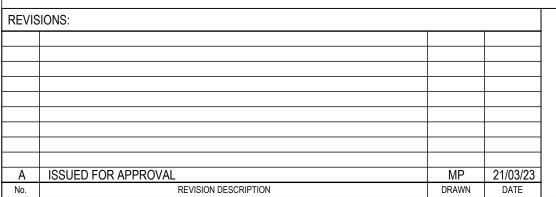
#### INSTALLATION

- 1. REFER TO APPROVED PLANS FOR LOCATION AND INSTALLATION DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION, DIMENSIONS OR METHOD OF INSTALLATION CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.
- 2. ENSURE THE SOCKS ARE PLACED INDIVIDUALLY OR COLLECTIVELY (AS A SINGLE SEDIMENT TRAP)
  - (i) LEAKAGE AROUND OR UNDER THE SOCKS IS MINIMISED
- (ii) ADJOINING SOCKS ARE TIGHTLY BUTTED OR OVERLAPPED AT LEAST 450mm.
- (iii) THE SURFACE AREA OF POTENTIAL WATER PONDING UP-SLOPE OF EACH SEDIMENT TRAP IS MAXIMISED.
- (iv) TO THE MAXIMUM DEGREE PRACTICAL, ALL SEDIMENT-LADEN WATER WILL PASS THROUGH THE FORMED POND BEFORE FLOWING OVER THE DOWN-SLOPE END OF THE SEDIMENT TRAP.
- 3. WHEN PLACED ACROSS THE INVERT OF MINOR DRAINS, ENSURE THE SOCKS ARE PLACED SUCH THAT (i) THE CREST OF THE DOWNSTREAM SOCK IS LEVEL WITH THE CHANNEL INVERT AT THE IMMEDIATE UPSTREAM SOCK (IF ANY):
- (ii) EACH SOCK EXTENDS UP THE CHANNEL BANKS SUCK THAT THE CREST OF THE SOCK AT ITS LOWEST POINT IS LOWER THAN GROUND LEVEL AT EITHER END OF THE SOCK.
- 4. IF STAKES ARE REQUIRED TO ANCHOR THE SOCKS, THEIR SPACING DOES NOT EXCEED 1.2m OR SIX TIMES THE SOCK DIAMETER (WHICHEVER IS THE LESSER). A MAXIMUM STAKE SPACING OF 0.3m APPLIES WHEN USED TO FORM CHECK DAMS.

#### **MAINTENANCE**

- 1. INSPECT ALL FILTER SOCKS PRIOR TO FORECAST RAIN, DAILY DURING EXTENDED PERIODS OF RAINFALL, AFTER SIGNIFICANT RUNOFF PRODUCING STORMS OR OTHERWISE AT WEEKLY INTERVALS.
- 2. REPAIR OR REPLACE DAMAGED SOCKS.
- 3. THE BULK OF THE SEDIMENT COLLECTED BEHIND THE FILTER SOCKS SHOULD BE REMOVED BY SHOVEL AFTER EACH STORM EVENT.
- 4. REMOVE COLLECTED SEDIMENT AND DISPOSE OF IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

- 1. ALL SAND, SOIL, SEDIMENT OR MUD MUST BE PHYSICALLY REMOVED FROM SEALED SURFACES, FIRST USING A SQUARE-EDGED SHOVEL, AND THEN A STIFF-BRISTLED BROOM, AND THEN BY A MECHANICAL VACUUM UNIT, IF AVAILABLE.
- 2. IF NECESSARY FOR SAFETY REASONS, THE SEALED SURFACE SHALL ONLY BE WASHED CLEAN AFTER ALL REASONABLE EFFORTS HAVE BEEN TAKEN TO SHOVEL AND SWEEP THE MATERIAL FROM THE SURFACE.
- 3. DISPOSE OF COLLECTED SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.
- 4. ALL SYNTHETIC (PLASTIC) MESH OR OTHER NON READILY BIODEGRADABLE MATERIAL MUST BE REMOVED FROM THE SITE ONCE THE SLOPE OR DRAIN IS STABILISED, OR THE SOCKS HAVE DETERIORATED TO A POINT WHERE THEY ARE NO LONGER PROVIDING THEIR INTENDED DRAINAGE OR SEDIMENT CONTROL FUNCTION.







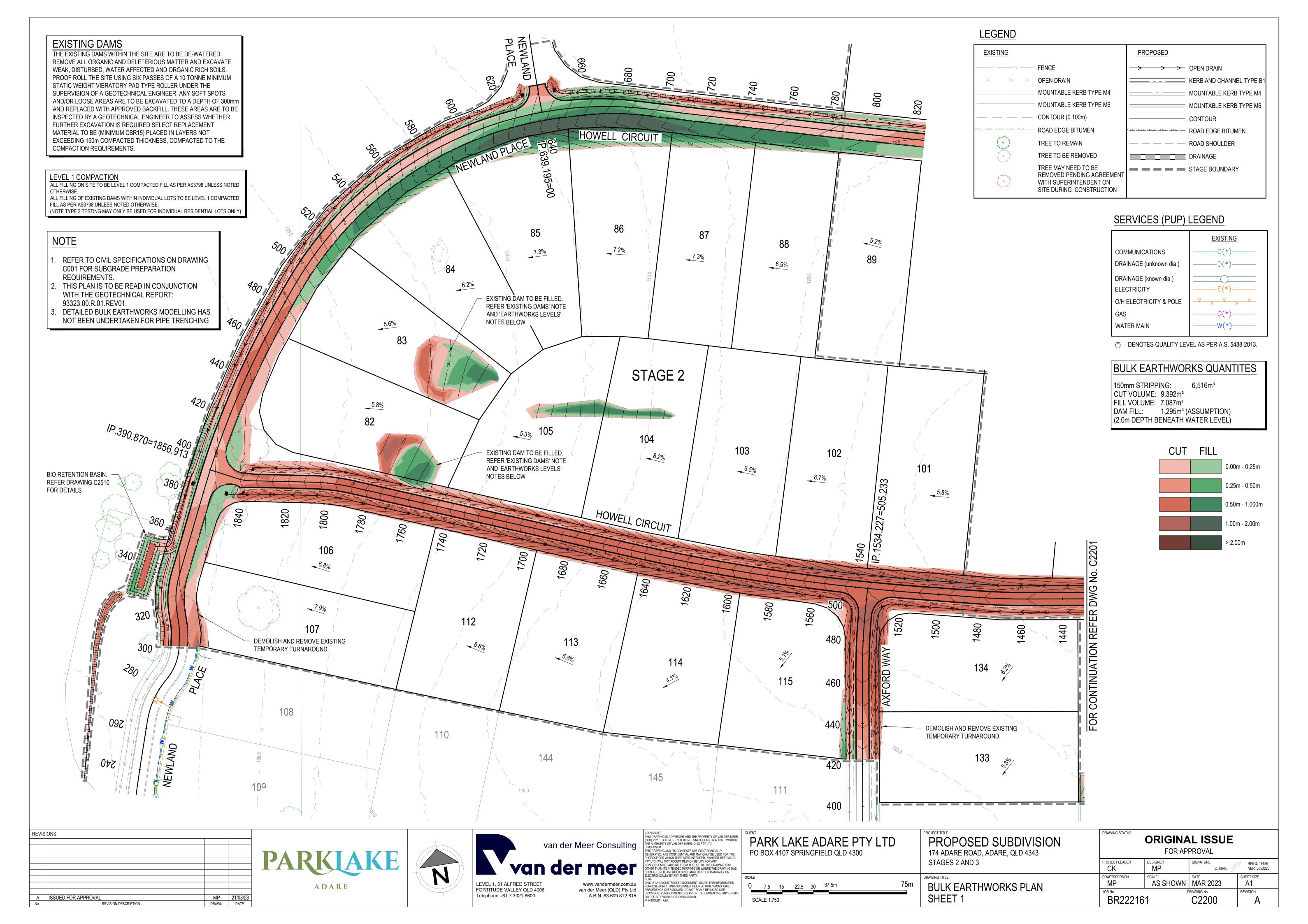
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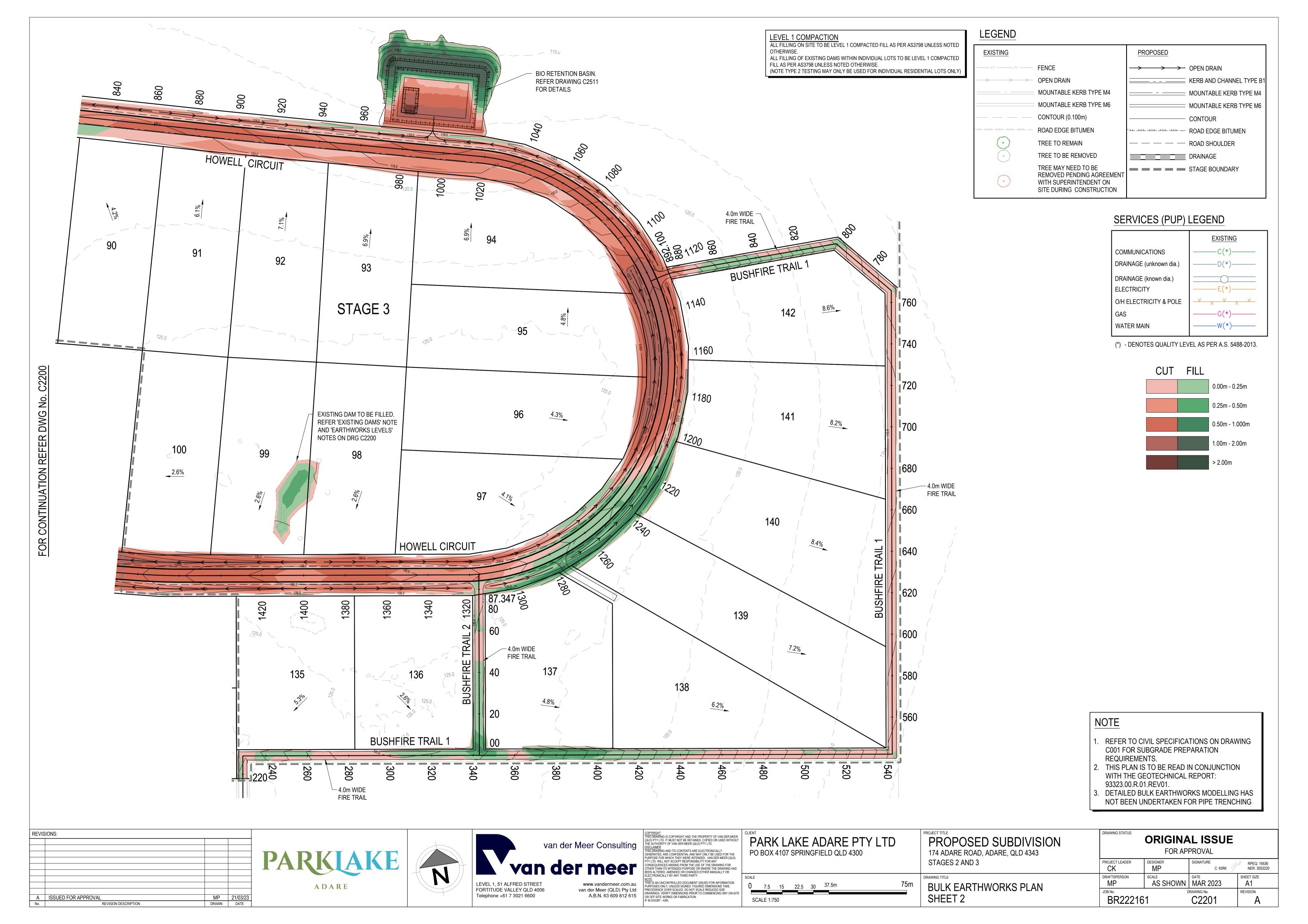
PRECEDENCE OVER SCALED. DO NOT SCALE REDUCED SIZE
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IF IN DOUBT - ASK.

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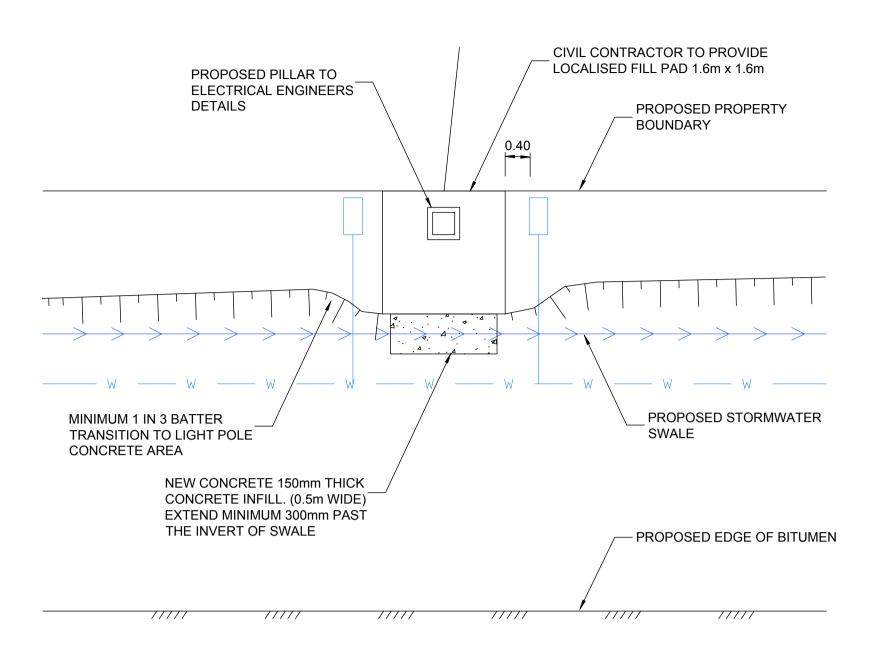
PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343 STAGES 2 AND 3

ORIGINAL ISSUE FOR APPROVAL RPEQ: 19536 C KIRK NER: 3053220 DRAFTSPERSON SHEET SIZE MP AS SHOWN | MAR 2023 Α1 EROSION AND SEDIMENT **CONTROL NOTES** BR222161 C2191

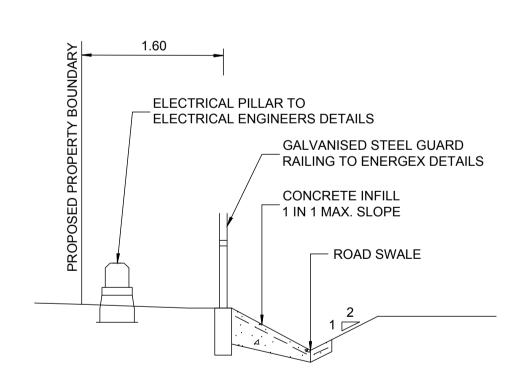




NOTE:
PMT SITE EARTHWORKS TO BE CONFIRMED WITH THE SUPERINTENDENT
PRIOR TO CONSTRUCTION AND BE IN CONSIDERATION OF ENERGEX
REQUIREMENTS FOR MAX CUT/FILL ACROSS PMT SITE



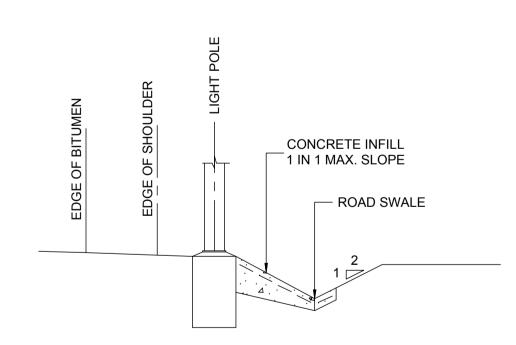
TYPICAL ELECTRICAL PILLAR PROTECTION DETAIL
SCALE 1:50



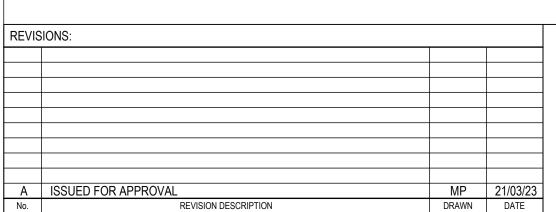
B TYPICAL ELECTRICAL PILLAR SECTION SCALE 1:50

PMT SITE EARTHWORKS TO BE CONFIRMED WITH THE SUPERINTENDENT PRIOR TO CONSTRUCTION AND BE IN CONSIDERATION OF ENERGEX REQUIREMENTS FOR MAX CUT/FILL ACROSS PMT SITE – PROPOSED STORMWATER MAXIMUM 1 IN 2 BATTER TRANSITION TO LIGHT POLE CONCRETE AREA CIVIL CONTRACTOR TO PROVIDE CONCRETE INFILL LOCALISED FILL PAD AT LIGHT POLE 150mm THICK. EXTEND 300mm LOCATIONS. GRADE OF PAD IS TO MIN AT THE INVERT OF SWALE. MATCH THE ROAD SHOULDER PROFILE. PROPOSED LIGHT POLE. REFER ELECTRICAL ENGINEERS DRAWINGS FOR DETAILS

TYPICAL LIGHT POLE PROTECTION DETAIL
SCALE 1:50



TYPICAL LIGHT POLE SECTION A
SCALE 1:50







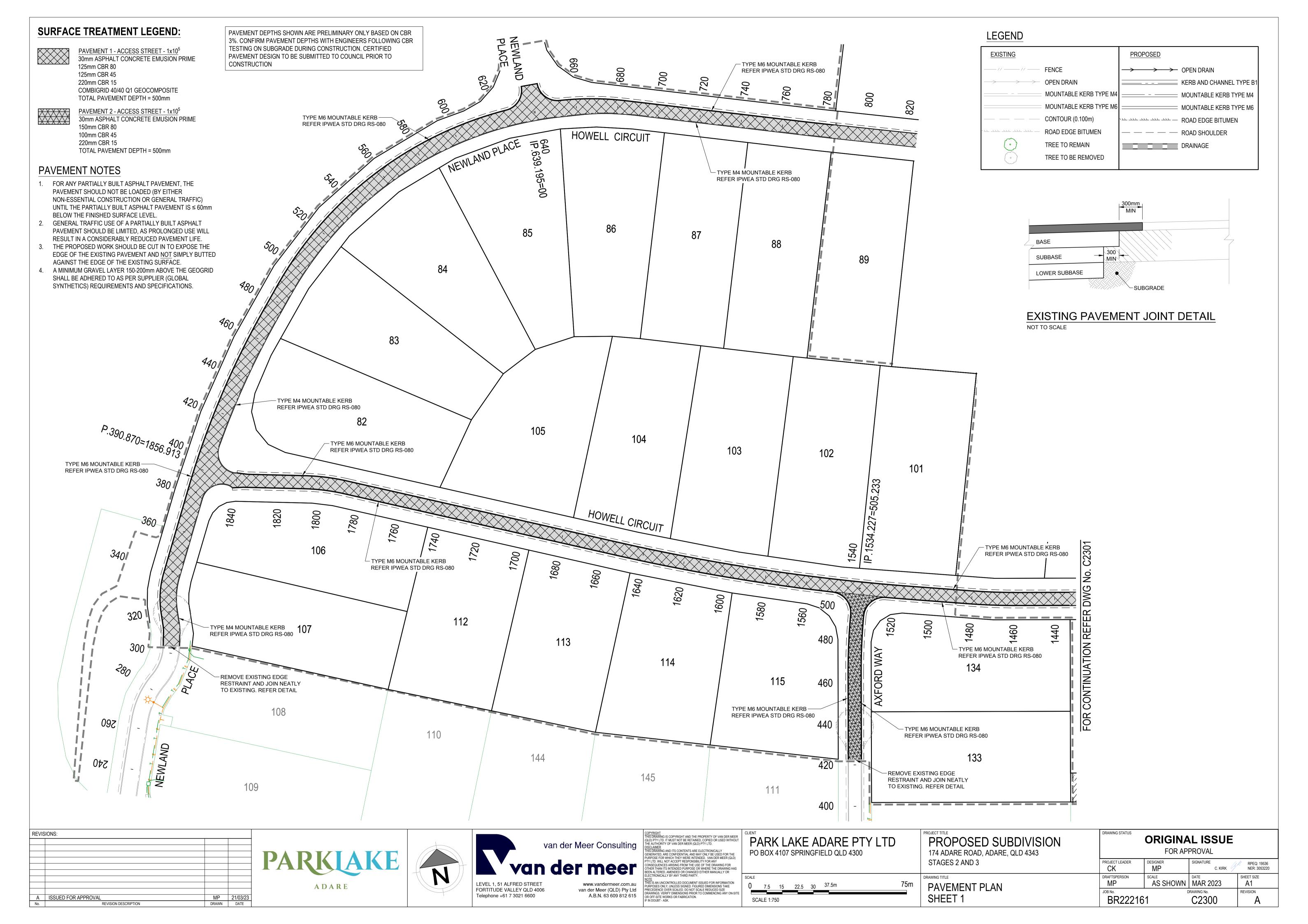
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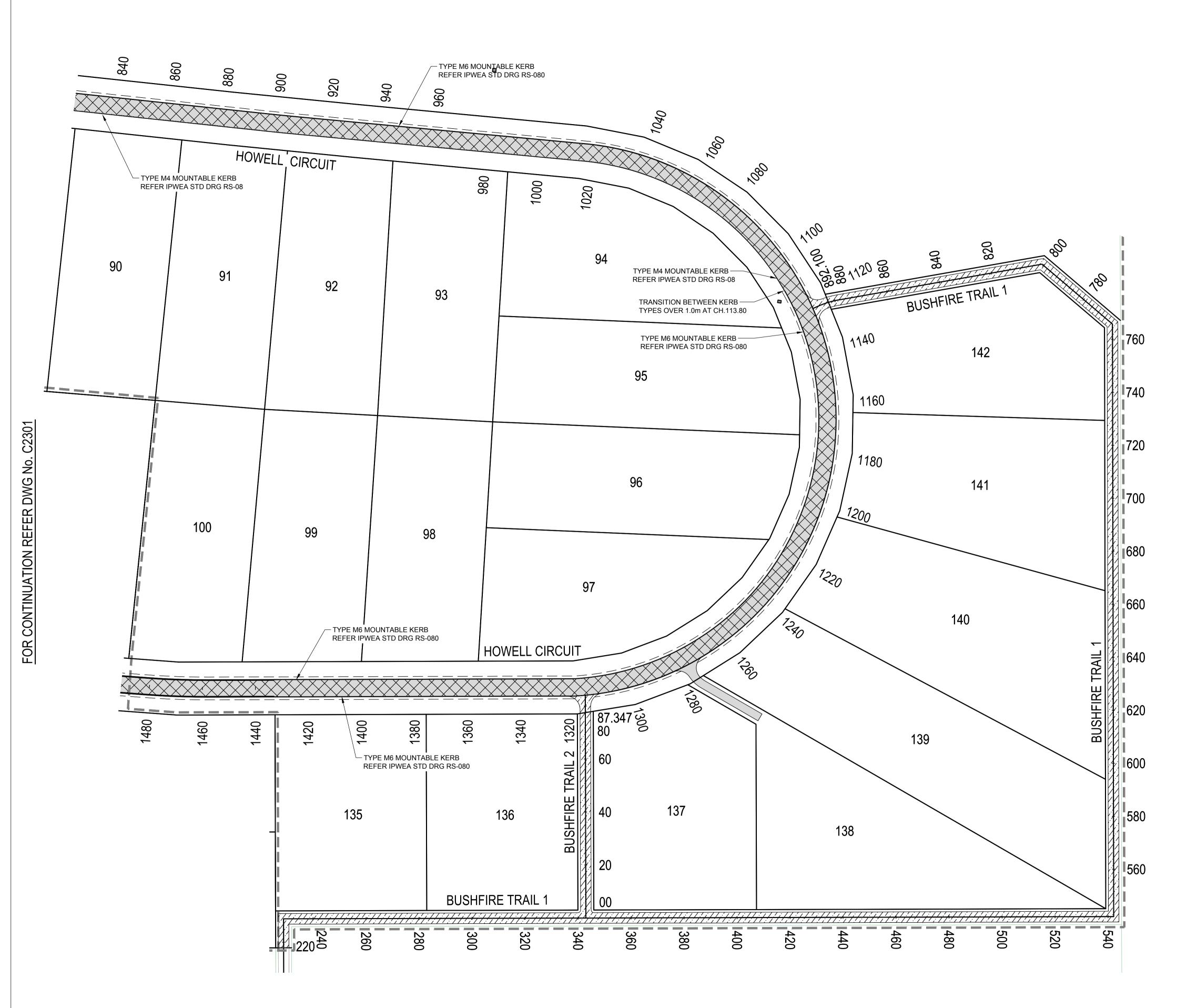
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SCALE 1:50

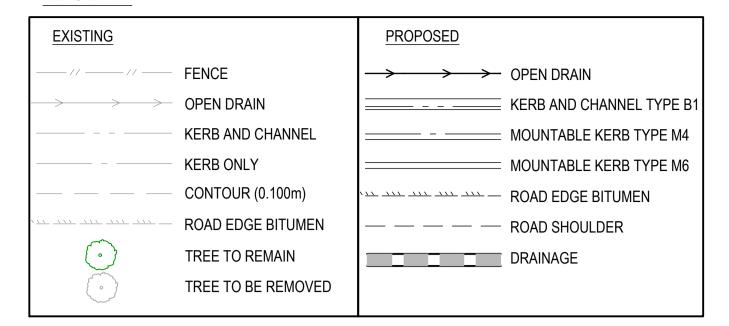
PROPOSED SUBDIVISION  174 ADARE ROAD, ADARE, QLD 4343  STAGES 2 AND 3
DDAWING TITLE
BULK EARTHWORKS DETAILS

DRAWING STATUS		PORIGINAL ISSUE FOR APPROVAL			
PROJECT LEADER  CK	DESIGNER MP	SIGNATURE	C. KIRK	i K	RPEQ: 19536 NER: 3053220
DRAFTSPERSON MP	AS SHOWN	MAR 20	023		EET SIZE <b>A1</b>
JOB No. BR22216	=	C229	0	RE	VISION





#### LEGEND



#### **SURFACE TREATMENT LEGEND:**



PAVEMENT 1 - ACCESS STREET - 1x10<sup>5</sup> 30mm ASPHALT CONCRETE PRIMER SEAL 100mm CBR 80

150mm CBR 45 150mm CBR 15

TOTAL PAVEMENT DEPTH = 430mm



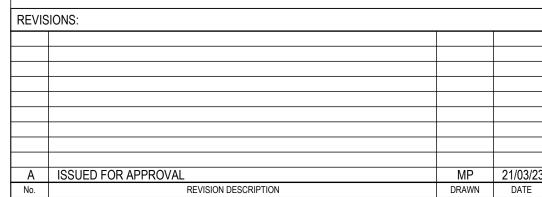
BUSHFIRE TRAIL REFER DRG'S C2700-C2708

DRIVEWAY REFER IPWEA STD DRG RS-056

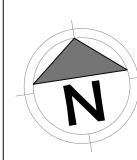
#### PAVEMENT NOTES

- FOR ANY PARTIALLY BUILT ASPHALT PAVEMENT, THE PAVEMENT SHOULD NOT BE LOADED (BY EITHER NON-ESSENTIAL CONSTRUCTION OR GENERAL TRAFFIC) UNTIL THE PARTIALLY BUILT ASPHALT PAVEMENT IS ≤ 60mm BELOW THE FINISHED SURFACE LEVEL.
- 2. GENERAL TRAFFIC USE OF A PARTIALLY BUILT ASPHALT PAVEMENT SHOULD BE LIMITED, AS PROLONGED USE WILL RESULT IN A CONSIDERABLY REDUCED PAVEMENT LIFE.
- 3. THE PROPOSED WORK SHOULD BE CUT IN TO EXPOSE THE EDGE OF THE EXISTING PAVEMENT AND NOT SIMPLY BUTTED AGAINST THE EDGE OF THE EXISTING SURFACE.

