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REVISION DESCRIPTION	DRAWN	DATE
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CIVIL ENGINEERING DRAWINGS					
SHEET NUMBER	SHEET TITLE	REVISION			
C2000	COVER SHEET	A			
C2001	STANDARD NOTES	A			
C2002	SITE PLAN	A			
C2100	EROSION AND SEDIMENT CONTROL PLAN	A			
C2190	EROSION AND SEDIMENT CONTROL DETAILS	A			
C2191	EROSION AND SEDIMENT CONTROL NOTES	A			
C2200	BULK EARTHWORKS PLAN	A			
C2290	BULK EARTHWORKS DETAILS	A			
C2300	PAVEMENT PLAN	A			
C2400	ROADWORKS PLAN	A			
C2430	SIGNS AND PAVEMENT MARKING PLAN	A			
C2450	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 1	A			
C2451	ROAD LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 2	A			
C2452	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 1	A			
C2453	ROAD CROSS SECTIONS - NEWLAND PLACE / HOWELL CIRCUIT - SHEET 2	A			
C2454	ROAD LONGITUDINAL SECTION - HOWELL CIRCUIT - SHEET 1	A			
C2455	ROAD LONGITUDINAL SECTION - HOWELL CIRCUIT - SHEET 2	A			
C2456	ROAD CROSS SECTIONS - HOWELL CIRCUIT - SHEET 1	A			
C2457	ROAD CROSS SECTIONS - HOWELL CIRCUIT - SHEET 2	A			
C2458	ROAD LONGITUDINAL AND CROSS SECTIONS - AXFORD WAY	A			
C2480	INTERSECTION DETAILS	A			
C2490	ROADWORKS DETAILS	A			
C2500	DRAINAGE CATCHMENT PLAN	A			
C2501	DRAINAGE PLAN	A			
C2510	DRAINAGE BIO RETENTION BASIN DETAILS	A			
C2550	DRAINAGE LONGITUDINAL SECTIONS - SHEET 1	A			
C2560	DRAINAGE CALCULATIONS	А			
C2600	WATER RETICULATION COVER SHEET	А			
C2601	WATER RETICULATION PLAN	A			



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PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300			Y LTD		PROJECT TITLE <b>PROPOSED SUBDIVISION</b> 174 ADARE ROAD, ADARE, QLD 4343	DRAWING STATUS	DRAWING STATUS ORIGINAL ISSUE FOR APPROVAL			
		STAGE 2	PROJECT LEADER		SIGNATURE					
RCHITECT	100	200	300	400	500m	DRAWING TITLE	DRAFTSPERSON	SCALE AS SHOWN	DATE NOV 2022	SHEET SIZE
SCALE 1:50							JOB No. BR22202		RAWING No.	