PAVEMENT DEPTHS SHOWN ARE PRELIMINARY ONLY BASED ON CBR 3%. CONFIRM PAVEMENT DEPTHS WITH ENGINEERS FOLLOWING CBR TESTING ON SUBGRADE DURING CONSTRUCTION. CERTIFIED PAVEMENT DESIGN TO BE SUBMITTED TO COUNCIL PRIOR TO CONSTRUCTION











R JT	PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300							
3	SCALE							
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IE	SC	ALE 1:7	50					

PROPOSED SUBDIVISION

174 ADARE ROAD, ADARE, QLD 4343

STAGES 2 AND 3

PAVEMENT PLAN

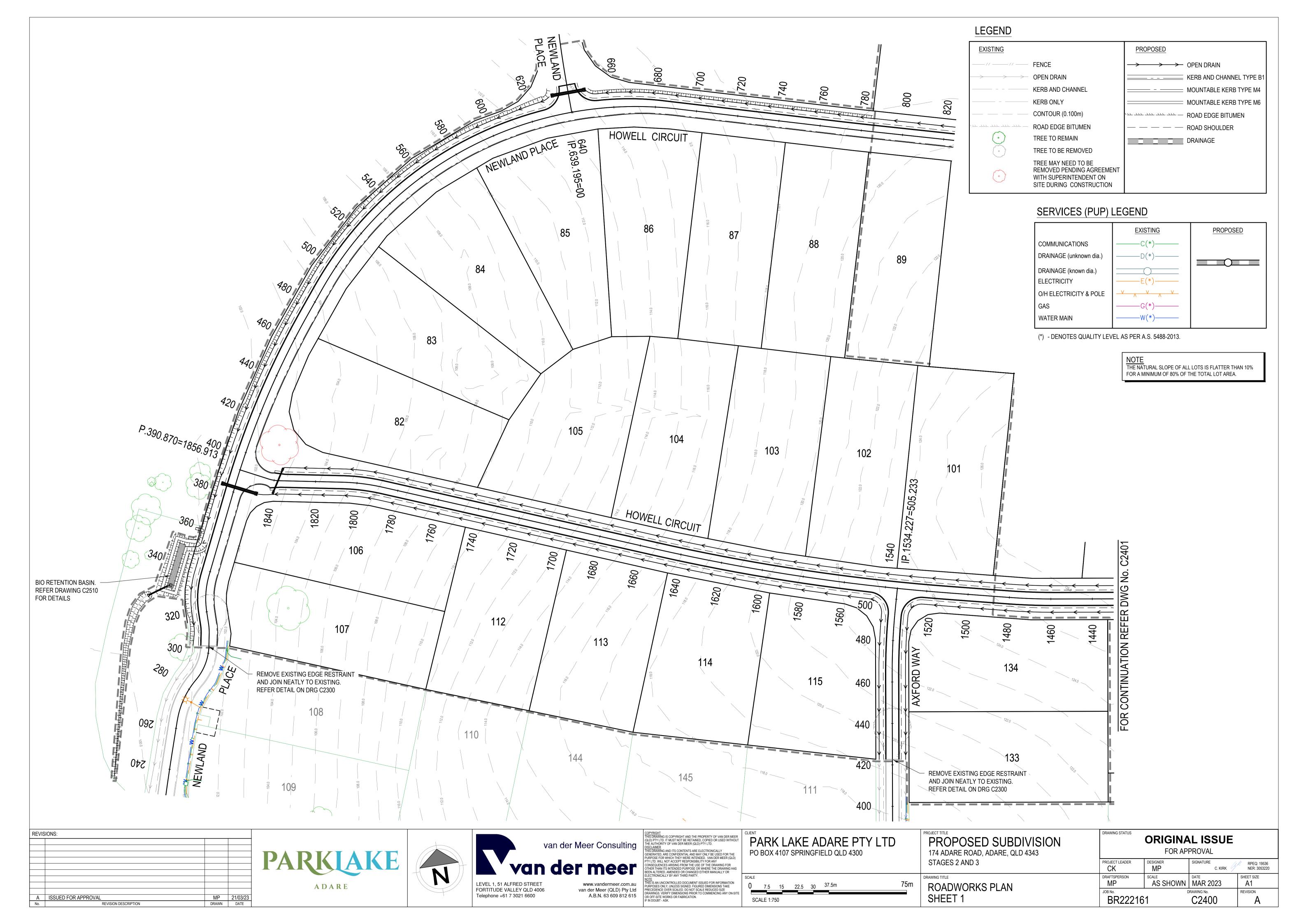
SHEET 2

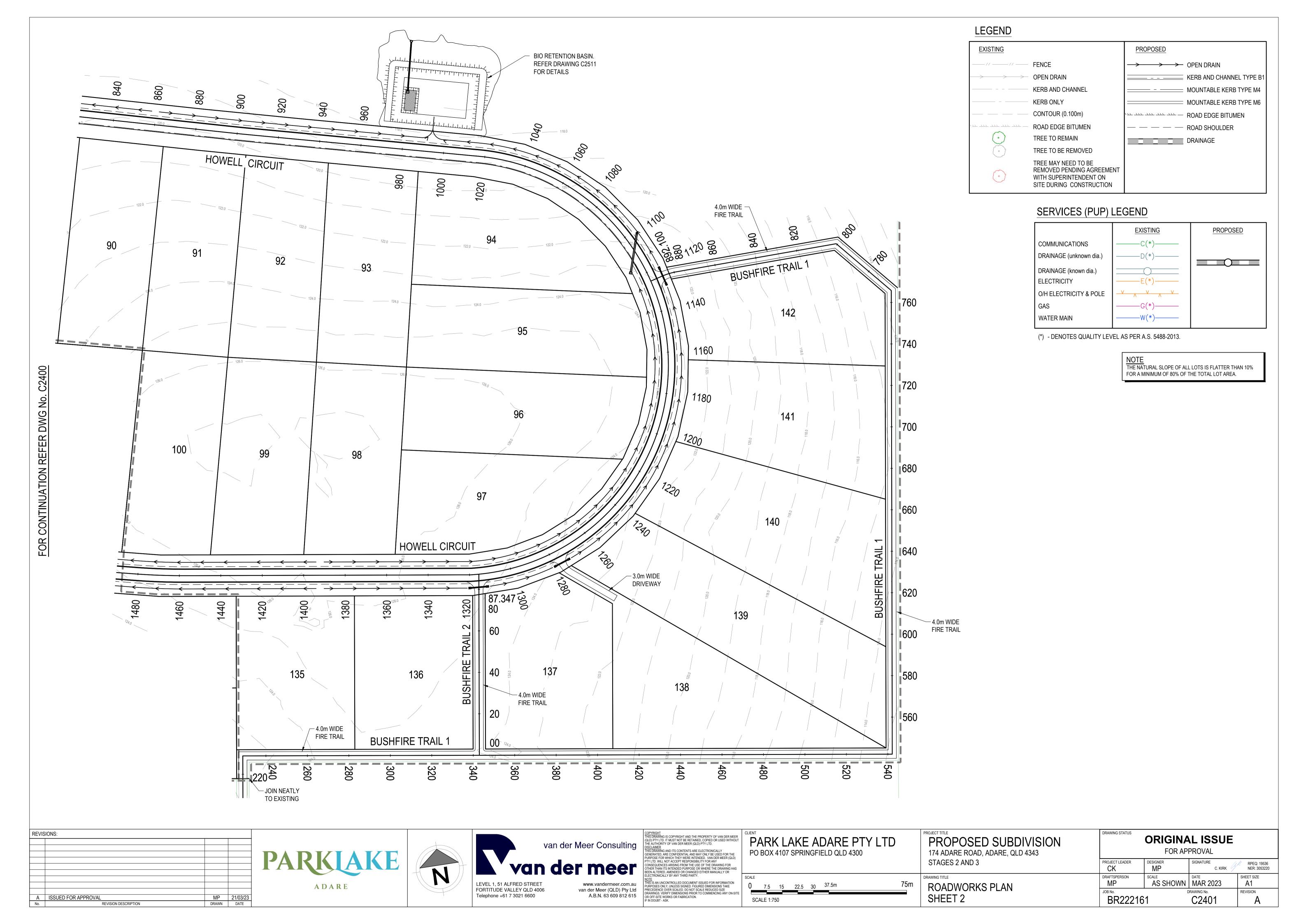
ORIGINAL ISSUE
FOR APPROVAL

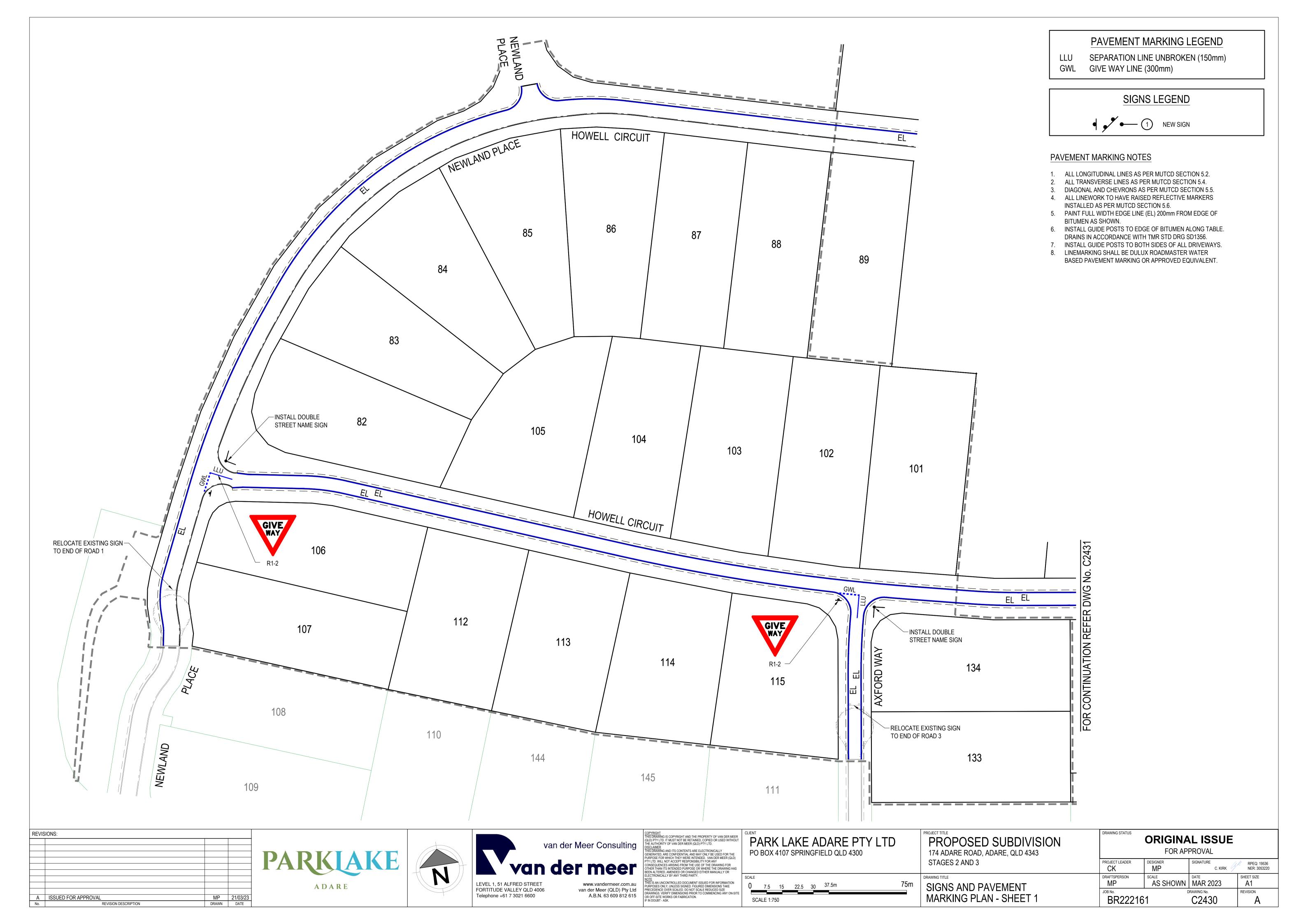
PROJECT LEADER CK MP SIGNATURE C. KIRK RPEQ: 19536 NER: 3053220

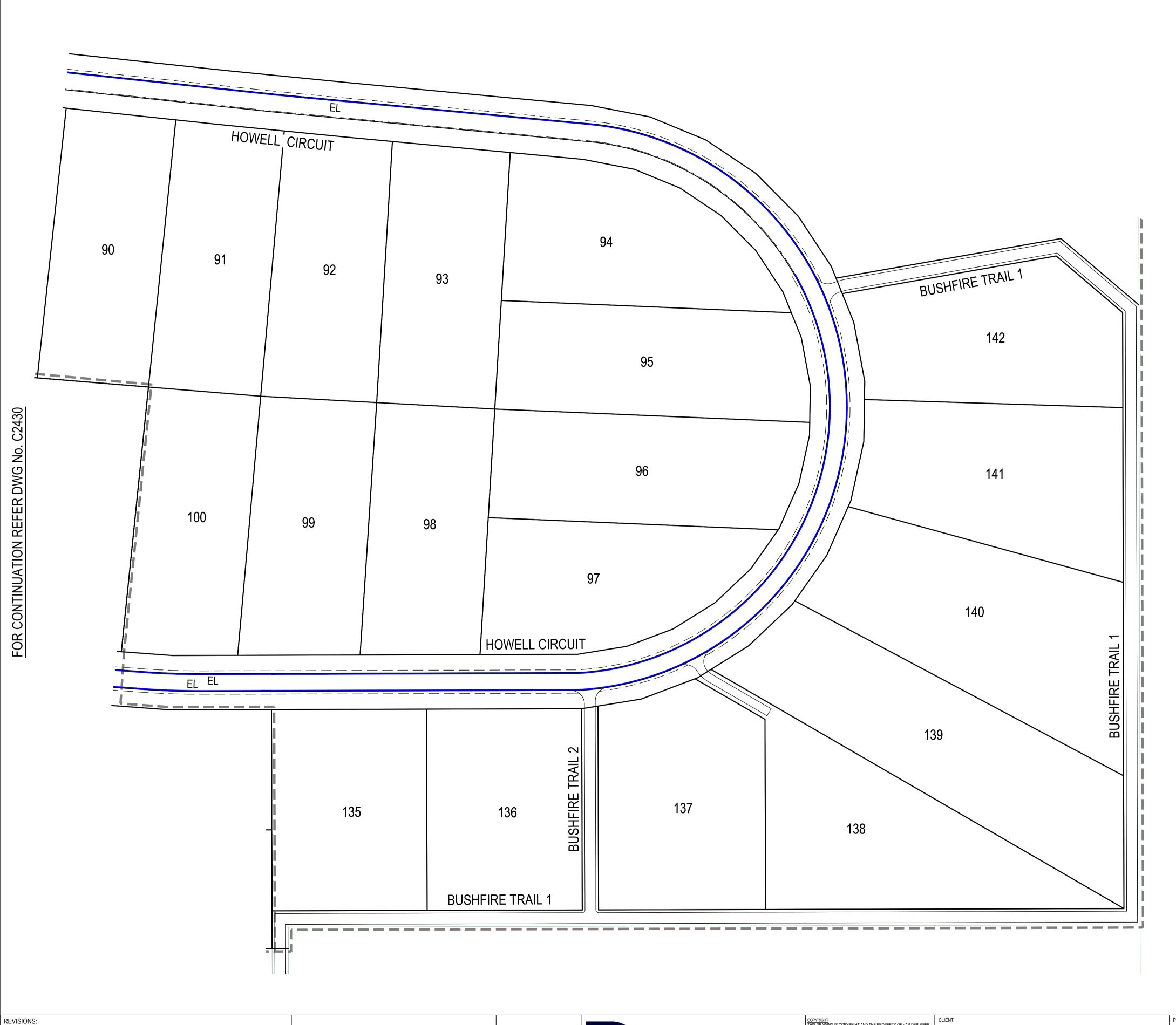
DRAFTSPERSON SCALE DATE SHEET SIZE A1

JOB No. DRAWING No. REVISION
BR222161 C2301 A







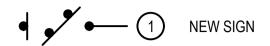


PAVEMENT MARKING LEGEND

SEPARATION LINE UNBROKEN (150mm)

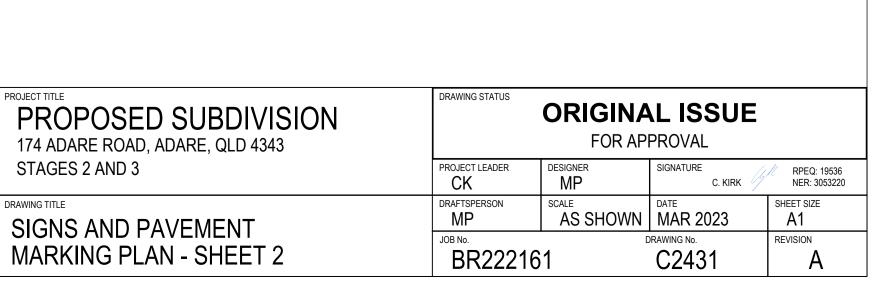
GWL GIVE WAY LINE (300mm)

SIGNS LEGEND



#### PAVEMENT MARKING NOTES

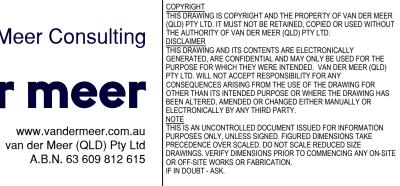
- 1. ALL LONGITUDINAL LINES AS PER MUTCD SECTION 5.2.
- ALL TRANSVERSE LINES AS PER MUTCD SECTION 5.4.
- DIAGONAL AND CHEVRONS AS PER MUTCD SECTION 5.5. ALL LINEWORK TO HAVE RAISED REFLECTIVE MARKERS
- INSTALLED AS PER MUTCD SECTION 5.6.
- 5. PAINT FULL WIDTH EDGE LINE (EL) 200mm FROM EDGE OF
- BITUMEN AS SHOWN.
- 6. INSTALL GUIDE POSTS TO EDGE OF BITUMEN ALONG TABLE.
- DRAINS IN ACCORDANCE WITH TMR STD DRG SD1356.
- 7. INSTALL GUIDE POSTS TO BOTH SIDES OF ALL DRIVEWAYS.
- 8. LINEMARKING SHALL BE DULUX ROADMASTER WATER
- BASED PAVEMENT MARKING OR APPROVED EQUIVALENT.

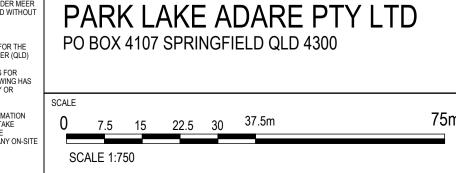


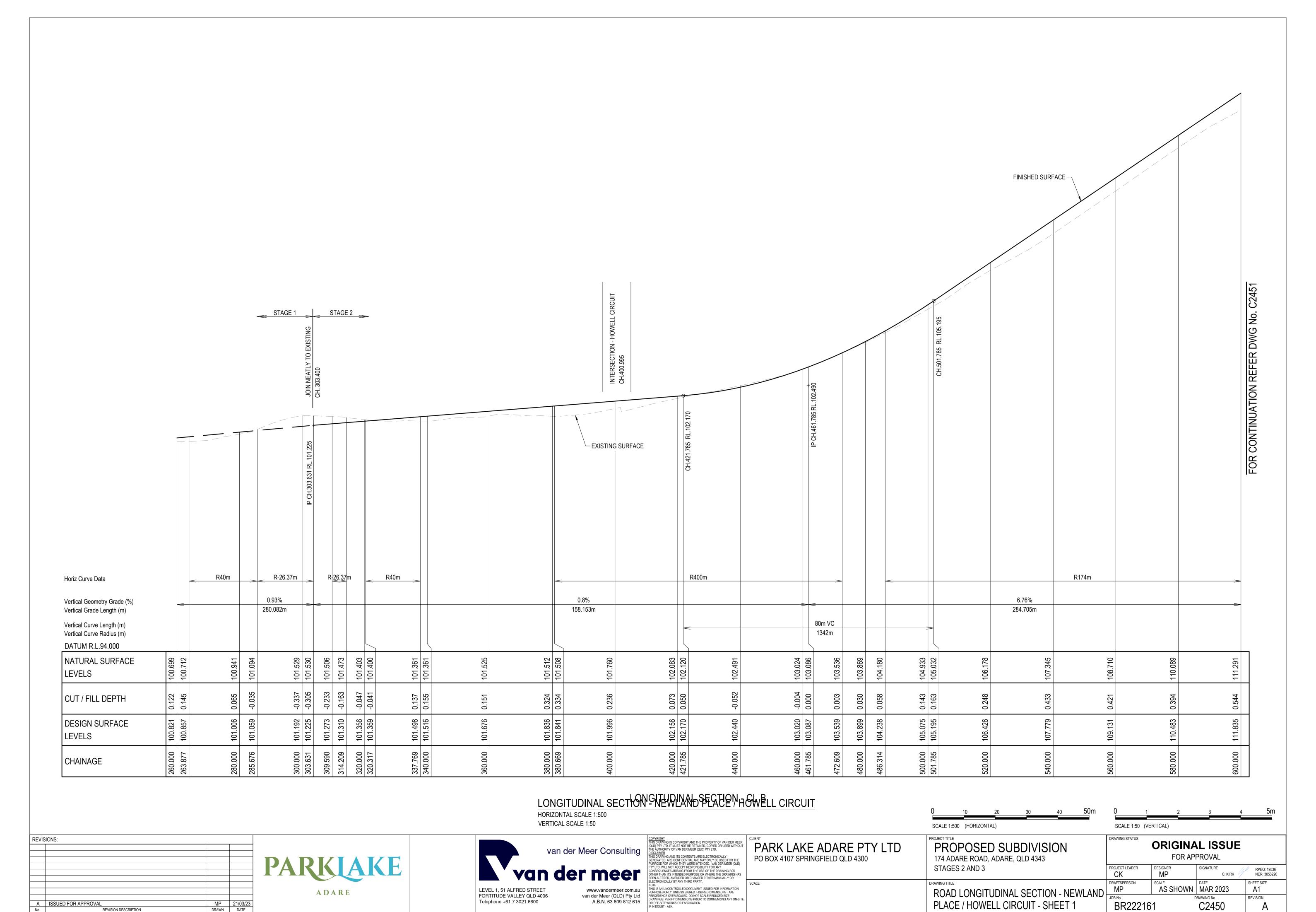


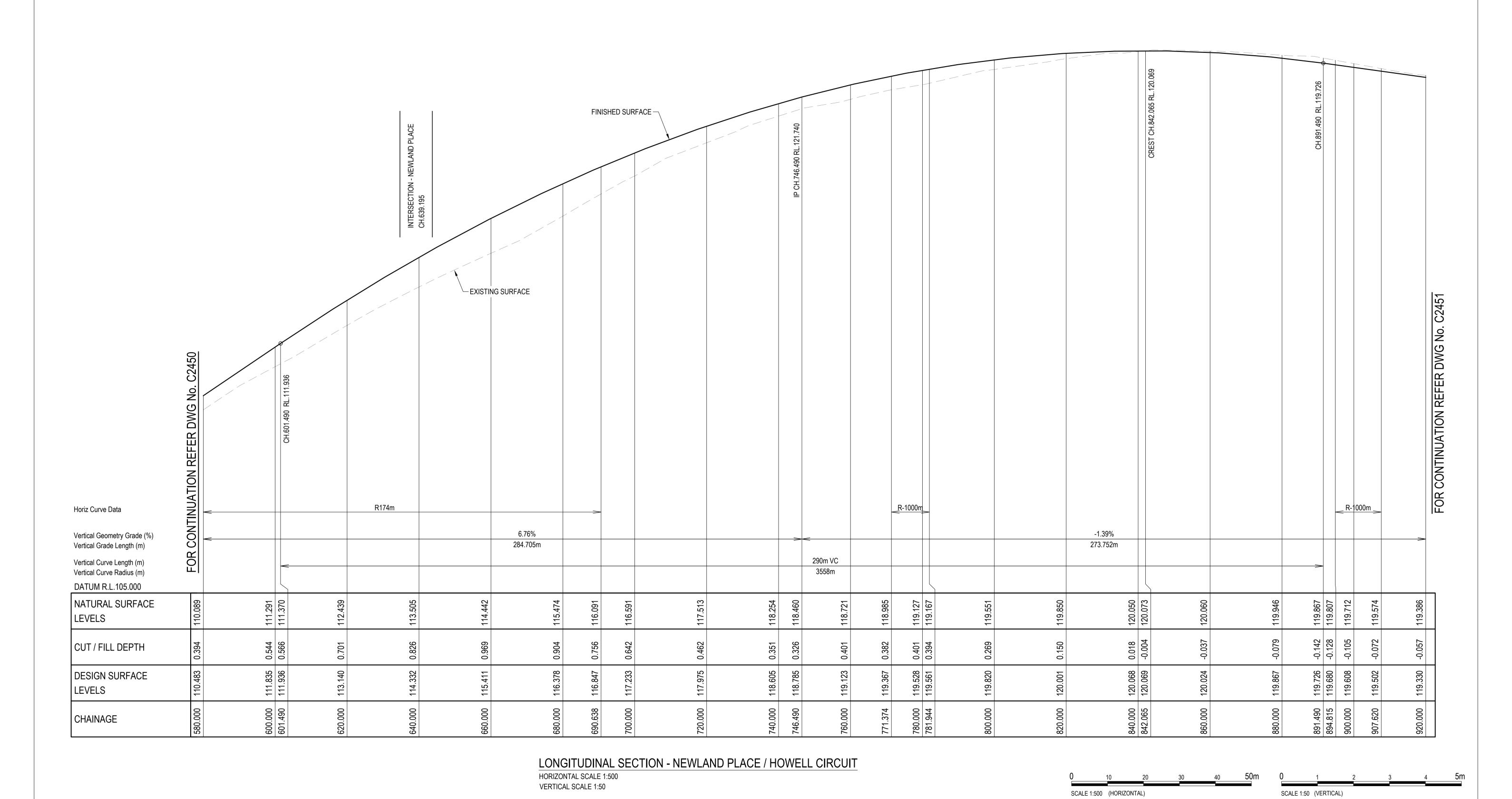
REVISION DESCRIPTION











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van der Meer Consulting

van der Meer (QLD) Pty Ltd

FORTITUDE VALLEY QLD 4006

Telephone +61 7 3021 6600

ADARE

PARK LAKE ADARE PTY LTD

PO BOX 4107 SPRINGFIELD QLD 4300

DRAWING STATUS

DRAFTSPERSON

BR222161

PROPOSED SUBDIVISION

PLACE / HOWELL CIRCUIT - SHEET 2

ROAD LONGITUDINAL SECTION - NEWLAND

174 ADARE ROAD, ADARE, QLD 4343

STAGES 2 AND 3

**ORIGINAL ISSUE** 

FOR APPROVAL

AS SHOWN | MAR 2023

C2451

C. KIRK

RPEQ: 19536

NER: 3053220

SHEET SIZE

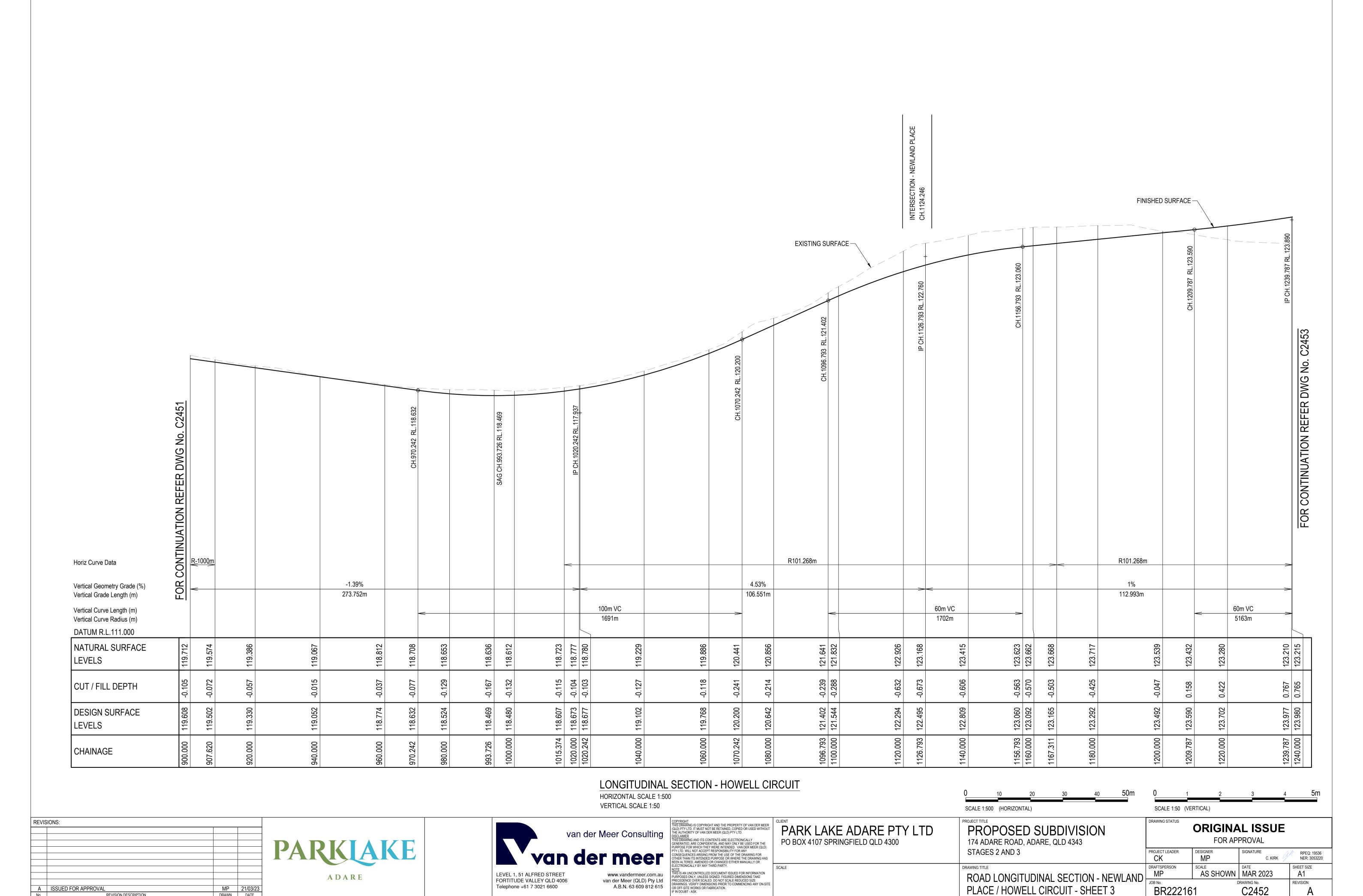
A1

REVISION

DESIGNER MP

REVISIONS:

REVISION DESCRIPTION



Telephone +61 7 3021 6600

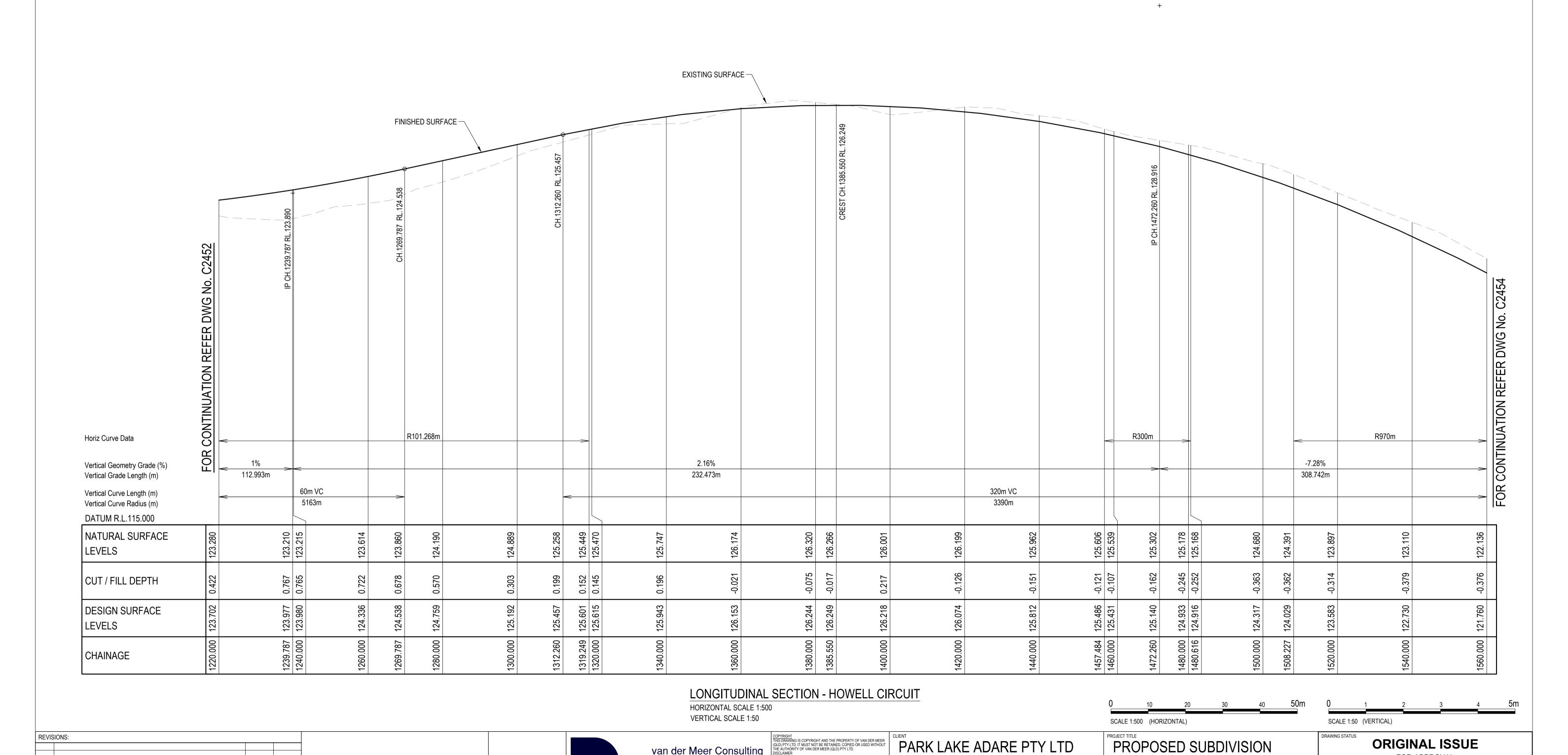
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PLACE / HOWELL CIRCUIT - SHEET 3

BR222161

C2452



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ADARE

REVISION DESCRIPTION

PARK LAKE ADARE PTY LTD

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ROAD LONGITUDINAL SECTION - NEWLAND

PLACE / HOWELL CIRCUIT - SHEET 4

STAGES 2 AND 3

PO BOX 4107 SPRINGFIELD QLD 4300

FOR APPROVAL

AS SHOWN | MAR 2023

C2453

C. KIRK

MP

DRAFTSPERSON

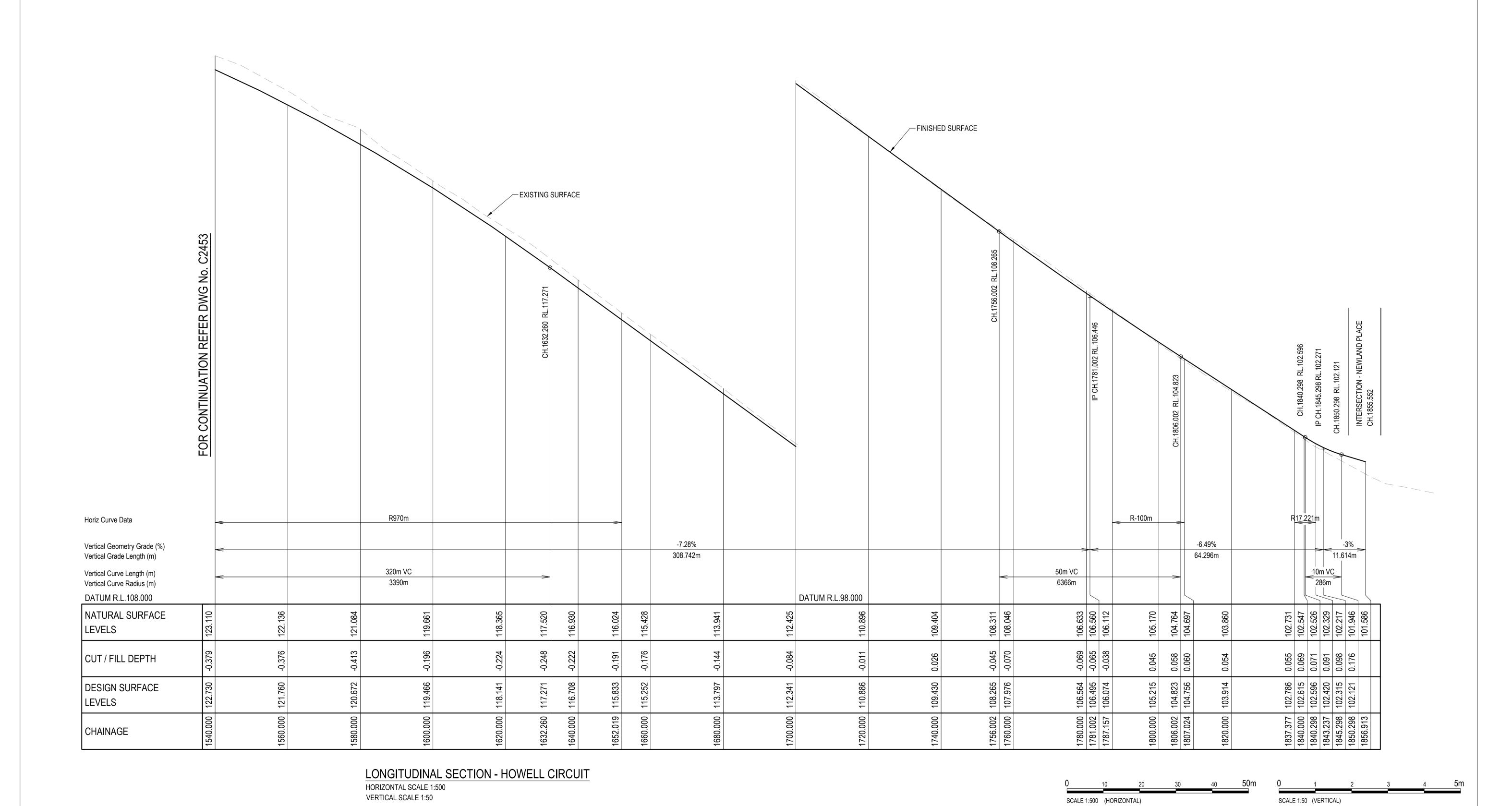
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van der Meer Consulting