### **GENERAL NOTES**

- G1. ALL LEVELS SHALL BE OBTAINED FROM ESTABLISHED BMS OR SSM.
- G2. CONTRACTOR MUST VERIFY ALL DIMENSIONS AND EXISTING LEVELS ON SITE PRIOR TO COMMENCEMENT OF WORK.
- G3. ALL WORKS ARE TO BE UNDERTAKEN IN ACCORDANCE WITH COUNCIL'S SPECIFICATIONS AND THE DIRECTIONS OF THE SUPERINTENDENT.
- G4. DIMENSIONS MUST NOT BE SCALED FROM DRAWINGS.
- G5. CONTRACTOR TO ENSURE THAT ALL ROADWORKS ARE SMOOTHLY TRANSITIONED TO EXISTING LEVELS FREE FROM ABRUPT CHANGES
- G6. THE CONTRACTOR SHALL ARRANGE ALL SURVEY SETOUT TO BE CARRIED OUT BY A REGISTERED SURVEYOR. FURTHER, THE LOCATION OF RECOVERY MARKS SHOULD BE VERIFIED AND CONFIRMED BY THE CONTRACTOR AND ANY DISCREPANCIES SHOULD BE CLARIFIED IN WRITING WITH THE SUPERINTENDENT PRIOR TO THE COMMENCEMENT OF WORK.
- G7. AT COMPLETION OF WORKS ALL ADJOINING DISTURBED AREAS ARE TO BE REINSTATED TO THE "AS FOUND" CONDITION.
- G8. THE CONTRACTOR SHALL ENSURE ALL AREAS DRAIN WITH A MINIMUM FALL OF 1% (1:100) GRADE TO OUTLETS UNLESS INDICATED OTHERWISE. NO WORKS SHALL CAUSE PONDING OF STORMWATER ON UPSTREAM PROPERTIES OR CONCENTRATE RUNOFF ONTO DOWNSTREAM PROPERTIES.
- G9. THESE PLANS SHALL BE READ IN CONJUNCTION WITH APPROVED LANDSCAPE, ARCHITECTURAL, ELECTRICAL, RETICULATION, WATER AND SEWER DRAWINGS AND SPECIFICATIONS AND OTHER WRITTEN INSTRUCTIONS AS MAY BE ISSUED.
- G10. THE CONTRACTOR SHALL ENSURE THAT ALL PAVEMENTS GRADE EVENLY BETWEEN NOMINATED RL'S ON PLAN AND NO POND OF WATER OCCURS.
- G11. ALL DIMENSIONS ARE IN METERS UNLESS STATED OTHERWISE, ALL LEVELS ARE EXPRESSED IN METERS.
- G12. DURING CONSTRUCTION THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING THE STRUCTURE IN A STABLE CONDITION AND ENSURING NO PART SHALL BE OVERSTRESSED UNDER CONSTRUCTION ACTIVITIES.
- G13. WORKMANSHIP AND MATERIALS ARE TO BE IN ACCORDANCE WITH THE RELEVANT CURRENT S.A.A. CODES INCLUDING ALL AMENDMENTS, AND THE LOCAL STATUTORY AUTHORITIES, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- G14 THE APPROVAL OF A SUBSTITUTION SHALL BE SOUGHT FROM THE ENGINEER BUT IS NOT AN AUTHORIZATION FOR AN EXTRA. ANY EXTRAS INVOLVED MUST BE TAKEN UP WITH THE SUPERINTENDENT BEFORE THE WORK COMMENCES.
- G15 THE CONTRACTOR IS TO EMPLOY A QUALIFIED GEOTECHNICAL ENGINEER AS REQUIRED FOR ALL GEOTECHNICAL ASPECTS OF THE BUILDING WORKS. REFER TO FOUNDATION, GROUNDWORKS AND RETENTION/SHORING NOTES. REFER ALSO TO THE GEOTECHNICAL REPORT FOR THIS PROJECT.
- G16 ORIGINAL SURVEY WAS COMPLETED BY BPLANNED & SURVEYED PH.1300 275 266

### SUBGRADE PREPARATION

- RW1. REMOVE ALL VEGETATION. TOPSOIL AND DELETERIOUS MATERIAL FROM AREA OF PROPOSED BUILDING PLATFORM AND PAVEMENTS.
- RW2. PROOF ROLL EXPOSED SUB GRADE TO ACHIEVE A MINIMUM COMPACTION OF 98% STANDARD MAXIMUM DRY DENSITY (SMDD), DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1.
- RW3. REMOVE ANY SOFT, HEAVING, WET OR UNSTABLE AREAS IDENTIFIED DURING PROOF ROLLING AND REPLACE USING SELECT IMPORTED FILL COMPACTED IN LAYERS NOT EXCEEDING 200mm MEASURED LOOSE TO ACHIEVE A MINIMUM 98% STANDARD MAXIMUM DRY DENSITY.
- RW4. NOTE THAT THE SITE IS UNDERLAIN BY EXISTING SERVICES AND COMPACTION UTILISING VIBRATION MAY NOT BE SUITABLE IN THE VICINITY OF UNDERGROUND SERVICES.
- RW5. ANY FILL REQUIRED TO RAISE LEVELS TO BULK EARTHWORKS TO WITHIN 50mm OF NOMINATED LEVELS IS TO BE APPROVED GRANULAR MATERIAL COMPACTED IN LAYERS NOT EXCEEDING 300mm MEASURED LOOSE TO 98% STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SOMC).
- RW6. THE CONTRACTOR IS TO PROVIDE CERTIFICATION TO THE EFFECT THAT EARTHWORKS COMPACTION TO 98% STANDARD MAXIMUM DRY DENSITY, (AS 1289 E1.1, E4.1) HAS BEEN ACHIEVED, UNLESS OTHERWISE AGREED IN WRITING BY SITE SUPERINTENDENT.
- RW7. THE CONTRACTOR IS TO PROVIDE TO THE SITE SUPERINTENDENT A SURVEY CONFIRMATION FROM A REGISTERED SURVEYOR, CONFIRMING BULK EARTHWORKS LEVELS AS WITHIN +/-50mm OF LEVELS NOMINATED.
- RW8. SUBGRADE REPLACEMENT MATERIAL IS TO CONSIST OF CLEAN, UNCONTAMINATED, WELL-GRADED MATERIAL WITH A MAXIMUM PARTICLE SIZE OF 75mm, WITH 80% LESS THAN 20mm, AND A SOAKED C.B.R. GREATER THAN 10% AND A PLASTICITY INDEX LESS THAN 12.
- RW9. BACK FILLING FOR SERVICE TRENCHES AND REMOVED SERVICES OR PITS OR FOUNDATIONS IS TO USE APPROVED WELL-GRADED GRANULAR MATERIAL WITH MINIMUM VOIDS, (EITHER SELECT INSITU OR IMPORTED FILL), COMPACTION AS SPECIFIED ABOVE.
- RW10. ALL EARTHWORKS TO BE UNDERTAKEN IN ACCORDANCE WITH AS3798-1996: GUIDELINES ON EARTHWORKS FOR COMMERCIAL AND RESIDENTIAL DEVELOPMENTS.