FORTITUDE VALLEY QLD 4006

Telephone +61 7 3021 6600

ADARE

PARK LAKE ADARE PTY LTD

PO BOX 4107 SPRINGFIELD QLD 4300

DRAWING STATUS

DRAFTSPERSON

BR222161

PROPOSED SUBDIVISION

PLACE / HOWELL CIRCUIT - SHEET 5

ROAD LONGITUDINAL SECTION - NEWLAND

174 ADARE ROAD, ADARE, QLD 4343

STAGES 2 AND 3

**ORIGINAL ISSUE** 

FOR APPROVAL

AS SHOWN | MAR 2023

C2454

C. KIRK

RPEQ: 19536

NER: 3053220

SHEET SIZE

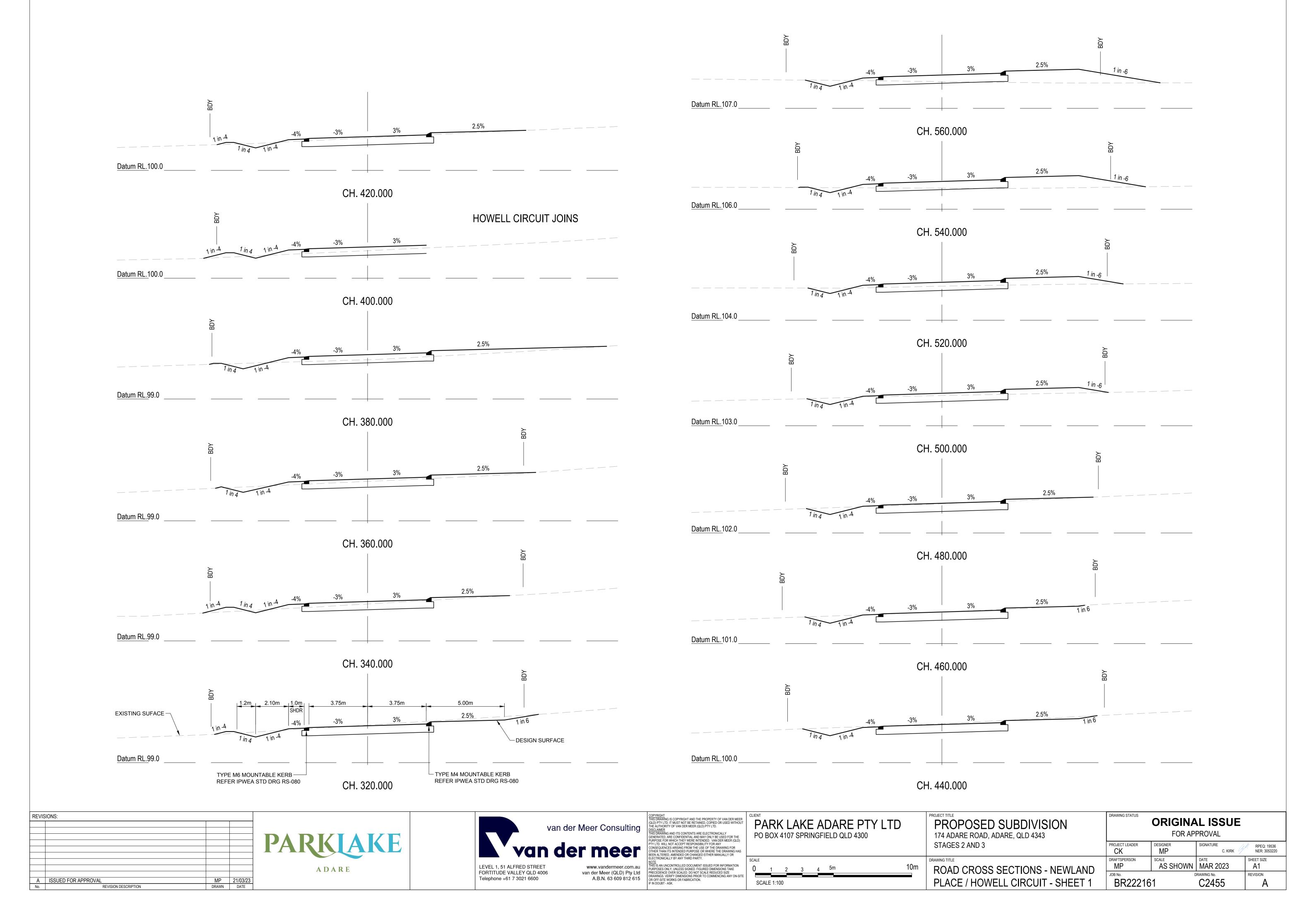
A1

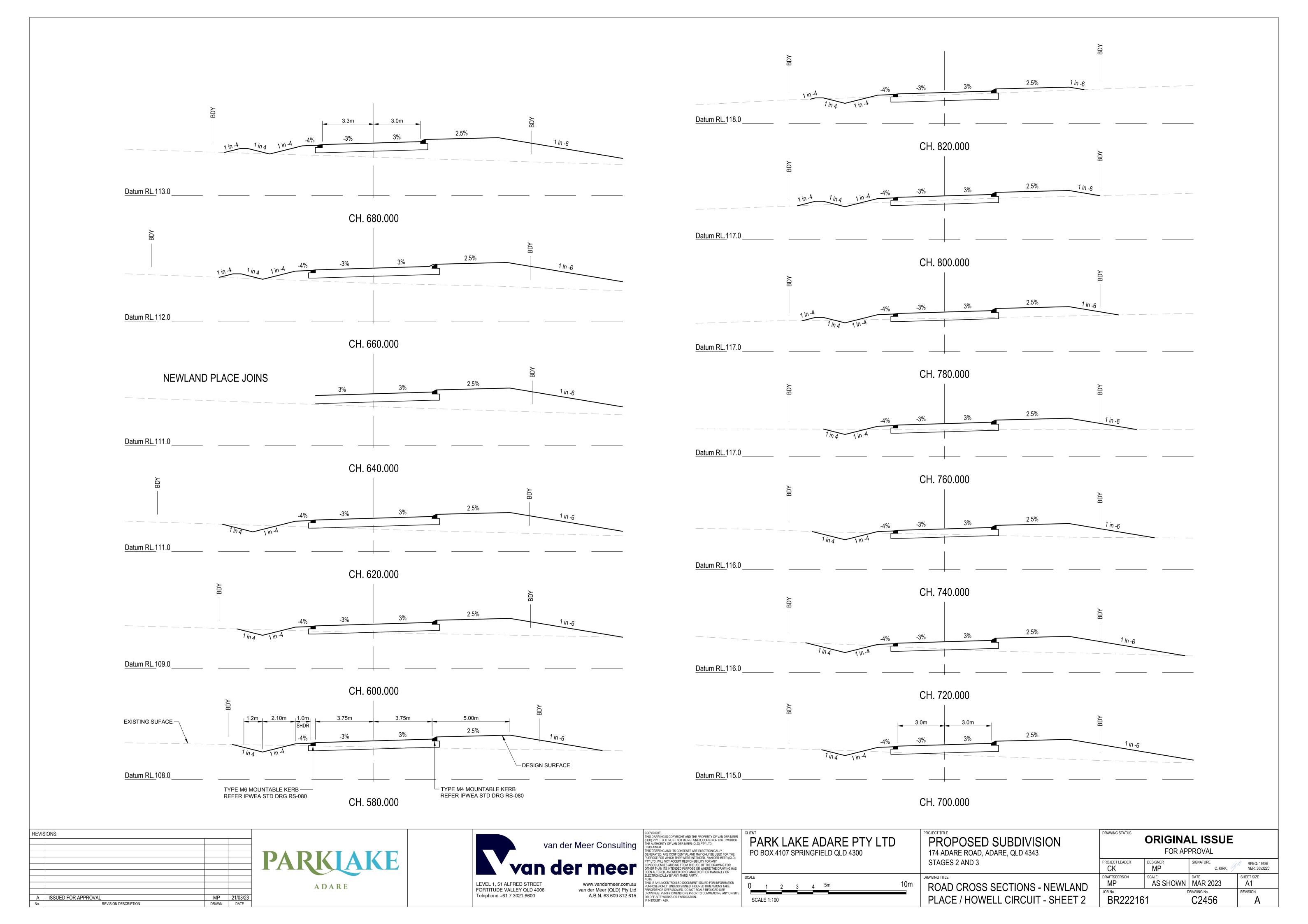
REVISION

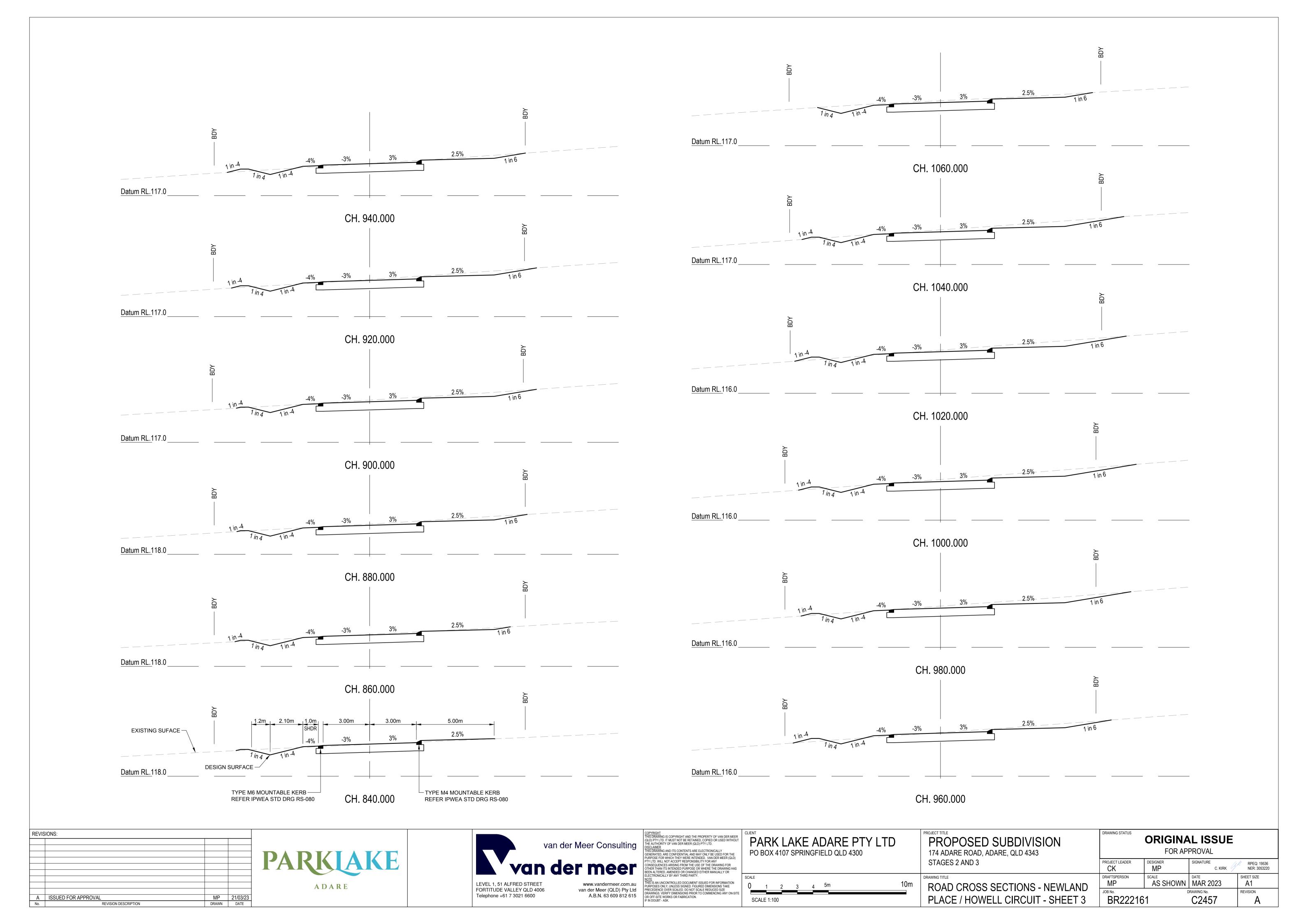
DESIGNER MP

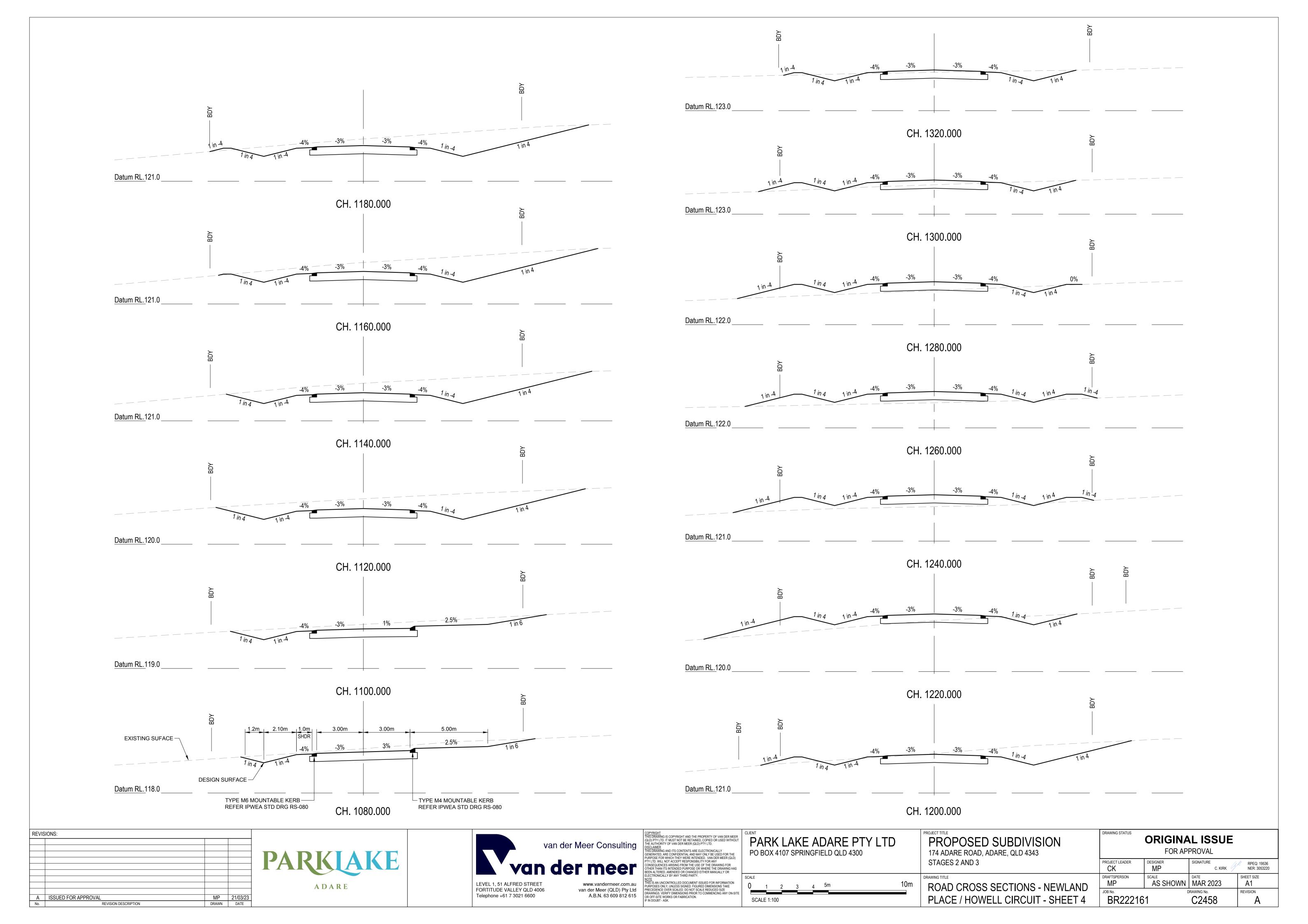
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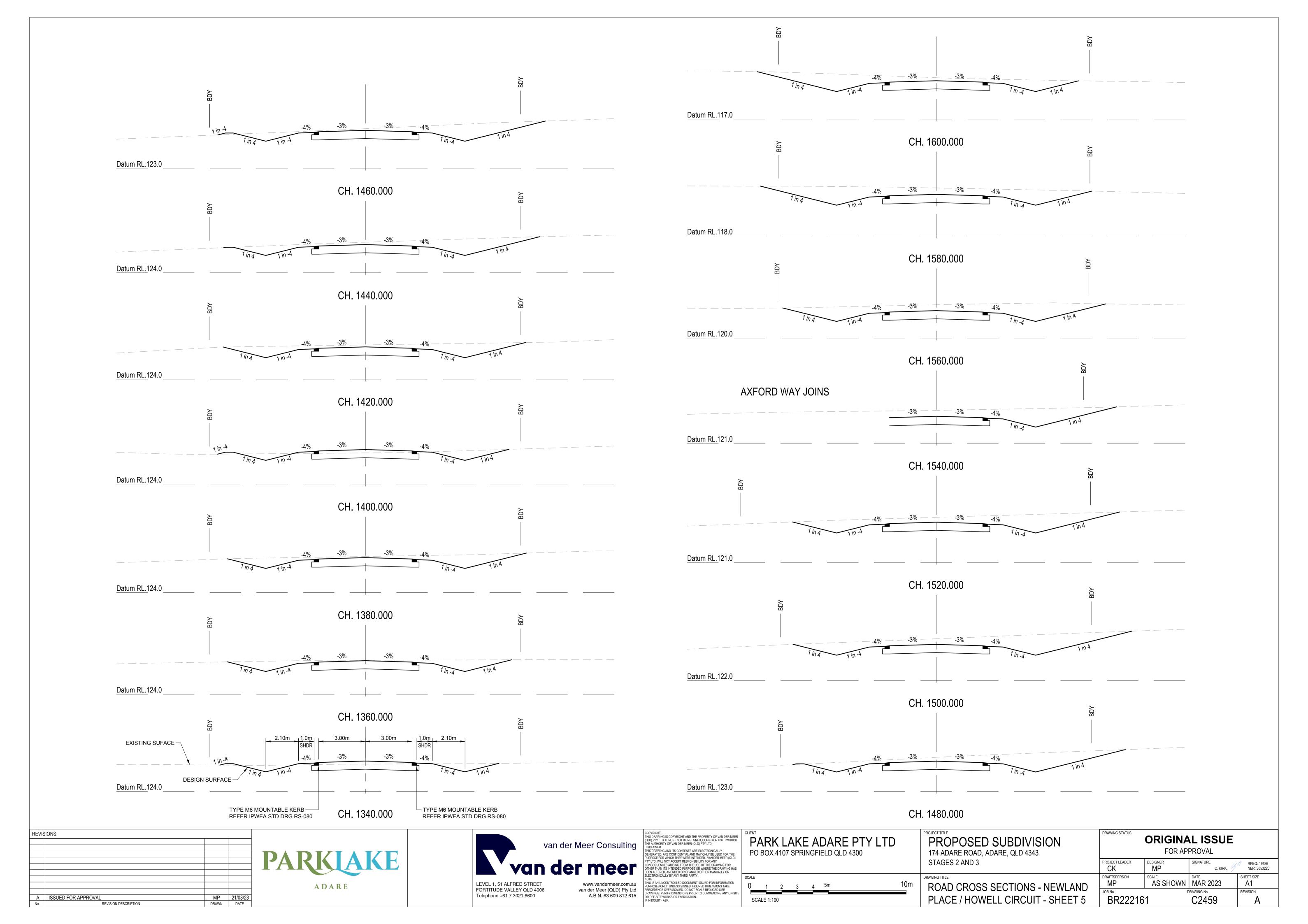
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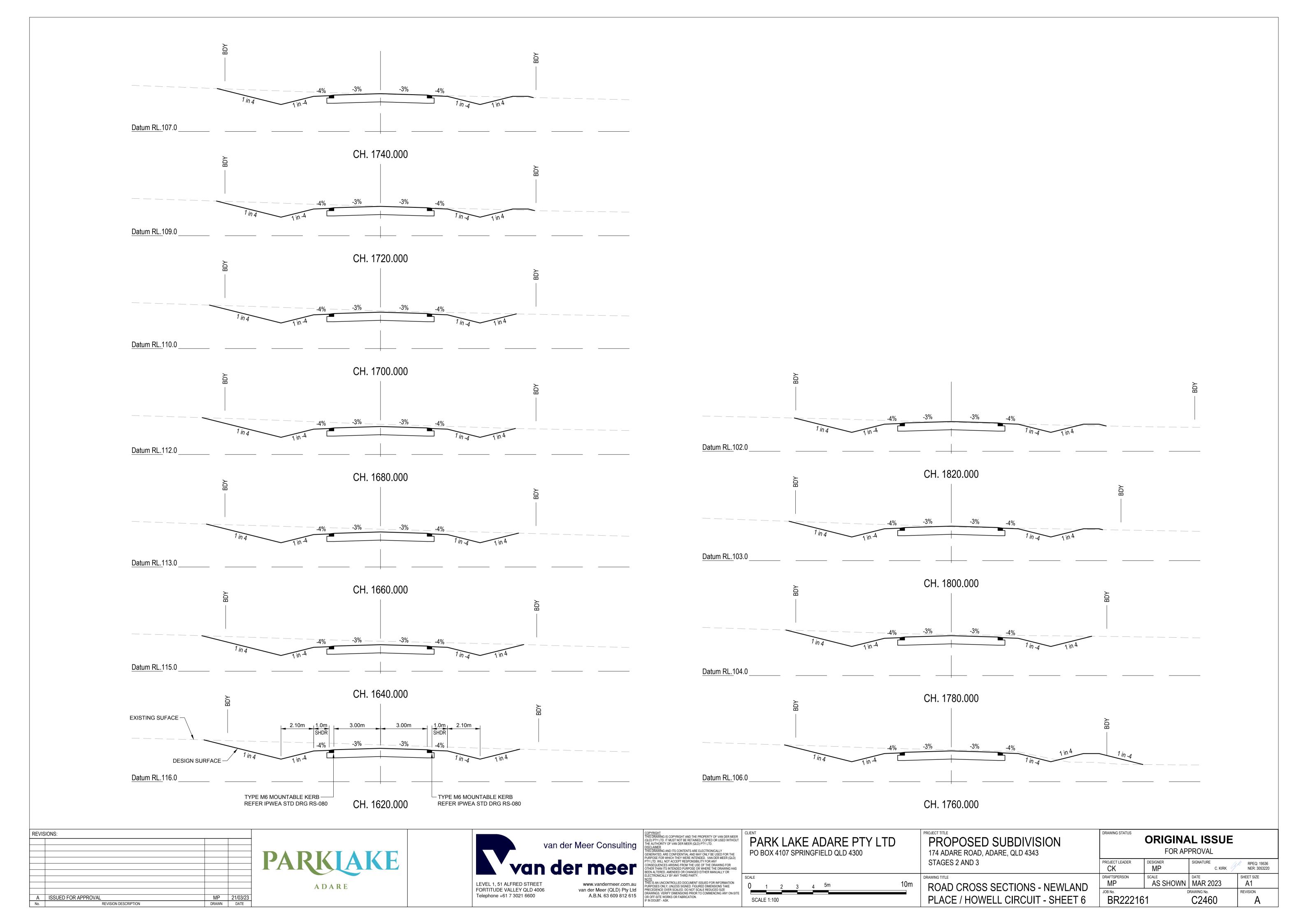


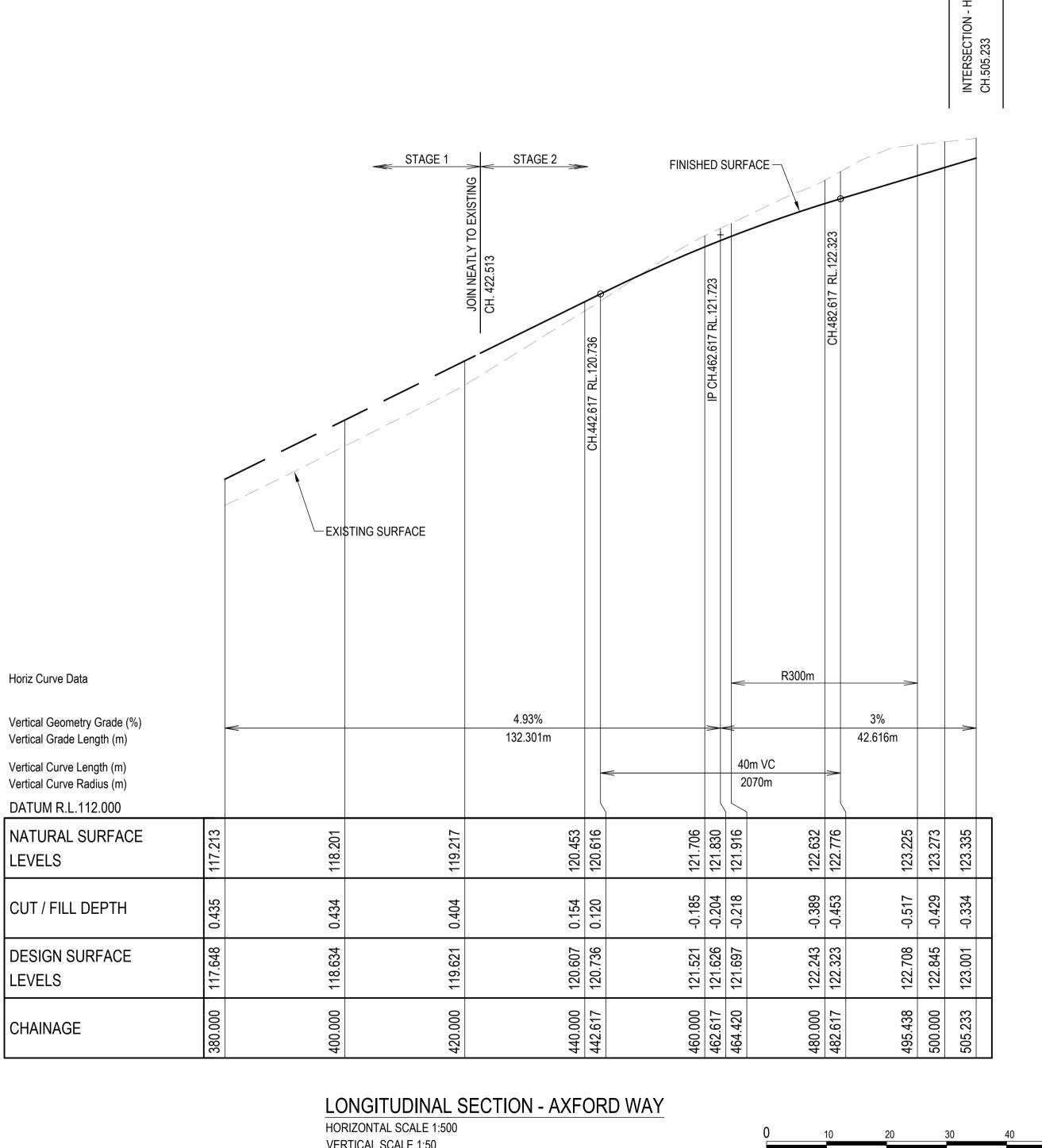


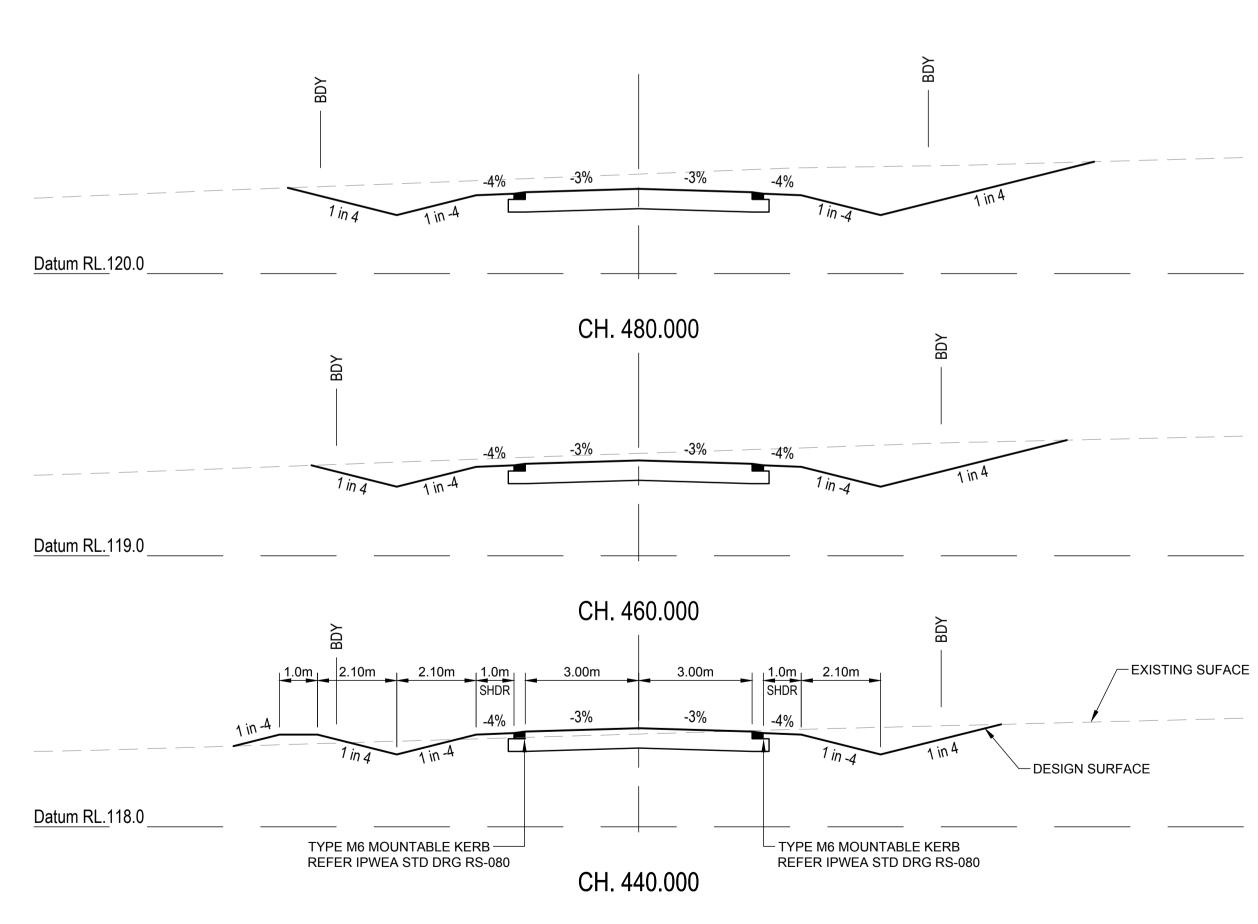




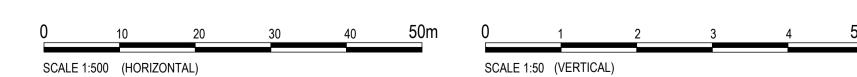








VERTICAL SCALE 1:50



I										
	REVIS	SIONS:								
	A	ISSUED FOR APPROVAL	MP	21/03/23						
ł	No.	REVISION DESCRIPTION	DRAWN	DATE	ł					
- 1	INO.	REVISION DESCRIPTION	DKAWN	DATE	l					



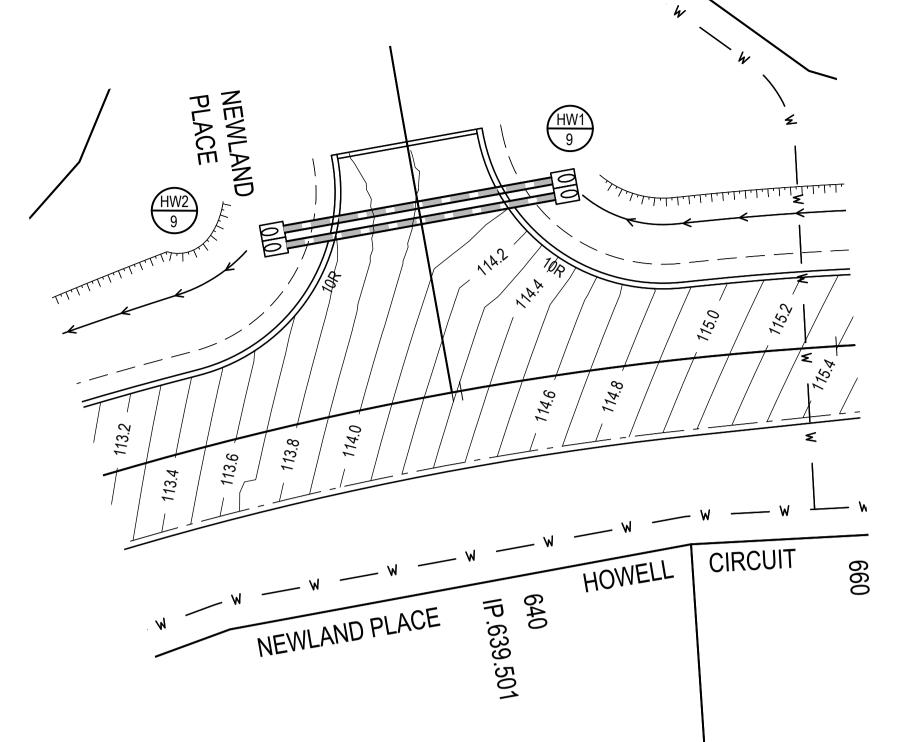


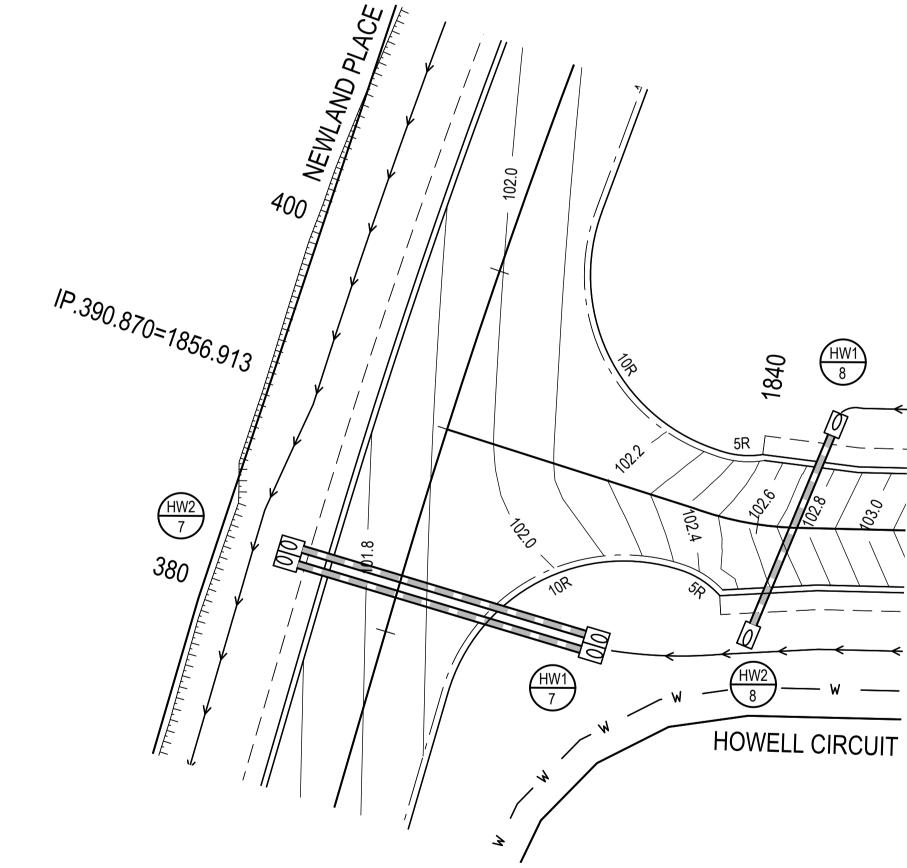
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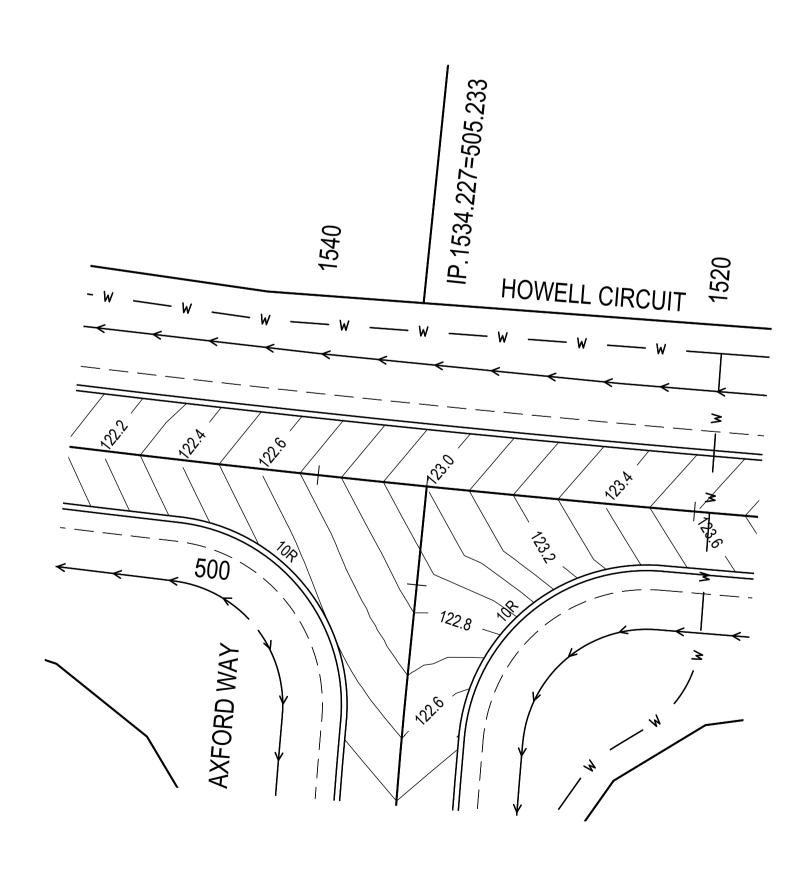
						RE PT LD 4300	Y LTD	
SCALE 0	1	2	3	4	5m			1
SCA	ALE 1:1	00						

PROPOSED SUBDIVISION 74 ADARE ROAD, ADARE, QLD 4343	DRAWING STATUS  ORIG		
STAGES 2 AND 3	PROJECT LEADER  CK	DESIGNER MP	
ROAD LONGITUDINAL AND CROSS	DRAFTSPERSON MP	SCALE AS SH	
SECTIONS - AXFORD WAY	JOB No. BR22216	1	

	DRAWING STATUS	0 0	AL ISSUE PROVAL	
	PROJECT LEADER CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220
	DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1
JOB No. BR2221			C2461	REVISION

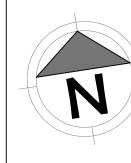




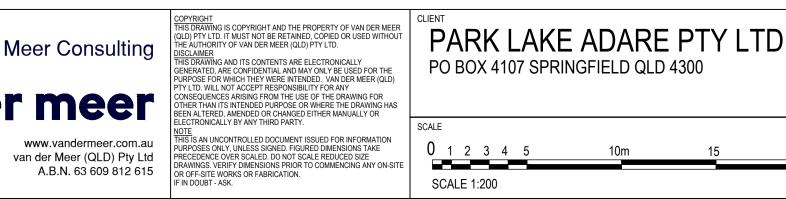


# REVISIONS: REVISION DESCRIPTION









	PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300						ΓD		
	SCALE								
	0	1	2	3	4_	5	10m	15	20
Ε									

SCALE 1:200

PROJECT TITLE	
PROPOSED SUBDIVISION	
174 ADARE ROAD, ADARE, QLD 4343	
STAGES 2 AND 3	

INTERSECTION DETAILS

DRAWING STATUS			
PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220
DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1
JOB No. BR22216	_	C2480	REVISION

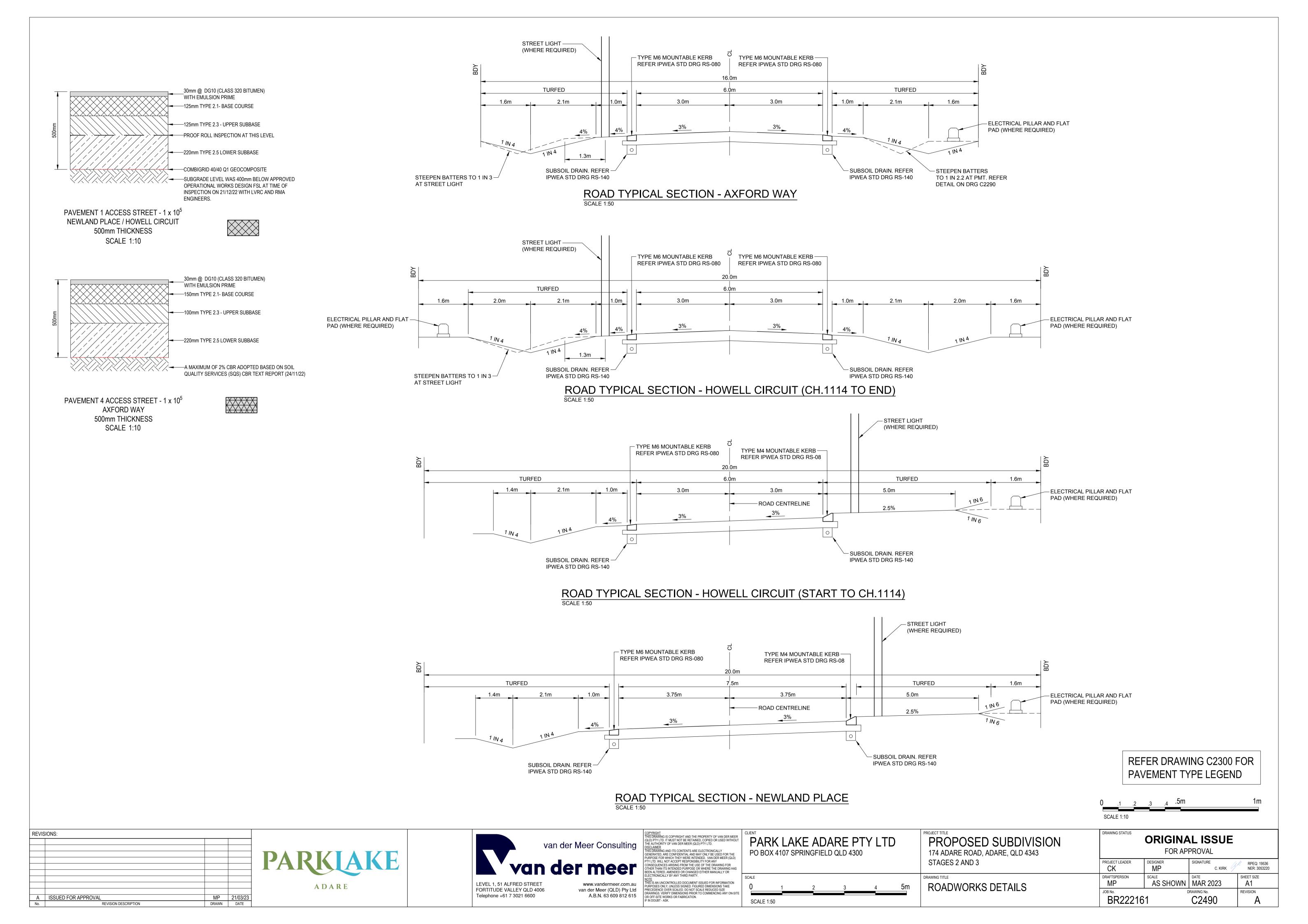
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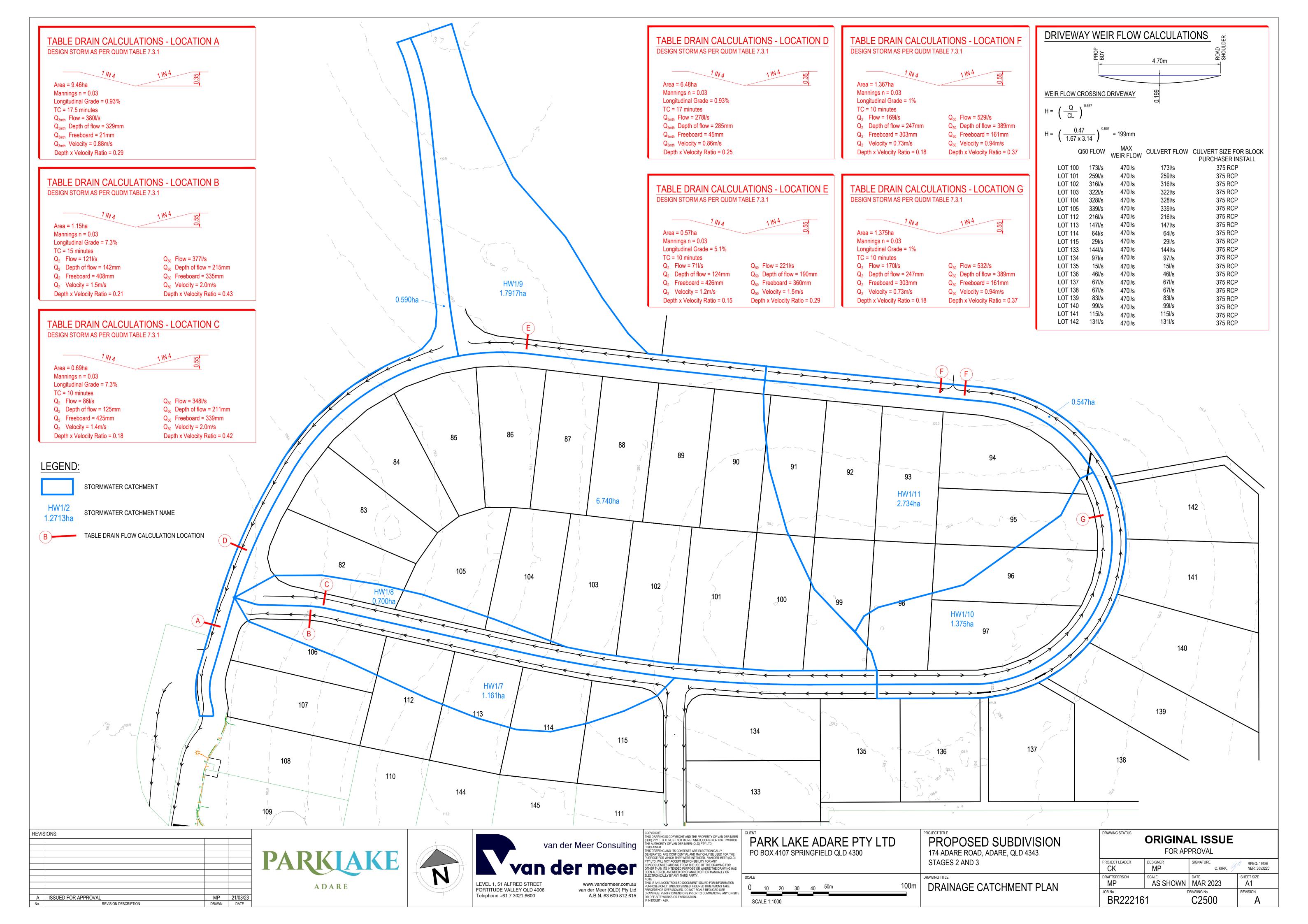
EXISTING		PROPOSED	
	FENCE	$\longrightarrow$	OPEN DRAIN
$\longrightarrow\longrightarrow\longrightarrow$	OPEN DRAIN		KERB AND CHANNEL TYPE B1
	MOUNTABLE KERB TYPE M4		MOUNTABLE KERB TYPE M4
	MOUNTABLE KERB TYPE M6		MOUNTABLE KERB TYPE M6
	CONTOUR (0.100m)	<u> </u>	ROAD EDGE BITUMEN
<u> </u>	ROAD EDGE BITUMEN		ROAD SHOULDER
	TREE TO REMAIN		DRAINAGE
	TREE TO BE REMOVED		

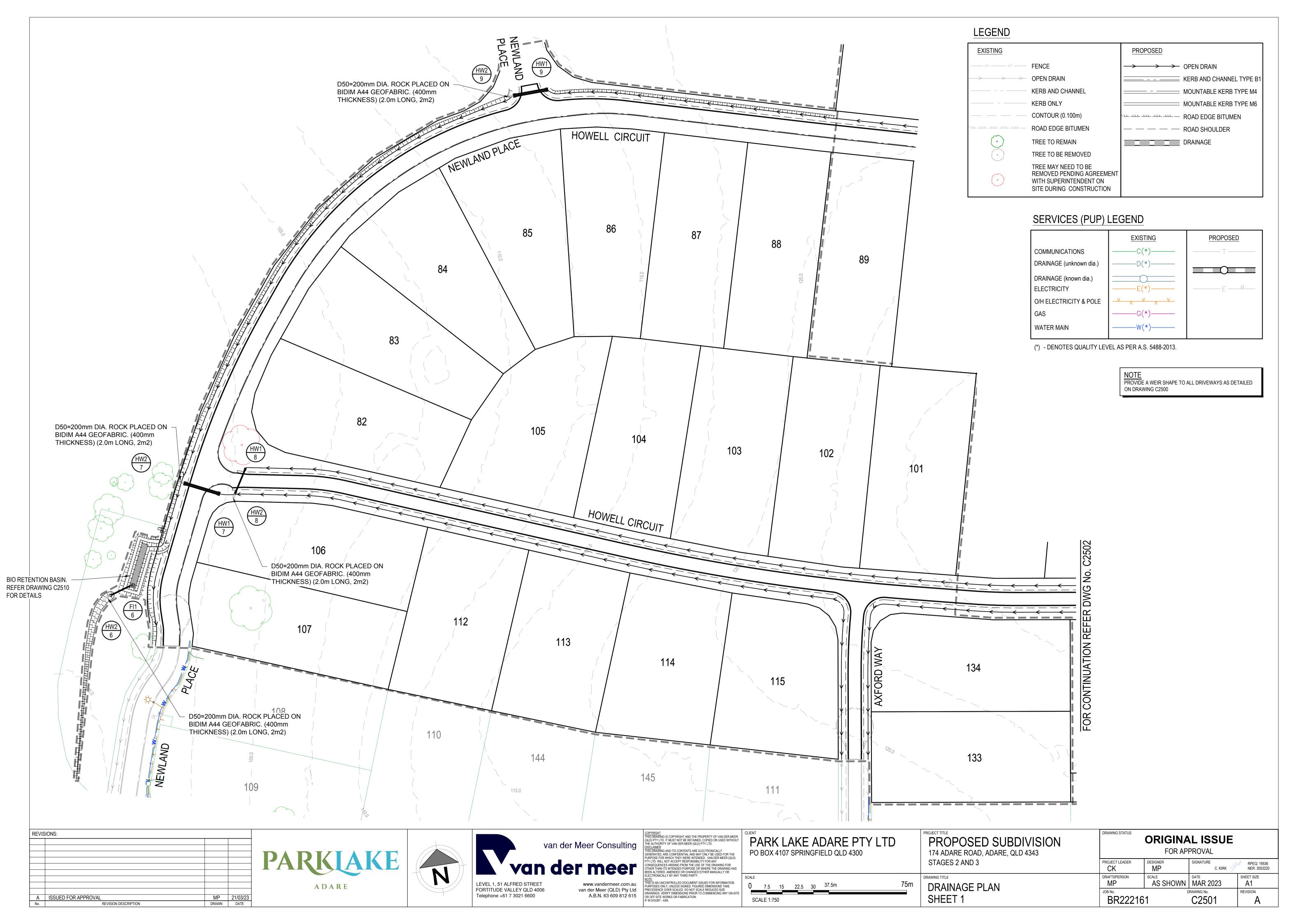
## SERVICES (PUP) LEGEND

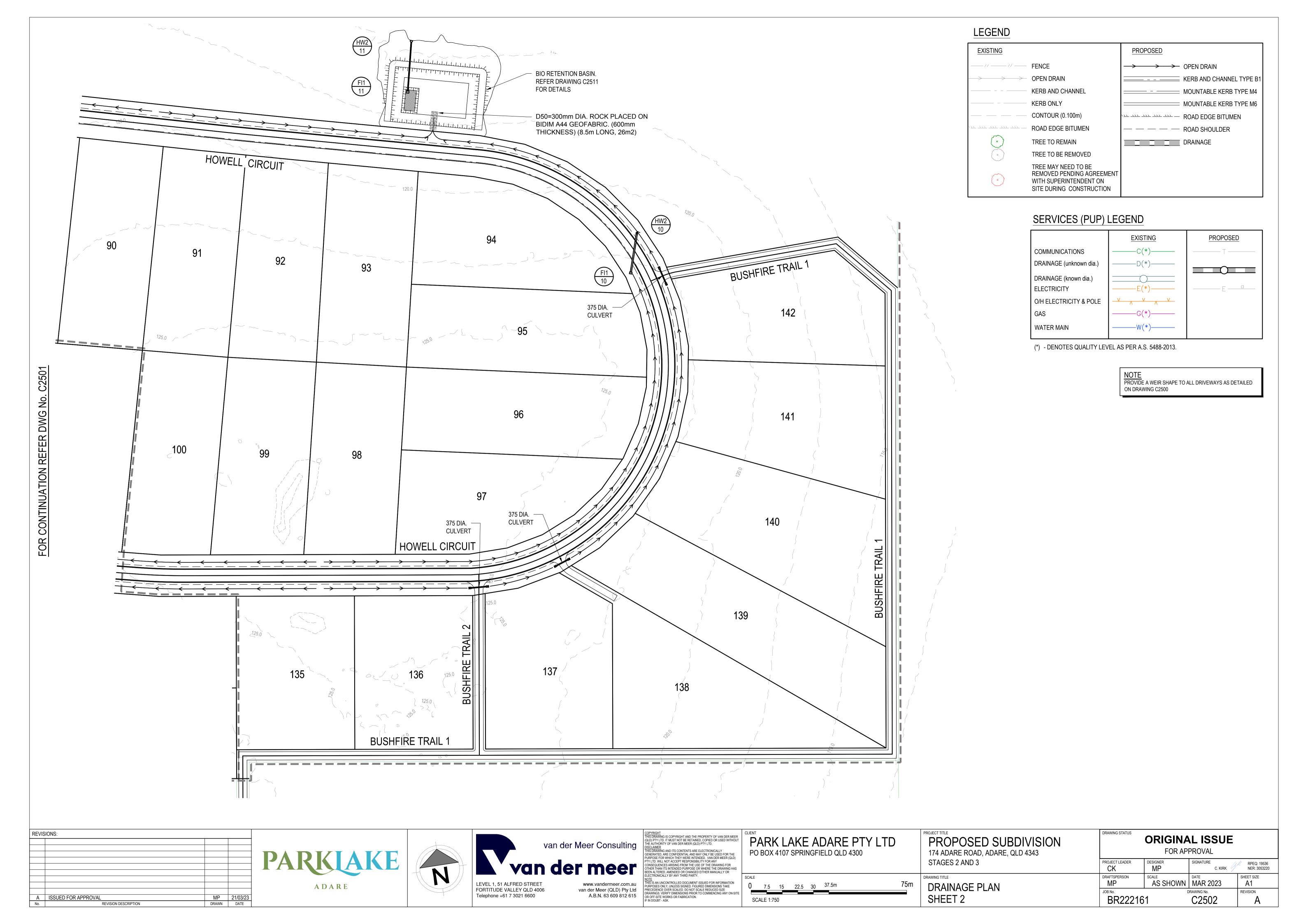
EXISTING	PROPOSED
C(*)	
D(*)	
———E(*)——	
G(*)	
W(*)	W

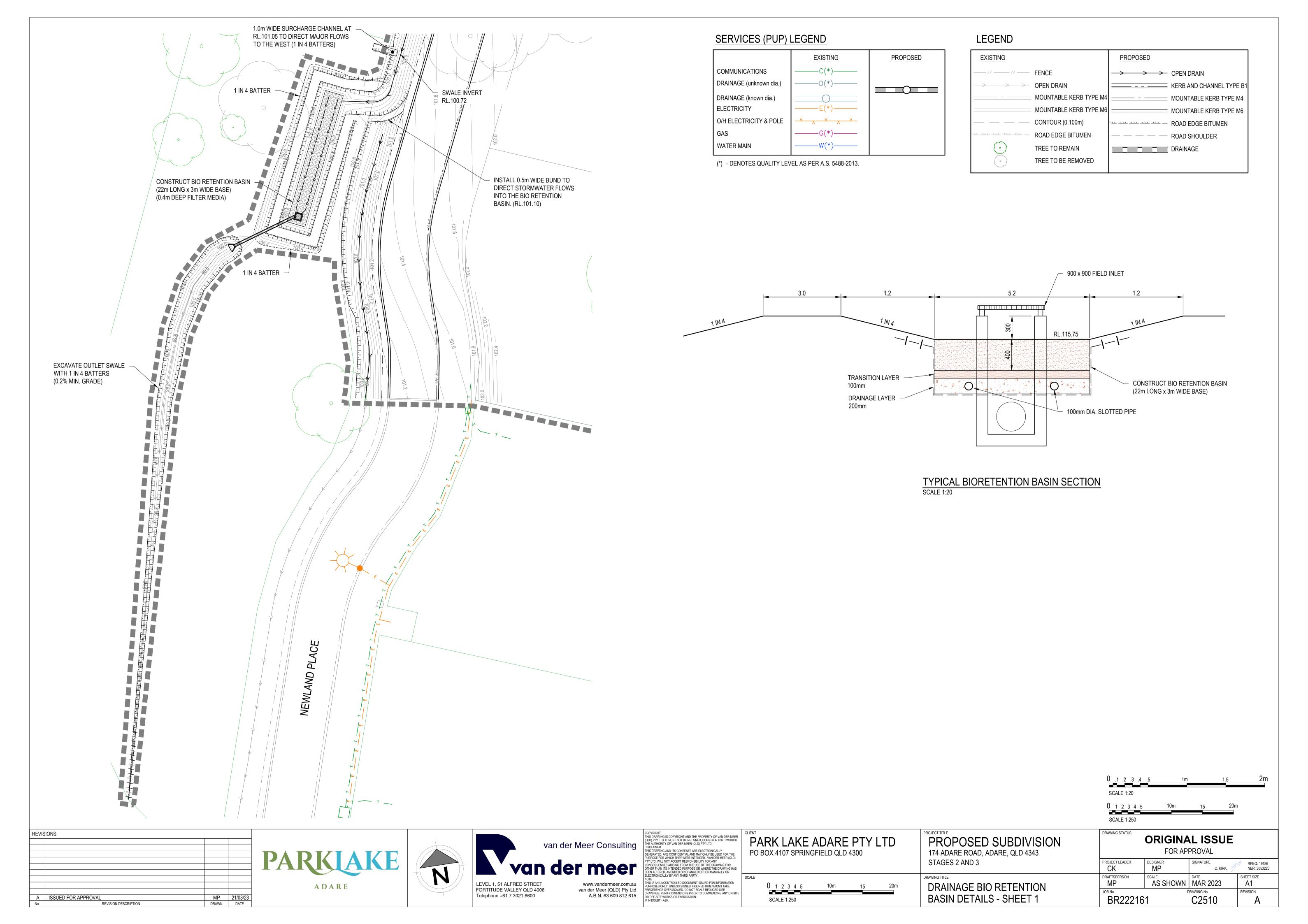
(\*) - DENOTES QUALITY LEVEL AS PER A.S. 5488-2013.

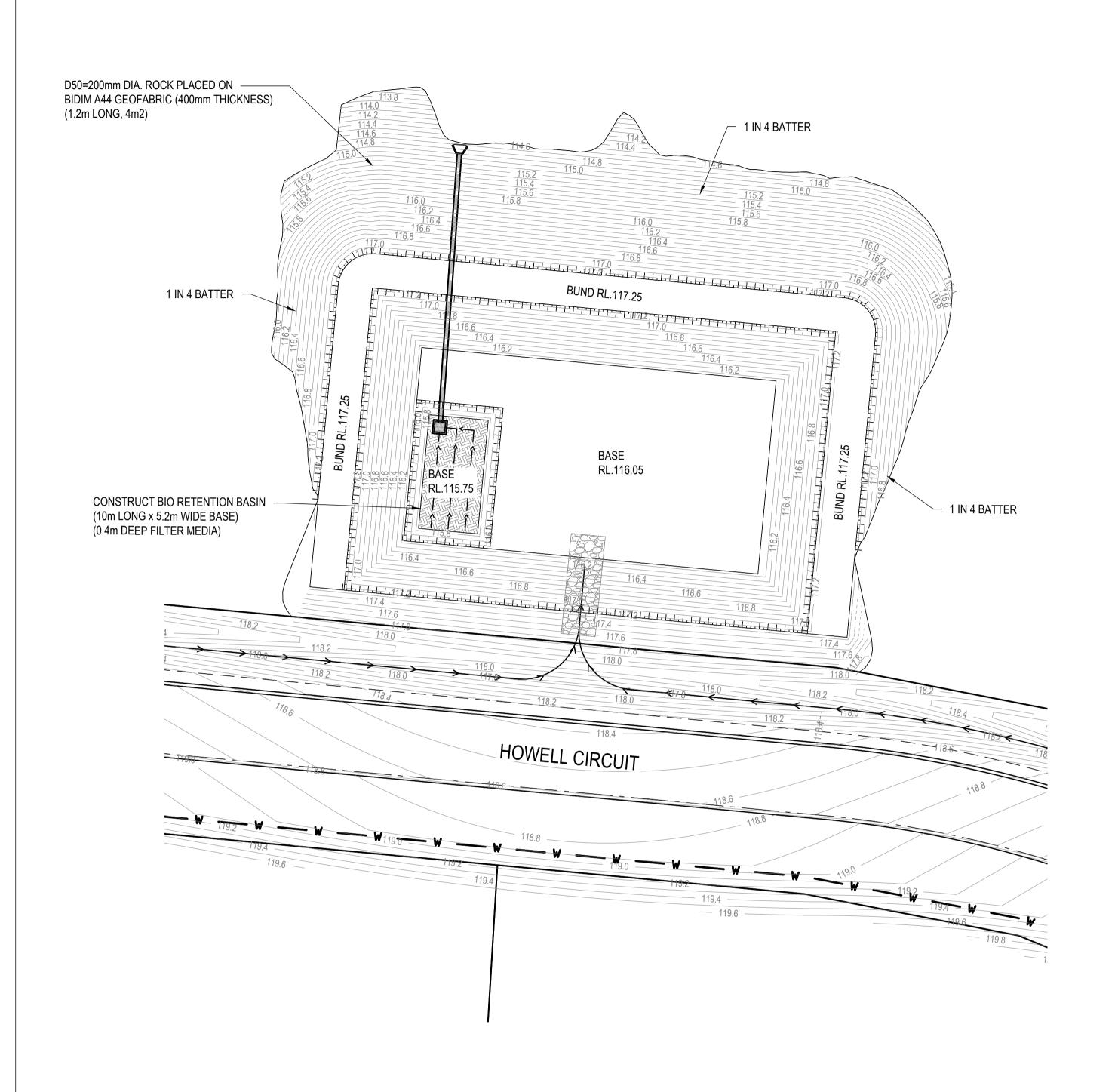




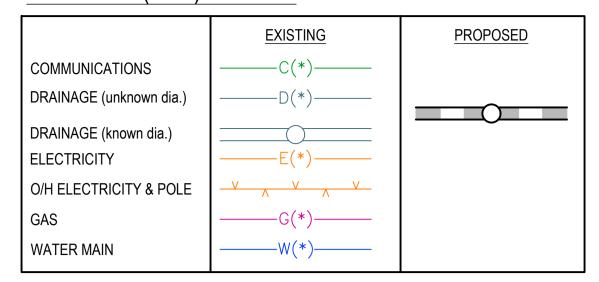






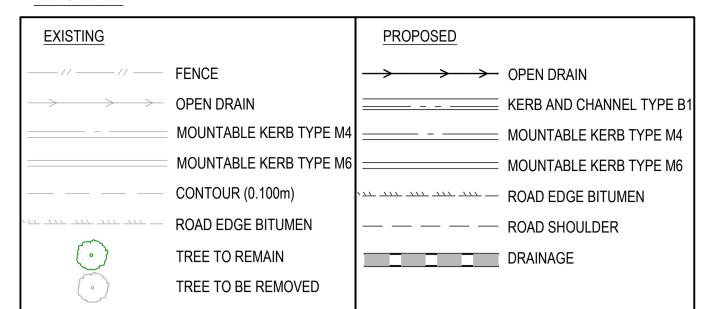


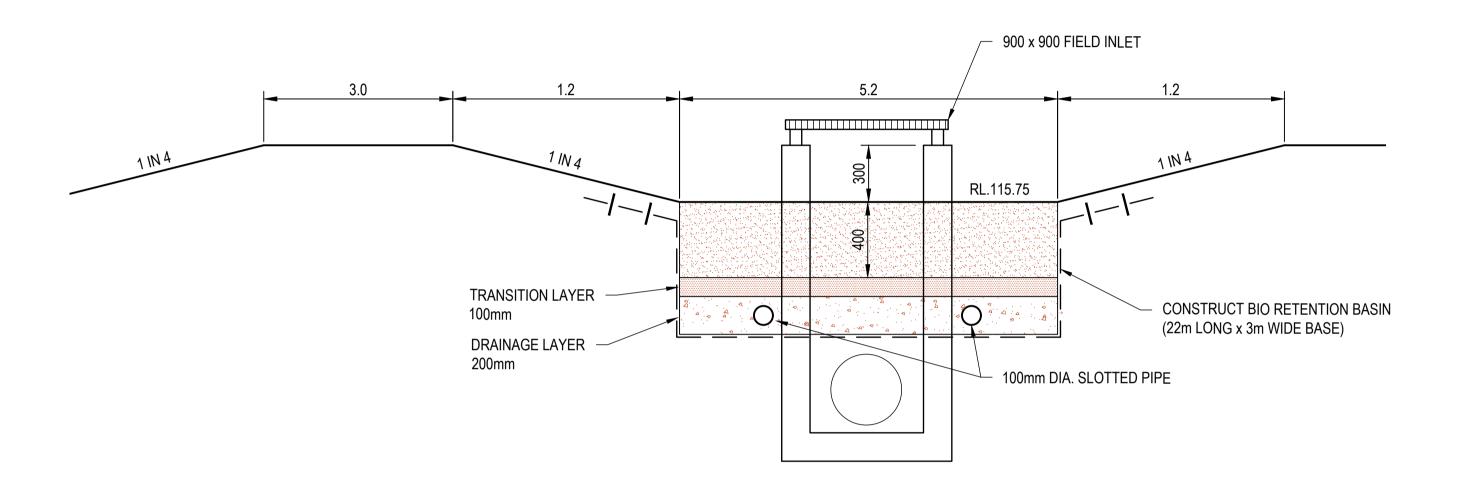
### SERVICES (PUP) LEGEND



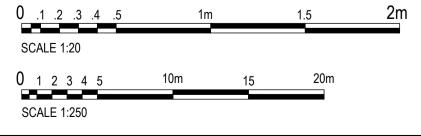
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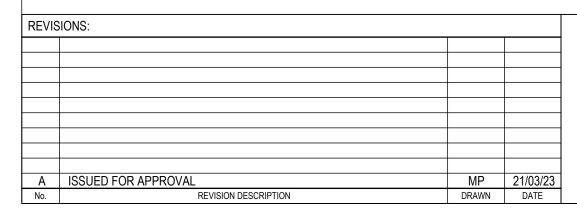
#### LEGEND





TYPICAL BIORETENTION BASIN SECTION









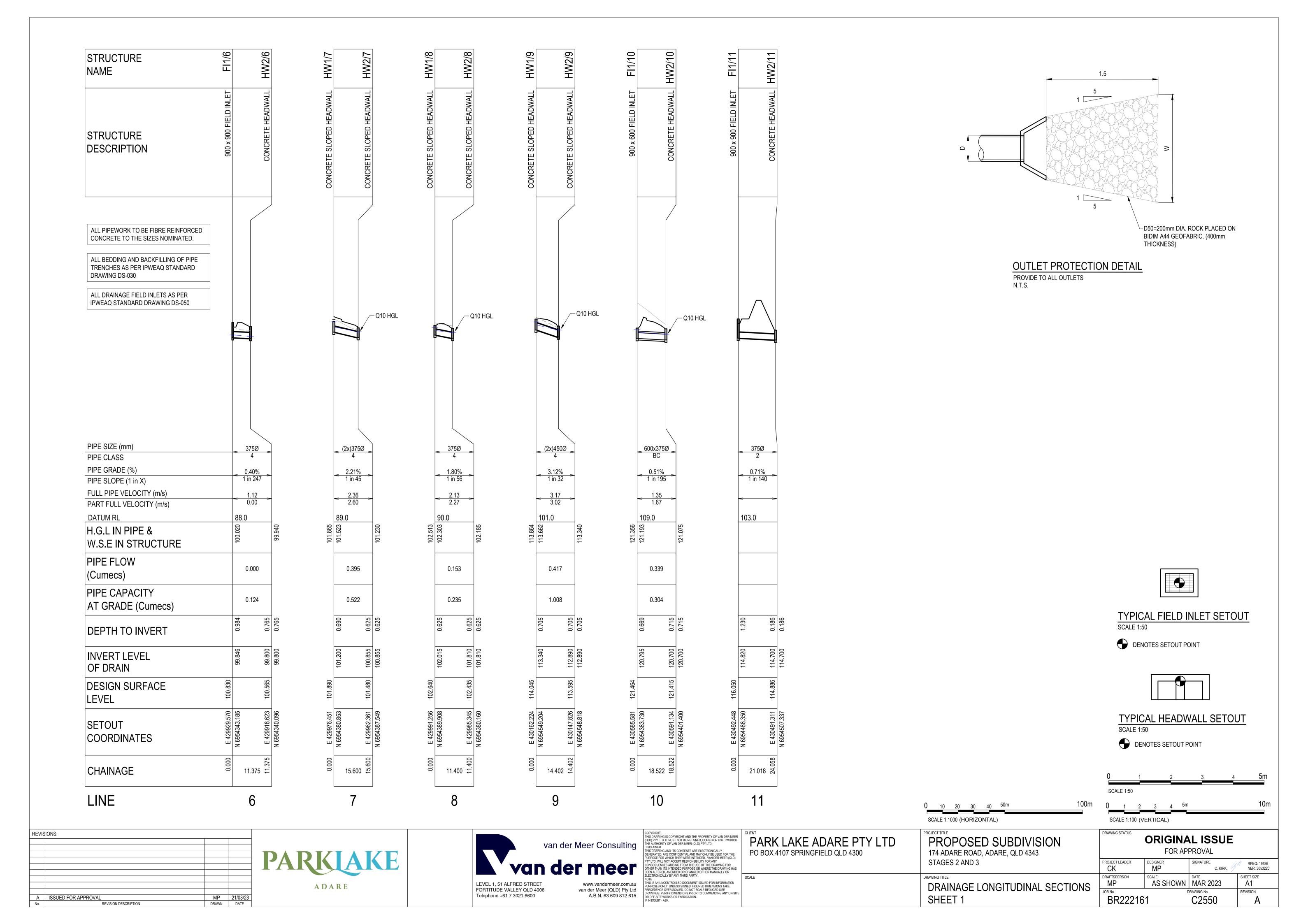




SCALE 1:250

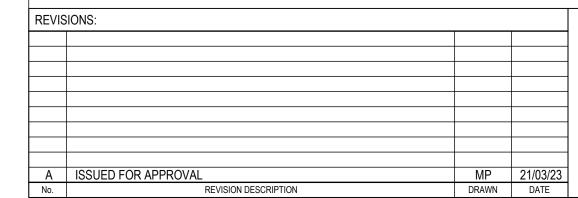
ARK LAKE A BOX 4107 SPRING		LTD	PROPOSED SUE 174 ADARE ROAD, ADARE, STAGES 2 AND 3	
0 1 2 3 4 5	10m	15	20m	DRAINAGE BIO RET

	SCALE 1:250			
PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343	ORIGINAL ISSUE FOR APPROVAL			
STAGES 2 AND 3	PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220
DRAINAGE BIO RETENTION	DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE  A1
BASIN DETAILS - SHEET 1	JOB No. BR22216		C2511	REVISION



LOCAT			ATCHMENT R									INLET DESIGN									IN DESIGN								D LOSSES				PART F	ULL			DESIGN LEVELS		
	Tc	I	A	CA	Qc	Qa									Qg Qb		Tc	I CA	Qrat	Q L	S	Vf=Q/	/A Qcap	Vcap \	/t		Vf²/2g	Ku h	hu Kw	hw	Sf	hf	dn	Vn					
STRUCTURE No. CATCHMENT CONTRIBUTING	DRAIN SECTION SUB-CATCHMENT TIME OF CONC.	CO-FFEICIENT OF	RUNOFF SUB-CATCHMENT AREA	EQUIVALENT AREA	SUB-CATCHMENT DISCHARGE	FLOW IN K&C (INC. BYPASS)	HALF ROAD CAPACITY	FLOW WIDTH	FLOW DEPTH	ROAD GRADE AT INLET	ROAD XFALL AT INLET	INLETTYPE	BLOCKAGE FACTOR	INLET CURVE	FLOW INTO INLET BYPASS FLOW	BYPASS STRUCTURE No.	CRITICAL TIME OF CONC.	RAINFALL INTENSITY TOTAL (C × A)	PEAK FLOW	PIPE FLOW REACH LENGTH	PIPE GRADE	PIPE SIZE FULL PIPE VELOCITY	CAPACITY FLOW	CAPACITY VELOCITY	I KAVEL VELOCI I Y	CHART(S) USED	VELOCITY HEAD	U/S HEAD LOSS COEFFICIENT	U/S HEAD LOSS W.S.E COEFFICIENT	CHANGE IN W.S.E	PIPE FRICTION SLOPE	PIPE FRICTION HEAD LOSS	NORMAL DEPTH	NORMAL DEPTH VEL.	PIPE U/S I.L	PIPE U/S H.G.L	PIPE D/S H.G.L	SURFACE LEVEL	FREEBOARD STRUCTURE NO.
	min mr	n/h	ha	ha	L/s	L/s	L/s	m	m m^2/s	%	%				L/s L/s		min m	nm/hr ha	L/s	L/s m	%	mm m/s	L/s	m/s m	n/s		m	n	m	m	%	m	m	m/s	m m	m	m m	m	
FI1/6 FI1/6	FI1/6 to HW2/6											900 x 900 FIELD INLET	0.5				5	182		11.374	6 0.40441	375	111.545	54 1.009949	2 Inle	et Control					0.40441	0.046		99	.846 99.8	99.846	99.846	100.83 0	.984 FI1/6
HW2/6 HW2/6												CONCRETE HEADWALL																									99.8	100.565	HW2/6
HW1/7 HW1/7	HW1/7 to HW2/7 14.96441 125.	1637	0.6 1.161	131 0.696678	242.219	242.219					-0.24911	CONCRETE HEADWALL	0.8		242.219		14.96441 12	5.1637 0.69667	8 395.219	395.219 15.6000	2.211537	7 (2x)375 1.7891	521.696	66 2.361759	2 Inle	et Control (	0.163326 2	.094857 0.343	42144	0.34214	1.877045	0.324294	0.244084	2.596179 10	1.2 100.855	101.5228 10	01.23 101.865	102.028 0.1	63037 HW1/7
HW2/7 HW2/7												CONCRETE HEADWALL																									101.23	101.48	HW2/7
HW1/8 HW1/8	HW1/8 to HW2/8 13.66939 131.	1208	0.6 0.699	582 0.419749	152.883	152.883					4	CONCRETE HEADWALL	0.8		152.883		13.66939 13	1.1208 0.41974	9 152.883	152.883 11.3998	87 1.798267	7 375 1.3842	225 235.216	66 2.129686	2 Inle	et Control	0.097759 2	.147086 0.209	09897	0.20989	1.03591	0.164186	0.220283	2.266472 102	2.015 101.81	102.3031 10	2.185 102.513	102.74 0.2	22701 HW1/8
HW2/8 HW2/8												CONCRETE HEADWALL																									102.185	102.435	HW2/8
HW1/9 HW1/9	HW1/9 to HW2/9 11.79067 139.	7629	0.6 1.79	1.075002	417.3484	417.3484					20.17928	CONCRETE HEADWALL	0.8		117.3484		11.79067 139	9.7629 1.07500	2 417.3484	417.3484 14.4023	3.124486	6 (2x)450 1.3120	06 1008.34	47 3.170044	2 Inle	et Control (	0.087832 2	.301313 0.20	02128	0.20212	2.23517	0.383977	0.201822	3.019445 11	3.34 112.89	113.6619	13.34 113.864	114.075 0.2	10954 HW1/9
HW2/9 HW2/9												CONCRETE HEADWALL																									113.34	113.595	HW2/9
FI1/10 FI1/10	FI1/10 to HW2/10 10 14	48 (	0.6 1.375	108 0.825065	339.1933	339.1933					24.99947	900 x 600 FIELD INLET	0.5		339.1933		10	148 0.82506	339.1933	339.1933 18.5217	76 0.51291	600x375 1.5075	304.279	95 1.352355	2 Inle	et Control	0.115951 1	.408411 0.16	63307	0.16330	0.683734	0.118052	0.337748	1.673798 120	0.795 120.7	121.1931 12	1.075 121.3564	121.7637 0.4	07328 FI1/10
HW2/10 HW2/10												CONCRETE HEADWALL																									121.075	121.415	HW2/1
FI1/11 FI1/11	FI1/11 to HW2/11											900 x 900 FIELD INLET	0.5							24.0576	0.499633	375												11	4.82 114.6998	3		116.05	FI1/11
HW2/11 HW2/11												CONCRETE HEADWALL																										114.8861	HW2/1

STORMWATER CALCULATIONS TABLE (Q10)









LIENT	PR
PARK LAKE ADARE PTY LTD	
PO BOX 4107 SPRINGFIELD QLD 4300	

ROJECT TITLE	
PROPOSED SUBDIVISION	
174 ADARE ROAD, ADARE, QLD 4343	
STAGES 2 AND 3	

DRAINAGE CALCULATIONS

DRAWING STATUS		AL ISSUE PROVAL			
PROJECT LEADER <b>CK</b>	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220		
DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1		
JOB No.		RAWING No.	REVISION		
BR22216	1	C2560	A		

	ASSET	REGISTER -	WATER	RETI	CULATION			
ESTATE/	STAGE	STAGES	STAGES 2 AND 3					
SITE ADI	DRESS	174 ADAF	174 ADARE ROAD, ADARE					
FILE/APF	PLICATIO	N						
DELEGA	TES APP	. DATE						
CLIENT		PARI	K LAKE /	ADAF	RE PTY LTD			
DRAWIN	G/PLAN I	No.						
MAINS	DIA.	MATE	RIAL		LEN	IGTH		
	DIA.	DESIGN	CONS	T	DESIGN	CONST.		
	DN125	PE			1252m			
	DN180	PE		406m				
SERVICES	DIA.	MATE	RIAL		LEN	IGTH		
	DIA.	DESIGN	CONS	T	DESIGN	CONST.		
	20mm							
	25mm							
	32mm	PE100 PN16			48m			
	40mm	PE100 PN16			70m			
METERS	DIA.	NUM	BER					
	20mm	40						
	25mm							
	32mm							
	150mm							
	50mm				1			

SERVICE DETAILS									
NO	SIZE	LOT NUMBERS							
28	DN25 PE	82-107, 133 AND 134							
12	DN32 PE	112-115 AND 135 TO 142							

ALL ENVIRONMENTAL PROTECTION MEASURES SHALL BE IMPLEMENTED PRIOR TO ANY CONSTRUCTION WORK, INCLUDING CLEARING COMMENCING.

ALL WATER AND SEWER CONSTRUCTION WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE QUEENSLAND WORKPLACE HEALTH AND SAFETY ACT 2011.

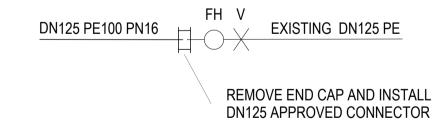
CONTACT THE DIVISION OF WORKPLACE HEALTH AND SAFETY FOR INFORMATION PHONE: 1300 362 128

SEQ CODE STANDARD DRAWING SCHEDULE

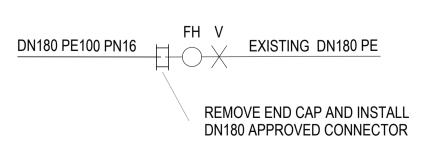
REVISION DESCRIPTION

SEQ-WAT-1200-1 SOIL CLASSIFICATION SEQ-WAT-1200-2 EMBEDMENT AND TRENCH FILL THRUST BLOCK DETAILS SEQ-WAT-1205-1 SEQ-WAT-1206-1 VALVE THRUST BLOCKS **IDENTIFICATION MARKERS** SEQ-WAT-1300-1,2

STAGE 3 **ADARE** STAGE 2 ROAD 134 133 STAGE 1 REDBANK CREEK ROAD O LOCALITY PLAN SCALE 1:5000

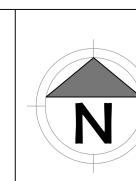


LIVE CONNECTION DETAIL '2' DIAGRAMMATIC



LIVE CONNECTION DETAIL '1' DIAGRAMMATIC

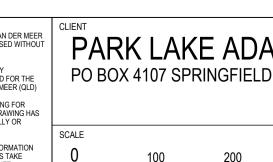
**REVISIONS:** ADARE MP 21/03/23 A ISSUED FOR APPROVAL





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IF IN DOUBT - ASK.



SCALE 1:5000

## PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300

PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343 STAGES 2 AND 3

SIGNATURE: **ORIGINAL ISSUE** FOR APPROVAL RPEQ: 19536 MP C. KIRK NER: 3053220 DRAFTSPERSON SHEET SIZE AS SHOWN | MAR 2023 Α1 WATER RETICULATION COVER SHEET REVISION BR222161 C2600

ALL LIVE WORKS ON AC MAIN SHALL INCLUDE REMOVAL OF EXISTING AC MAIN FROM COLLAR TO COLLAR.

VALVE MARKERS, HYDRANT MARKERS AND PAVEMENT MARKINGS ARE TO BE INSTALLED/REINSTATED IN ACCORDANCE WITH SEQ-WAT-1300-1

**GENERAL NOTES** 

AUSTRALIAN STANDARDS.

NO.SEQ-WAT-1410-1.

UTILITIES.

THE DEVELOPER'S EXPENSE.

**VEGETATION PROTECTION** 

REPLACED IF DESTROYED.

REQUIREMENTS.

FURTHER ADVICE.

REHABILITATION

SOIL

SUPPLY CODE SPECIFICATIONS AND STANDARDS.

8. ALL CONCRETE FOOTPATHS TO BE CLEAR OF WATER MAINS.

WATER MAIN CONSTRUCTION NOTES

ADOPT LIP OF KERB OR SHOULDER OF ROAD AS PERMANENT LEVEL

LOT IN ACCORDANCE WITH THE STANDARD DRAWING FOR THE SEQ-SP

NOT BE PERMITTED TO CONNECT TO THE RETICULATION SYSTEM.

WELDING CERTIFICATE IN ACCORDANCE WITH AS/NZS 2033.

1. ALL WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH CURRENT SOUTH EAST QUEENSLAND WATER

A WATER METER SUPPLIED AT THE DEVELOPER'S COST, IS TO BE INSTALLED AT THE SERVICE POINT OF EACH

MATERIALS LIST OR BE APPROPRIATELY SHOWN. LISTED AND DEFINED IN THE ENGINEERING SUBMISSION SO THAT THE ALTERNATIVE PRODUCT OR MATERIAL CAN BE ASSESSED AND IF APPROPRIATE, APPROVED BY

2. UNLESS SPECIFIED OTHERWISE ALL MATERIALS AND WORK SHALL COMPLY WITH THE RELEVANT

7. ALL MATERIALS USED IN THE WORKS SHALL COMPLY WITH THE SEQ-SP'S ACCEPTED PRODUCTS AND

TEST/CHLORINATION POINTS TO BE INSTALLED IN ACCORDANCE WITH STANDARD DRAWING

10. THE CONSTRUCTION OF THE WATER RETICULATION WORK SHOWN ON THIS DRAWING MUST BE SUPERVISED BY AN ENGINEER WHO HAS RPEQ REGISTRATION. WORKS NOT COMPLYING WITH THIS REQUIREMENT WILL

11. WATER MAIN ROAD CROSSING AND VALVE PAVEMENT MARKERS TO BE INSTALLED AS PER SEQ-WAT-1300-1.

1. DURING ANY CONSTRUCTION ACTIVITY AT LEAST ONE PERSON ON SITE MUST HAVE COMPLETED A PIPE

LAYING TRAINING COURSE APPROVED BY THE PIPE SUPPLIER OR MANUFACTURER AND APPROPRIATE TO THE PIPELINE UNDER CONSTRUCTION. THE TRAINING COURSE MUST HAVE BEEN COMPLETED WITHIN THE

ALL SITE AND FACTORY PE WELDING SHALL BE CARRIES OUT BY A PERSON WHO HAS COMPLETED RELEVANT NATIONALLY ACCREDITED TRAINING COURSES FOR BUTT WELDING/ELECTROFUSION AND HOLD A VALID

THE CONTRACTOR SHALL PROVIDE DOCUMENTED EVIDENCE OF ACCEPTABLE QUALIFICATIONS TO URBAN

CONSTRUCT FIRE HYDRANTS AND STOP VALVES TO SEQ-WAT-1301-1, 1302-1, 1303-2, 1305-1, 1306-1 & 1409-1. ALL LIVE WORKS SHALL BE UNDERTAKEN BY THE DEVELOPER'S LICENSED CONTRACTOR IN ACCORDANCE WITH A VALID URBAN UTILITIES NETWORK ACCESS PERMIT, UNDER SUPERVISION OF URBAN UTILITIES, AT

PROPOSED WORKS ARE LOCATED WITHIN FIRE ANT BIOSECURITY ZONE 2. ALL WORKS ARE TO BE TO DAFF

WHEN WORKING WITHIN 4m OF TREES, RUBBER OR HARDWOOD GIRDLES SHALL BE CONSTRUCTED WITH 1.8m BATTENS CLOSELY SPACED AND ARRANGED VERTICALLY FROM GROUND LEVEL. GIRDLES SHALL BE

CARE SHALL BE TAKEN TO PREVENT SEDIMENT FROM ENTERING THE STORMWATER SYSTEM. THIS MAY

IF ACID SULPHATE SOILS EXIST IN THE WORKS AREA THE OUTPUTS FROM THE RISK ASSESSMENT BASED ON

ALL DISTURBED AREAS ASSOCIATED WITH CONSTRUCTION SHALL BE REHABILITATED. HEAVILY COMPACTED

ALL PLANTING/RE-VEGETATION WILL NEED TO BE MAINTAINED THROUGHOUT THE MAINTENANCE PERIOD.

TRENCH REINSTATEMENT IS TO BE UNDERTAKEN IN ACCORDANCE WITH SEQ STANDARD REQUIREMENTS

FOR EMBEDMENT AND TRENCH FILL. STANDARD DRAWINGS HAVE BEEN NOTED ON THIS PLAN.

TREE ROOTS SHALL BE TUNNELLED UNDER, RATHER THAN SEVERED. IF ROOTS ARE SEVERED THE DAMAGED AREA SHALL BE TREATED WITH A SUITABLE FUNGICIDE. CONTACT RELEVANT COUNCIL ARBORIST FOR

1. TREES LOCATED ALONG THE FOOTPATH SHALL BE, TRANSPLANTED PRIOR TO CONSTRUCTION, OR

STRAPPED TO TREES PRIOR TO CONSTRUCTION AND REMAIN UNTIL COMPLETION.

ANY TREE LOPPING REQUIRED SHOULD BE UNDERTAKEN BY AN APPROVED ARBORIST.

INVOLVE PLACING APPROPRIATE SEDIMENT CONTROLS AROUND STOCKPILES.

THE QUEENSLAND ACID SULPHATE SOIL TECHNICAL MANUAL SHALL BE ADHERED TO.

PRE-DISTURBANCE SOIL PROFILES AND COMPACTION LEVELS SHALL BE REINSTATED.

TOPSOIL AND SUBSOIL SHALL BE STOCKPILED SEPARATELY

PRE-DISTURBANCE VEGETATION PATTERNS SHALL BE RESTORED.

ALL DISTURBED AREAS ARE TO BE LEFT IN STABLE CONDITION.

AREAS SHOULD BE RIPPED PRIOR TO TREATMENT

INSTALL DETECTABLE MARKER TAPE ON ALL WATER MAINS AND PROPERTY SERVICES.

COVER ON MAINS FROM PERMANENT LEVEL TO BE AS SHOWN IN SEQ-WAT-1200-2. CONDUITS TO BE INSTALLED IN ACCORDANCE WITH THE STANDARD DRAWINGS.

AS CONSTRUCTED

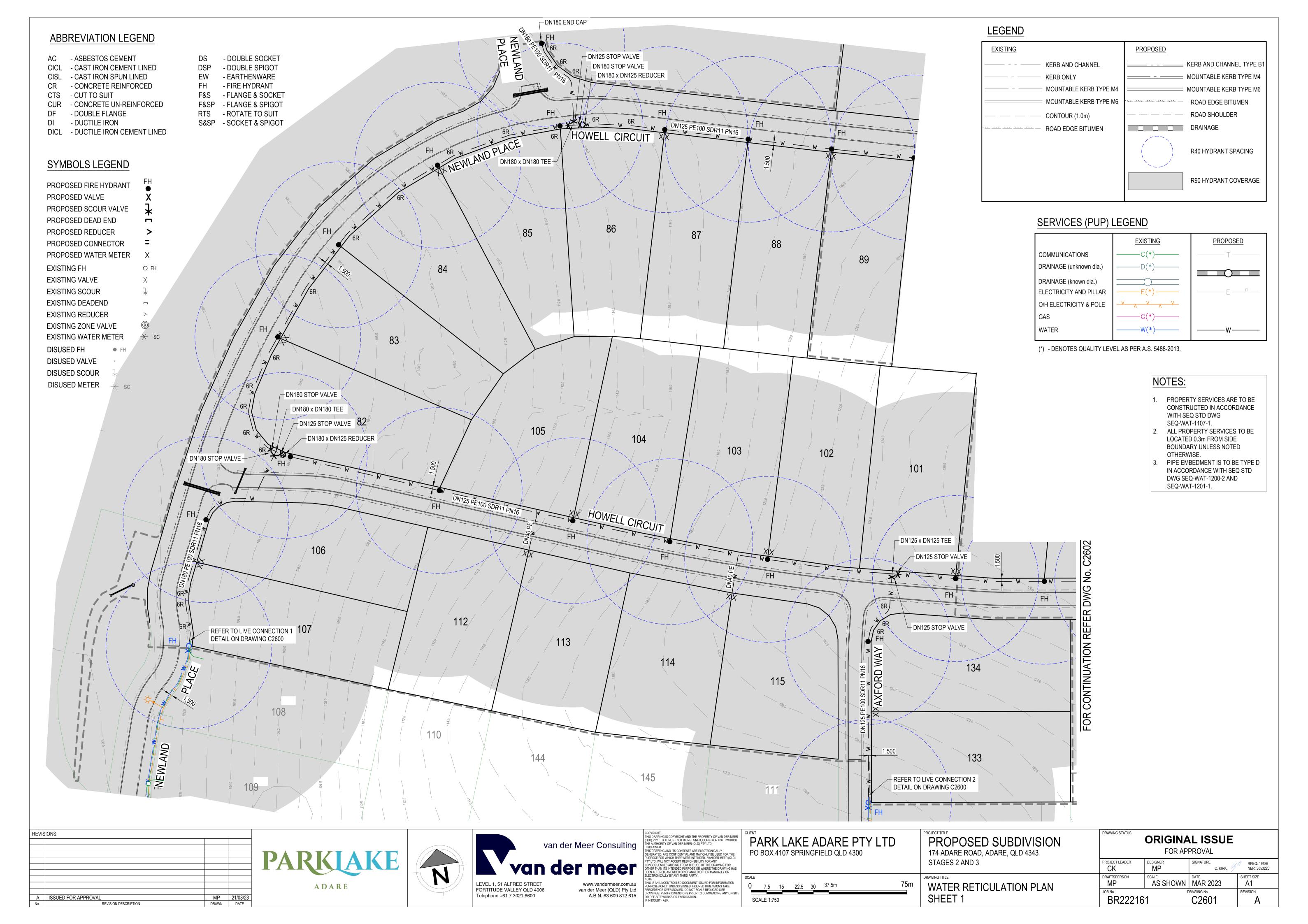
AS CONSTRUCTED DOCUMENTATION IS TO BE PROVIDED IN ACCORDANCE WITH REQUIREMENTS AS SPECIFIED IN THE SEQ D&C CODE INCLUSIVE OF FULL ADAC COMPLAINT SURVEY

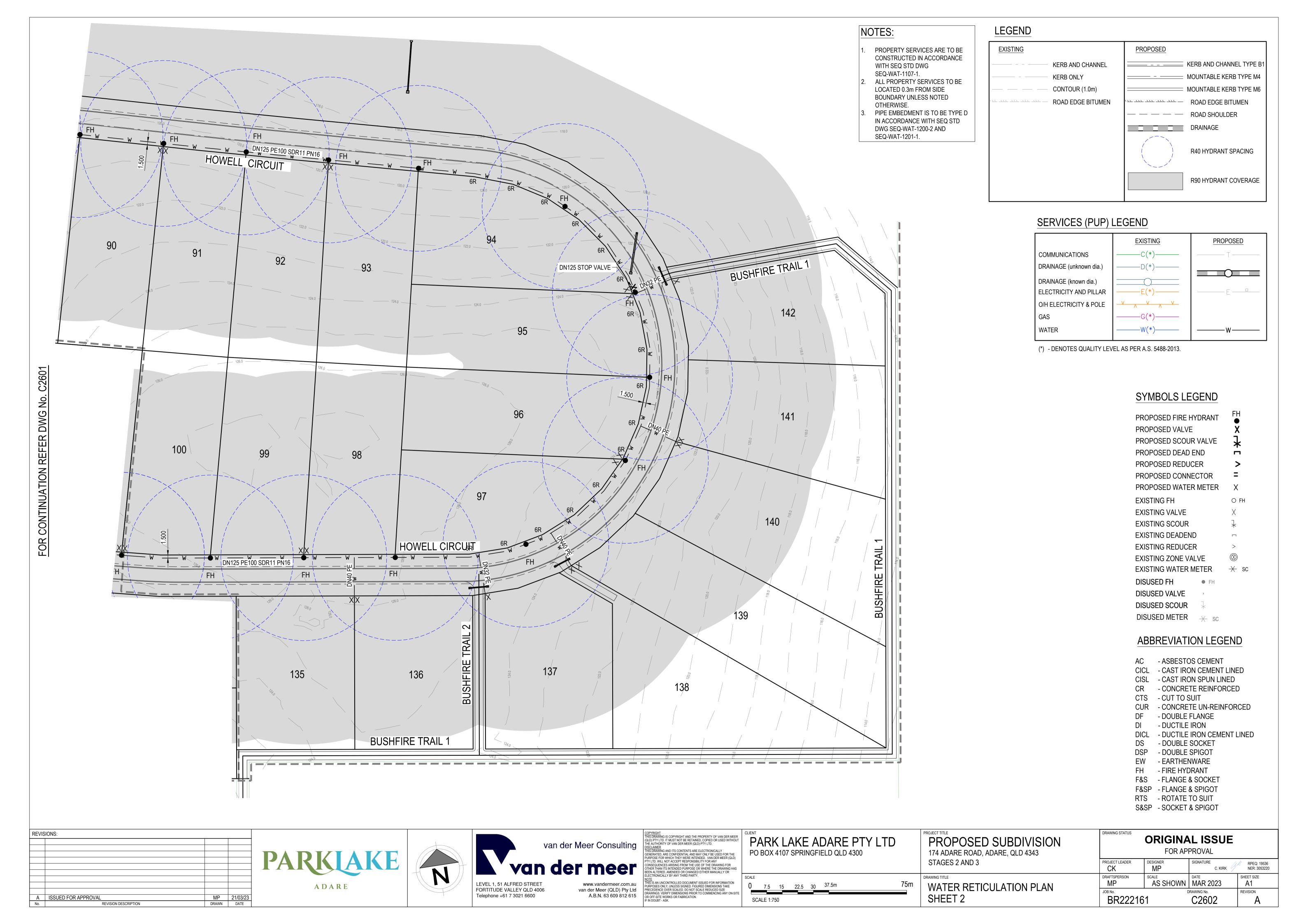
#### LIVE WORKS CONNECTION 1

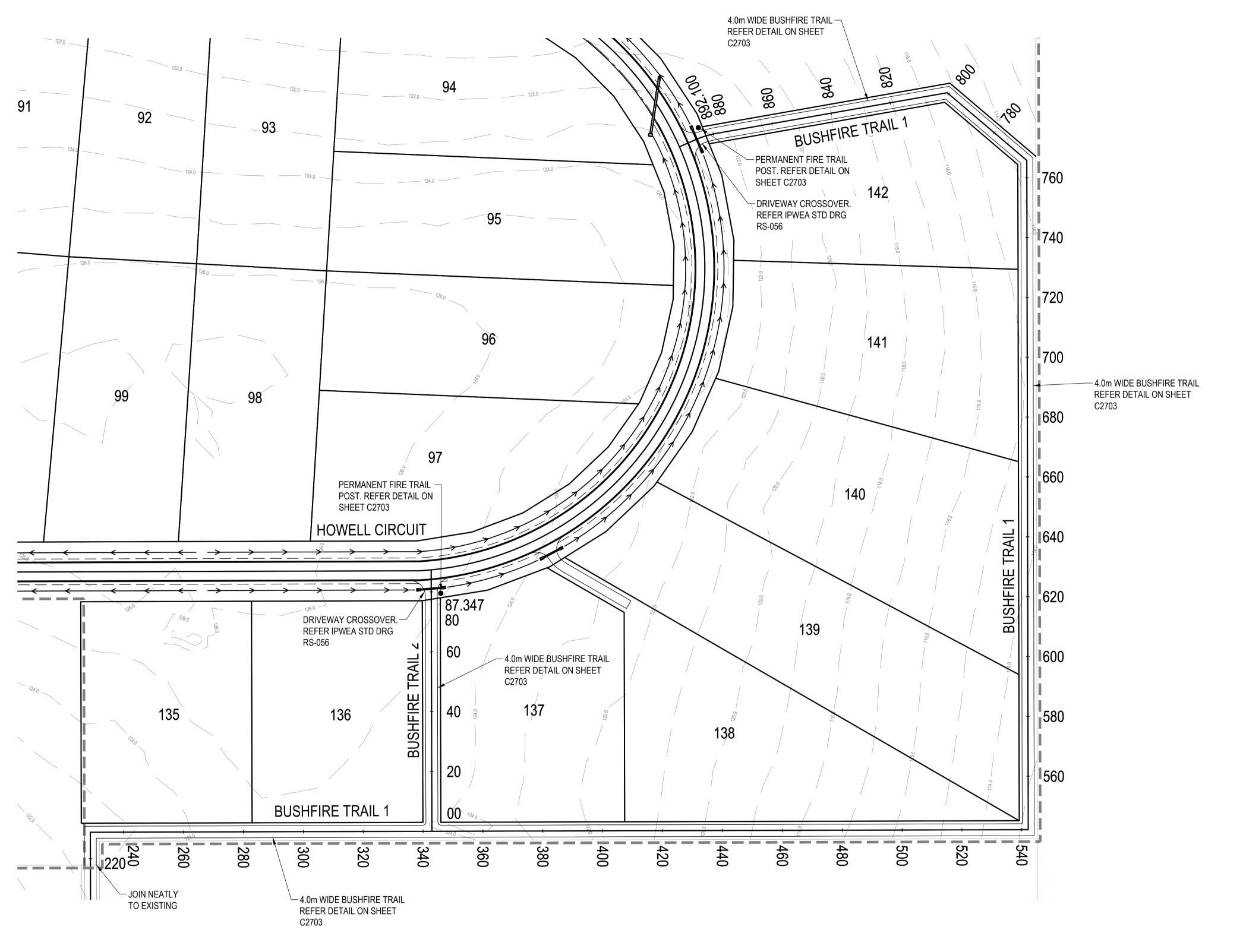
LIVE WORKS	<u>STATESTISTET</u>	
STREET:	NEWLAND PLACE	
INSTALLATION :		
TYPE OF MAIN :	EXISTING DN180 PE	
DATE COMMENCED:	DATE COMPLETED :	
SIGNATURE :		

### LIVE MODKS COMMECTION 2

LIVE WORKS CO	JANECTION 2	
STREET:	AXFORD WAY	
INSTALLATION :		
TYPE OF MAIN :	EXISTING DN125 PE	
DATE COMMENCED:	DATE COMPLETED :	
0.0		





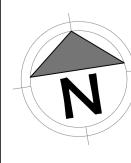


### LEGEND

	·
EXISTING	<u>PROPOSED</u>
	->->- OPEN DRAIN
OPEN DRAIN	KERB AND CHANNEL TYPE B1
KERB AND CHANNEL	——— - MOUNTABLE KERB TYPE M4
KERB ONLY	MOUNTABLE KERB TYPE M6
CONTOUR (0.100m)	ROAD EDGE BITUMEN
ROAD EDGE BITUMEN	— — — — ROAD SHOULDER
TREE TO REMAIN	DRAINAGE
TREE TO BE REMOVED	

REVISIONS: REVISION DESCRIPTION







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SCALE 0	7.5	15	22.5	30	37.5m		75m

0 7.5 15 22.5 30 37.5m

SCALE 1:750

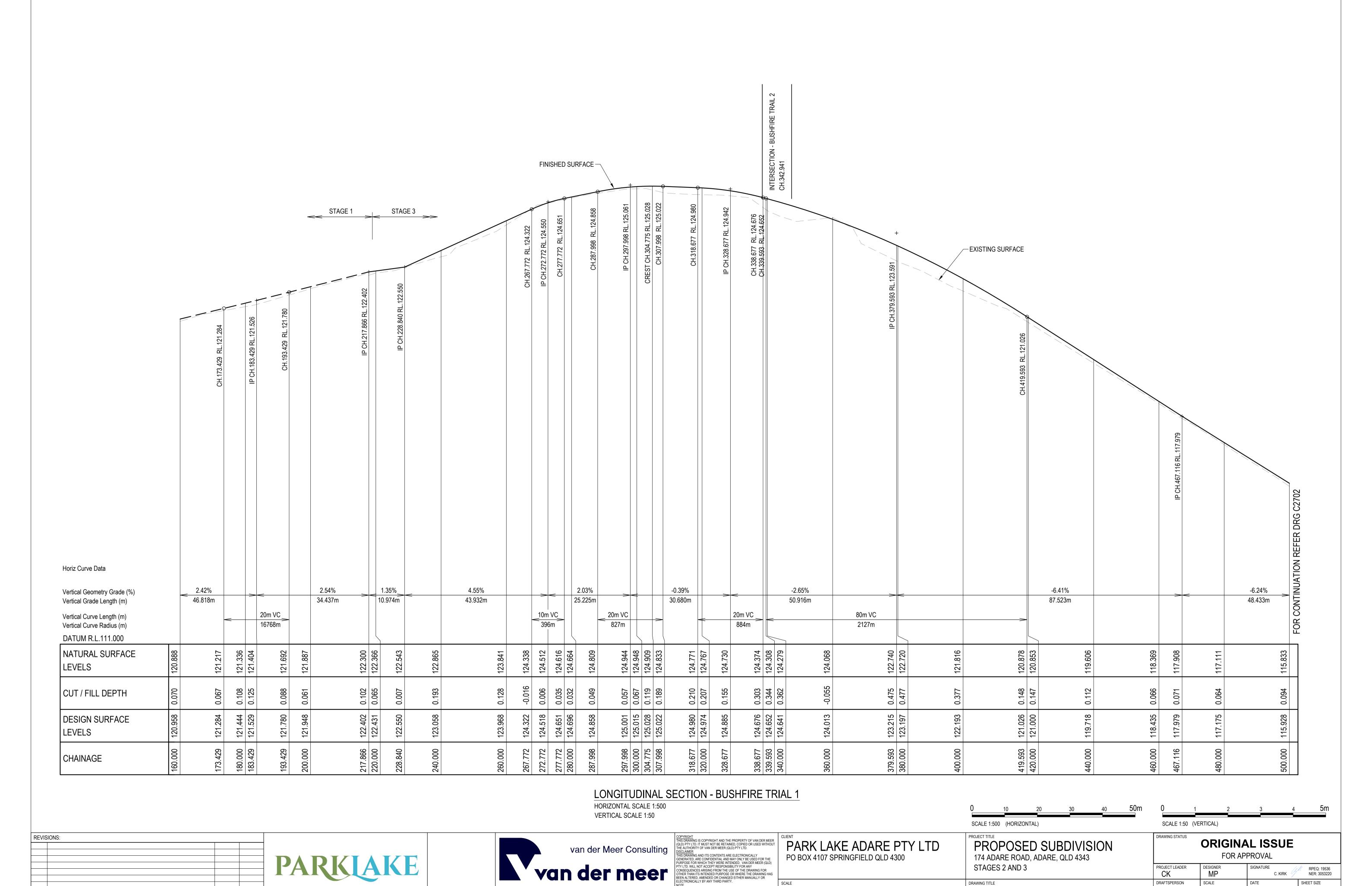
PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343	ORIGINAL I FOR APPROV					
STAGES 2 AND 3	PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK			
BUSHFIRE TRAIL PLAN	DRAFTSPERSON MP	AS SHOWN	MAR 2023			
DOUTH INC TRAIL FLAN	JOB No. BR22216		C2700			

RPEQ: 19536 NER: 3053220

SHEET SIZE

A1

REVISION



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www.vandermeer.com.au

A.B.N. 63 609 812 615

van der Meer (QLD) Pty Ltd

FORTITUDE VALLEY QLD 4006

Telephone +61 7 3021 6600

ADARE

REVISION DESCRIPTION

AS SHOWN | MAR 2023

C2701

Α1

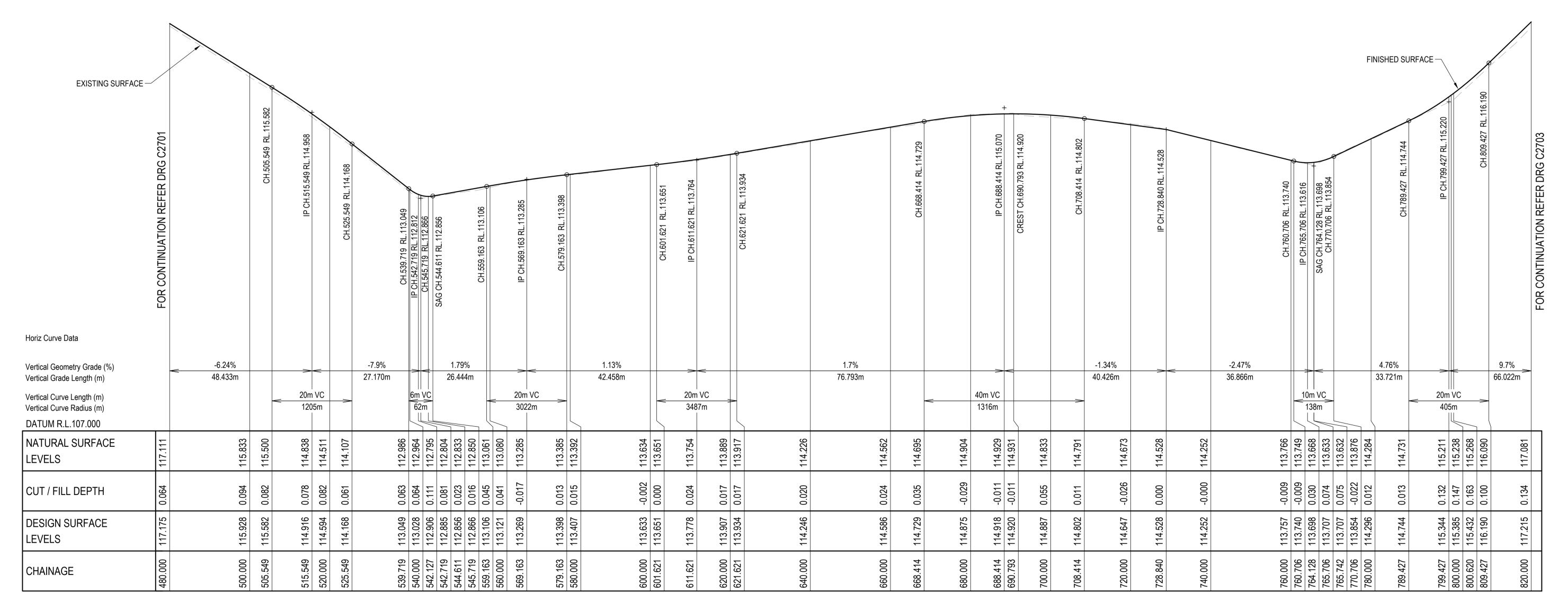
REVISION

MP

BR222161

BUSHFIRE TRAIL 1 LONGITUDINAL

SECTION - SHEET 1 OF 3



LONGITUDINAL SECTION - BUSHFIRE TRIAL 1

HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:50

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IF IN DOUBT - ASK.

PARK LAKE ADARE PTY LTD
PO BOX 4107 SPRINGFIELD QLD 4300

PROPOSED SUBDIVISION
174 ADARE ROAD, ADARE, QLD 4343
STAGES 2 AND 3

BUSHFIRE TRAIL 1 LONGITUDINAL

SECTION - SHEET 2 OF 3

SCALE 1:500 (HORIZONTAL)

50m

SCALE 1:50 (VERTICAL)

BR222161

40

ORIGINAL ISSUE
FOR APPROVAL

PROJECT LEADER CK MP SIGNATURE RPEQ: 19536
NER: 3053220

DRAFTSPERSON SCALE AS SHOWN MAR 2023 A1

JOB No. DRAWING No. REVISION

C2702

REVISIONS:

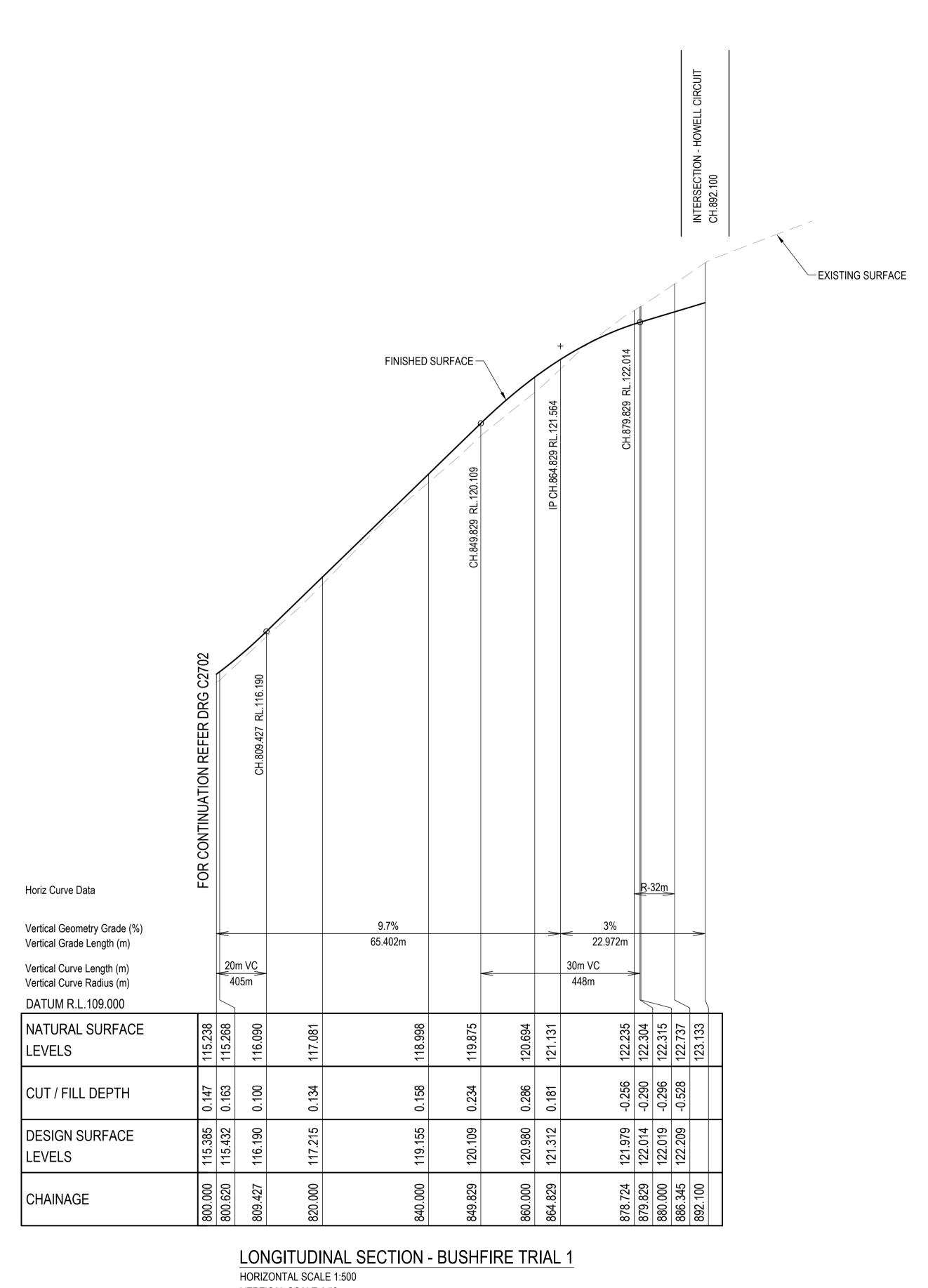
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No. REVISION DESCRIPTION

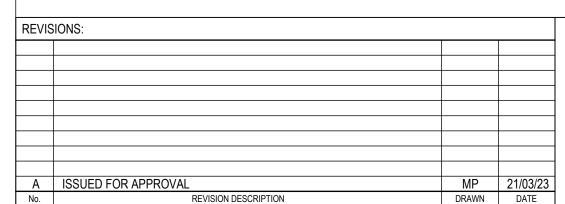
MP 21/03/23







VERTICAL SCALE 1:50









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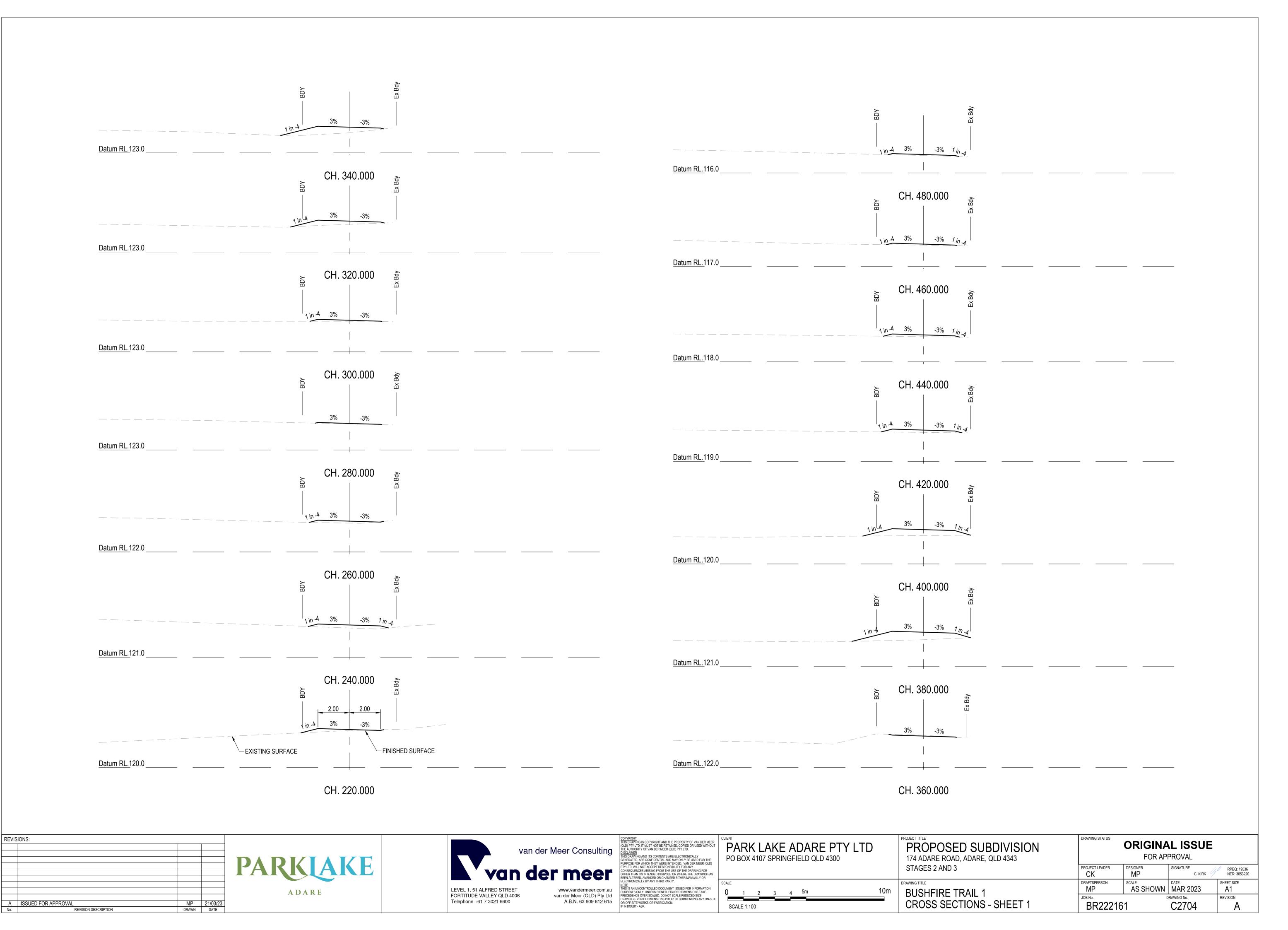
PROJECT TITLE
PROPOSED SUBDIVISION
174 ADARE ROAD, ADARE, QLD 4343
STAGES 2 AND 3
DRAWING TITLE
BUSHFIRE TRAIL 1 LONGITUDINAL

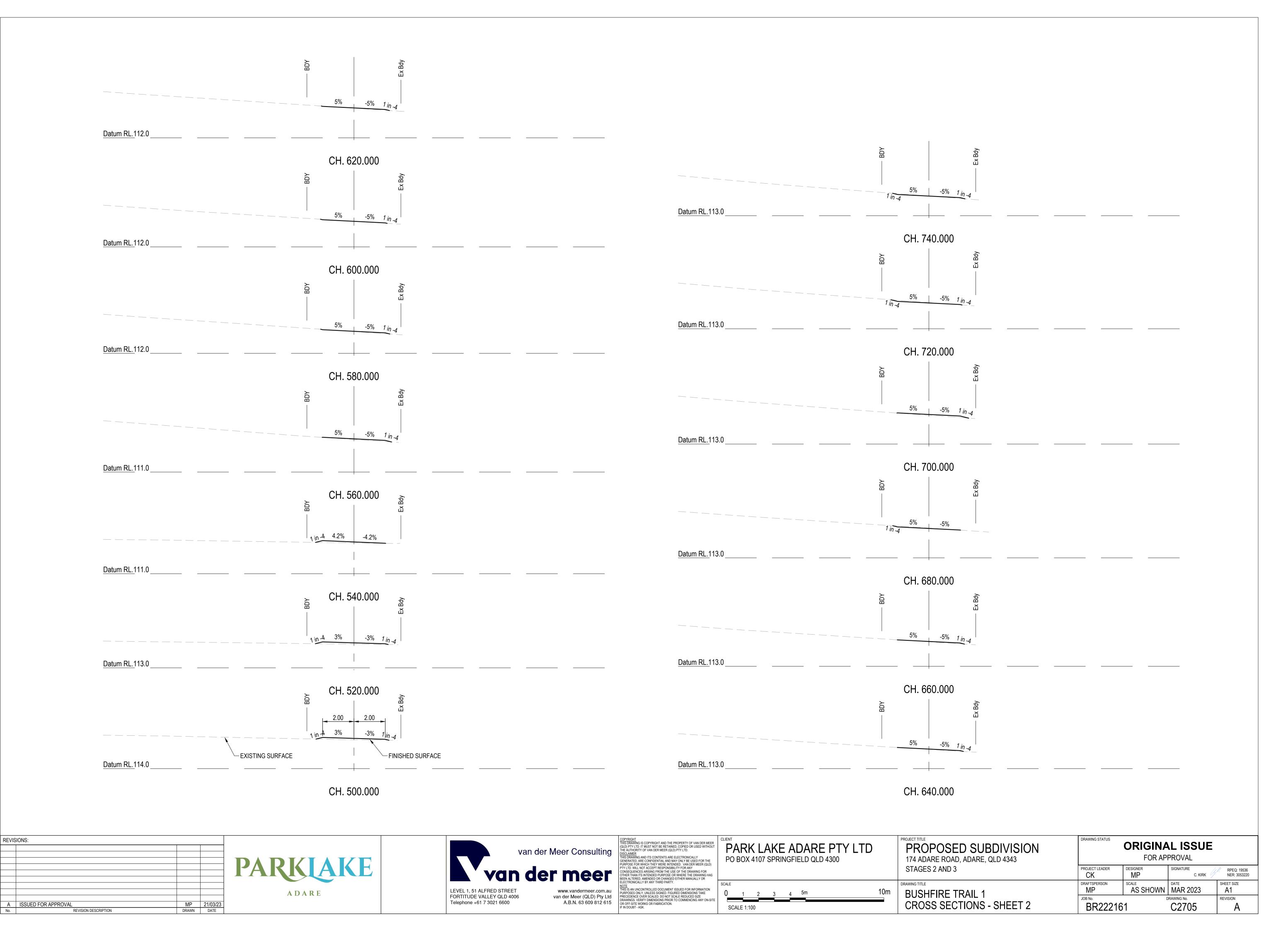
SECTION - SHEET 3 OF 3

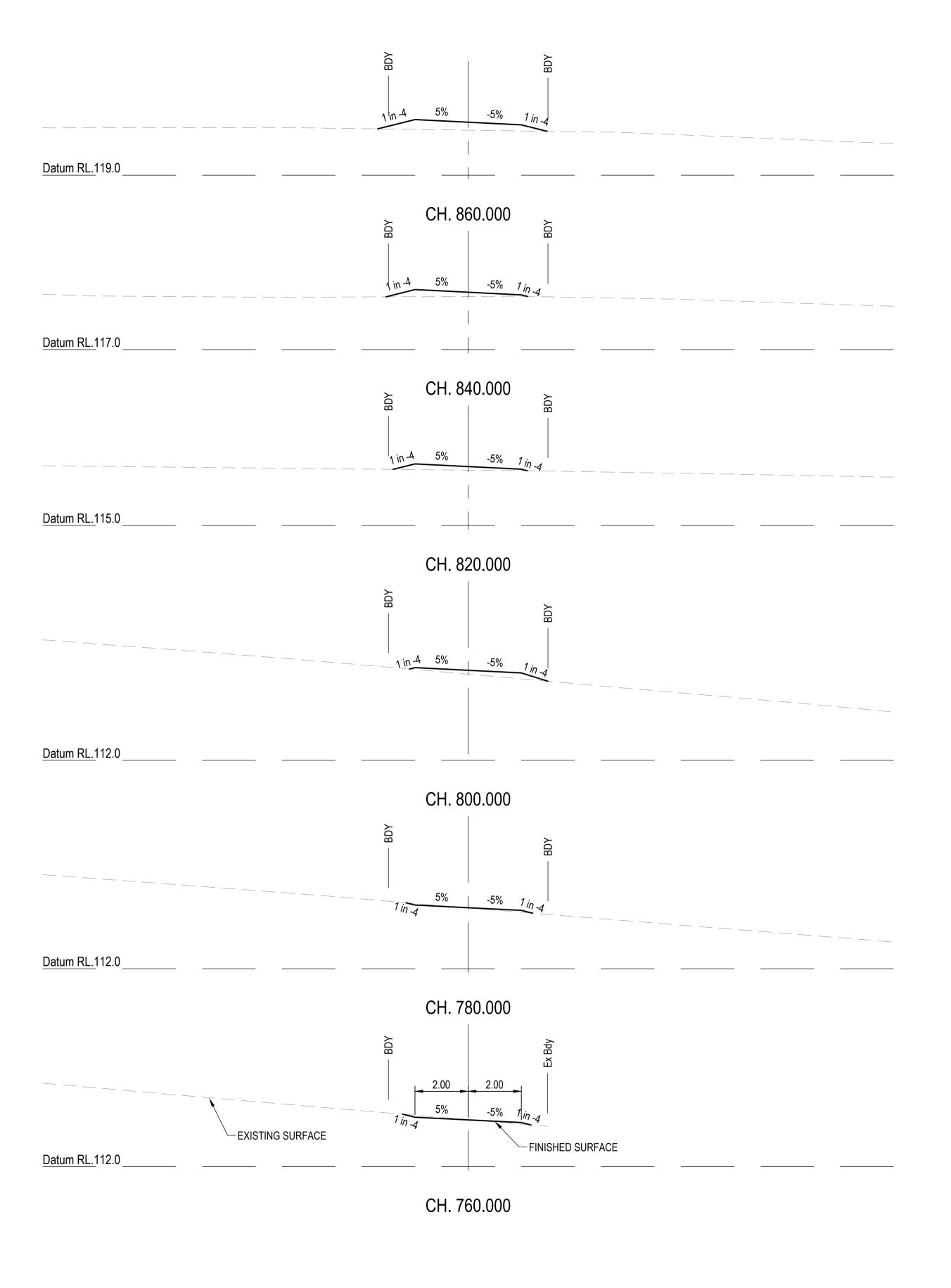
SCALE 1:500 (HORIZONTAL)

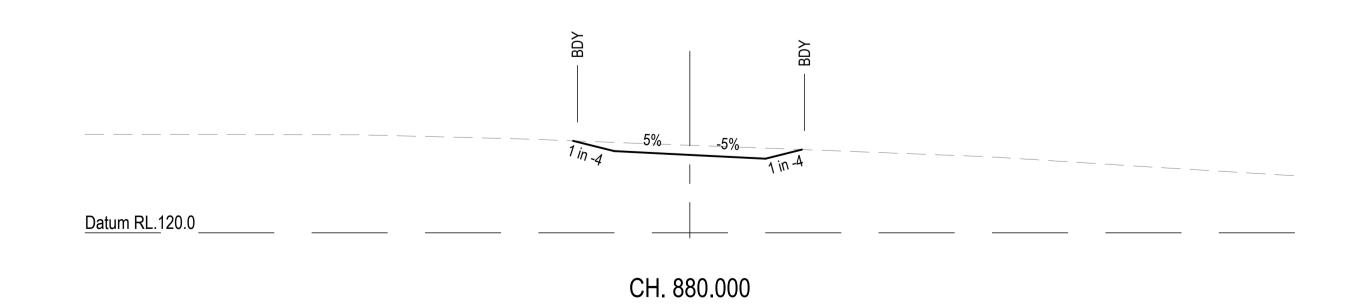
50m

SCALE 1:50 (VE	ERTICAL)		
DRAWING STATUS		AL ISSUE PROVAL	
PROJECT LEADER  CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 305322
DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE  A1
JOB No. BR22216	-	C2703	REVISION









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No.	REVISION DESCRIPTION	DRAWN	DATE				

PARKIAKE ADARE



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-	OTHER THAN ITS INTENDED PURPOSE OR WHERE THE DRAWING HAS BEEN ALTERED, AMENDED OR CHANGED EITHER MANUALLY OR ELECTRONICALLY BY ANY THIRD PARTY. NOTE	
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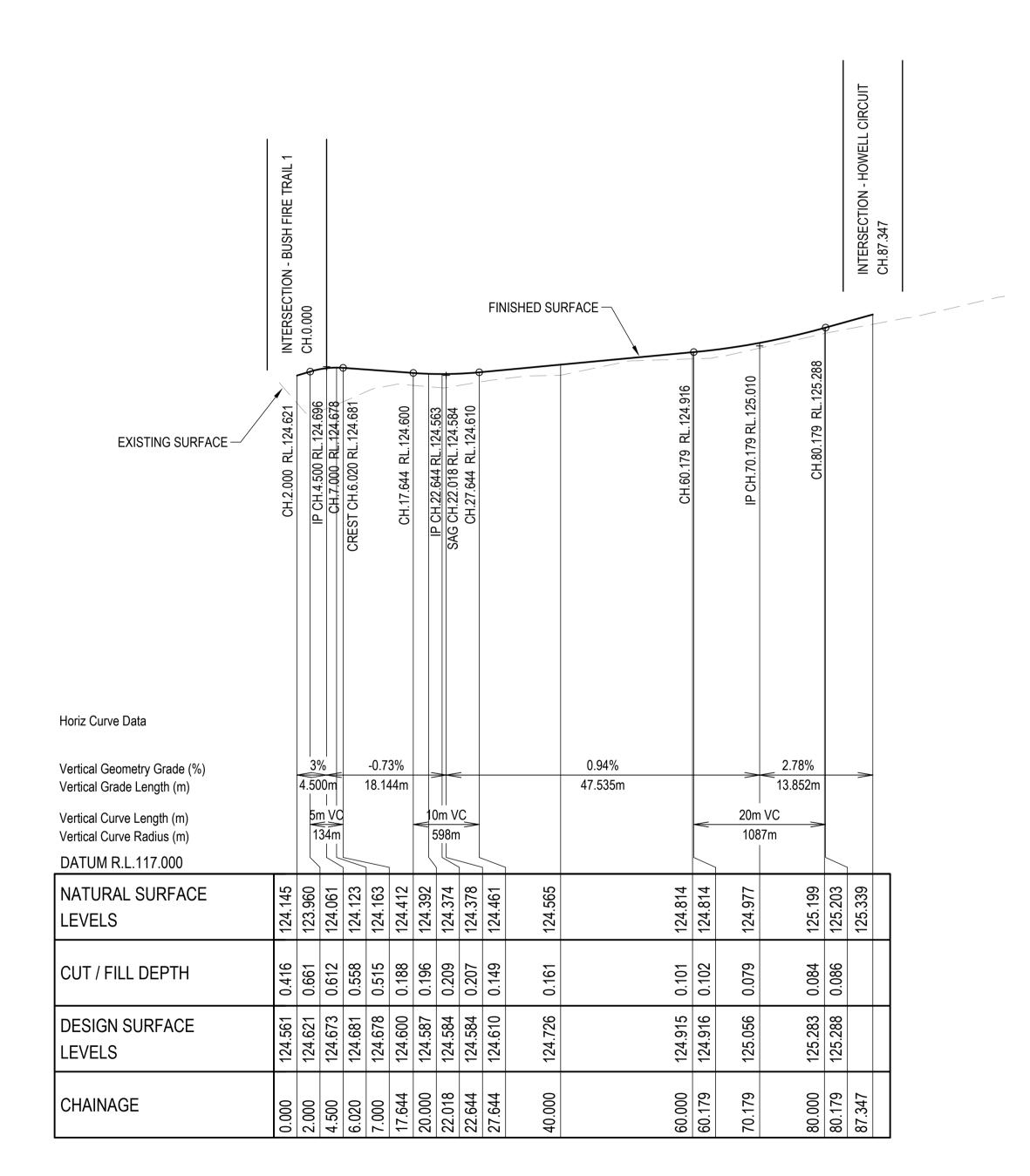
PARK LAKE ADARE PTY LTD	
PO BOX 4107 SPRINGFIELD QLD 4300	
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SCALE 1:100

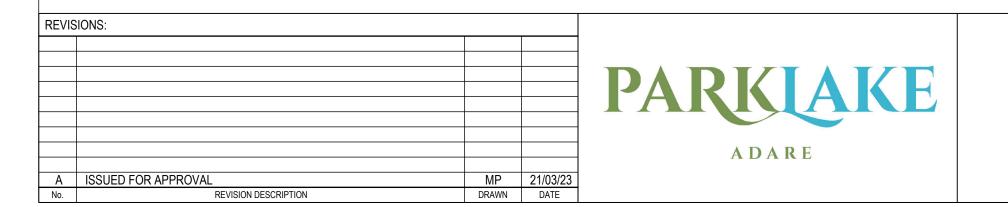
PROJECT TITLE
PROPOSED SUBDIVISION
174 ADARE ROAD, ADARE, QLD 4343
STAGES 2 AND 3
DDAWING TITLE

BUSHFIRE TRAIL 1 CROSS SECTIONS - SHEET 3

ORIGINAL ISSUE FOR APPROVAL					
PROJECT LEADER <b>CK</b>	DESIGNER MP	SIGNATURE C. KI	RK	RPEQ: 19536 NER: 3053220	
DRAFTSPERSON MP	AS SHOWN	MAR 2023		SHEET SIZE A1	
JOB No.	<del>-</del>	DRAWING No.		REVISION	
BR22216	1	C2706		A	



LONGITUDINAL SECTION - BUSHFIRE TRIAL 2
HORIZONTAL SCALE 1:500
VERTICAL SCALE 1:50







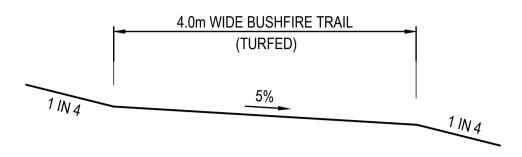
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	SCALE								
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SCALE 1:100

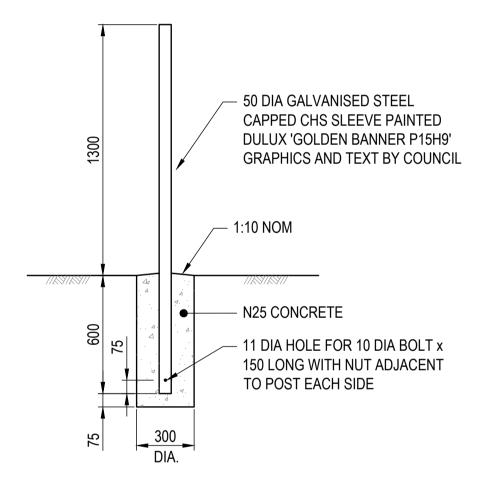
PROPOSED SUBDIVISION 174 ADARE ROAD, ADARE, QLD 4343	ORIGINAL ISSUE FOR APPROVAL			
STAGES 2 AND 3	PROJECT LEADER <b>CK</b>	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220
BUSHFIRE TRAIL 2	DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1
LONGITUDINAL AND CROSS SECTIONS	JOB No. BR22216	_	C2707	REVISION A

Datum RL.123.0 CH. 80.000 1 in -4 5% -5% 1 in -4 Datum RL.123.0 CH. 60.000 -5% 1 in <sub>-4</sub> 1 in -4 5% Datum RL.122.0 CH. 40.000 2.00 5% -5% FINISHED SURFACE EXISTING SURFACE Datum RL.122.0

CH. 20.000



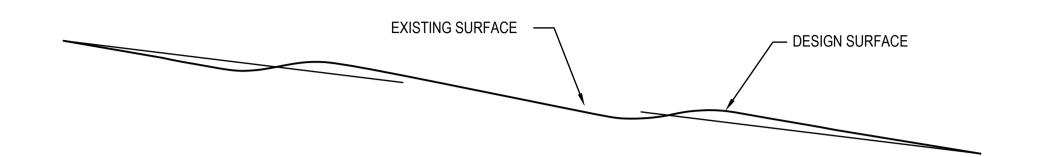
# TYPICAL SECTION - BUSHFIRE TRAIL SCALE 1:50



### PERMANENT FIRE TRAIL POST DETAIL

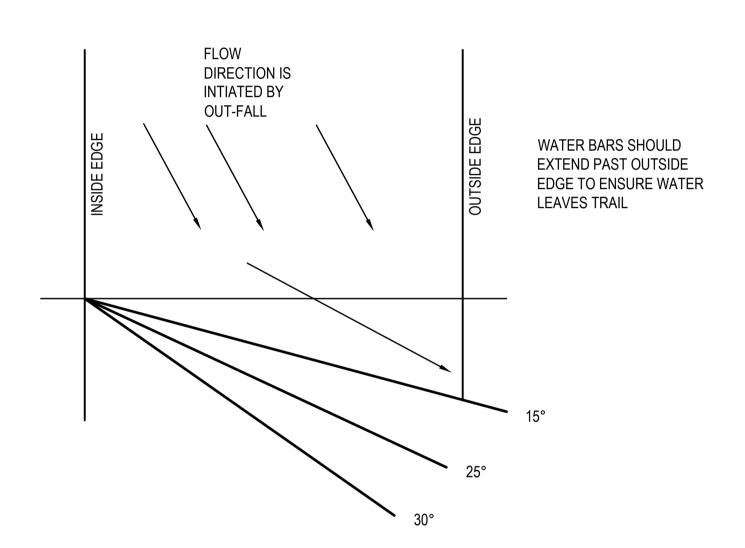
### FIRE TRAIL NOTES

- FIRE TRAIL TO BE IN ACCORDANCE WITH THE APPROVED REPORT "BUSHFIRE RISK ASSESSMENT & MANAGEMENT PLAN" PREPARED BY BUSHLAND PROTECTION SYSTEMS DATED 20TH APRIL 2022.
- 2. PROVIDE A FIRE TRAIL NUMBER SIGN AT EVERY ENTRANCE TO A FIRE TRAIL.
- 3. COUNCIL WILL ALLOCATE TRAIL NUMBERS AND INSTALL NUMBERING ON POST.



### FORMATION OF WATERBAR

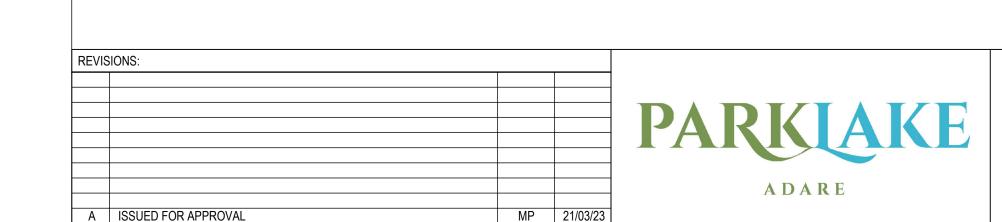
NOT TO SCALE



### WATER BAR ORIENTATION ON THE TRAIL

NOT TO SCALE

TABLE A						
ROAD GRADE	WATER BAR ORIENTATION	SOIL CLASS A WATER BAR SPACING	SOIL CLASS B WATER BAR SPACING	SOIL CLASS C WATER BAR SPACING	WATER BAR HEIGHT	
UP TO 10%	35°	15 - 20m	10 - 12m (apart)	7 - 10m (apart)	0.3 - 0.4m	
11% TO 15%	25°	8 - 10m	7 - 10m	UNDESIRABLE	0.4 - 0.6m	
15% TO 20%	15°	5 - 8m	CONCRETE	CONCRETE	CONCRETE & OUT-FALL	
21% TO 25%	CONCRETE	CONCRETE	CONCRETE	CONCRETE	CONCRETE & OUT-FALL	
26% TO 30%	CONCRETE	CONCRETE	CONCRETE	CONCRETE	CONCRETE & OUT-FALL	
ABOVE 30%	RELOCATE TRAIL ALIGNMENT	RELOCATE TRAIL ALIGNMENT	RELOCATE TRAIL ALIGNMENT	RELOCATE TRAIL ALIGNMENT	RELOCATE TRAIL ALIGNMENT	



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PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300	PROPOSED SUBDING 174 ADARE ROAD, ADARE, QLD 4 STAGES 2 AND 3
ALE	DRAWING TITLE  BUSHFIRE TRAIL DETAI

	SCALE 1:50				
ROPOSED SUBDIVISION ADARE ROAD, ADARE, QLD 4343	DRAWING STATUS	ORIGINAL ISSUE FOR APPROVAL			
GES 2 AND 3	PROJECT LEADER CK	DESIGNER MP	SIGNATURE C. KIRK	RPEQ: 19536 NER: 3053220	
SHFIRE TRAIL DETAILS	DRAFTSPERSON MP	AS SHOWN	MAR 2023	SHEET SIZE A1	
	JOB No. BR22216		C2708	REVISION	