## **GENERAL EARTHWORKS**

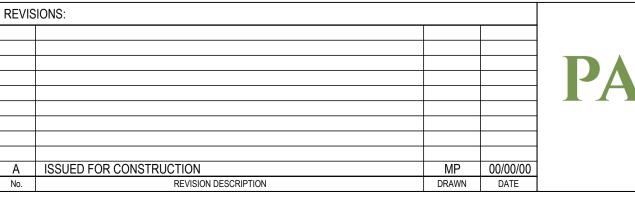
- E1. THE SITE OF THE WORKS SHALL BE PREPARED BY STRIPPING ALL EXISTING TOPSOIL, FILL AND VEGETATION.
- E2. COMPACT SUBGRADE TO 98% OF THE STANDARD MAXIMUM DRY DENSITY WHEN TESTED IN ACCORDANCE WITH AUSTRALIAN STANDARD AS 1289 TESTS E.1.1. OR E.1.2. THE EXPOSED SUBGRADE SHOULD BE PROOF ROLLED TO DETECT ANY SOFT OR WET AREAS WHICH SHOULD BE LOCALLY EXCAVATED AND BACK FILLED WITH SELECTED MATERIAL. THE BACK FILLING MATERIAL SHALL BE IMPORTED GRANULAR FILL OF LOW PLASTICITY, PREFERABLY CRUSHED SANDSTONE, AND TO BE PLACED IN LAYERS NOT EXCEEDING 300mm LOOSE THICKNESS AND COMPACTED TO 98% OF STANDARD MAXIMUM DRY DENSITY WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT. SITE WORKS ARE TO BE BATTERED TO ADJACENT PROPERTY LEVELS.
- E3. NO STORMWATER IS TO POND ON ADJOINING PROPERTIES. THE SITE SHALL BE GRADED AND DRAINED SO THAT STORMWATER WILL BE DIRECTED AWAY FROM THE BUILDING PLATFORM. STORMWATER DRAINAGE SHALL BE PROVIDED AND MAINTAINED THROUGHOUT THE COURSE OF CONSTRUCTION. ALL STORMWATER RUNOFF SHALL BE GRADED AWAY FROM THE DWELLING AND DISPOSED OF VIA SURFACE CATCHDRAINS AND STORMWATER COLLECTION PITS.
- E4. ENSURE ALL RETAINING WALLS ARE CONSTRUCTED WITH ADEQUATE SUBSOIL DRAINAGE.

### GROUND WORKS AND EXCAVATION

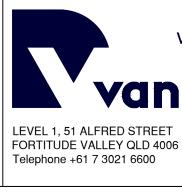
- GW1. ALL GROUND WORKS AND EXCAVATION SHALL BE IN ACCORDANCE WITH GEOTECHNICAL REPORT: 93323.00.R.01.Rev01.
- GW2. SEPARATE AND REMOVE ALL TOPSOIL, NON SOIL MATERIAL, CONCRETE, VEGETATION, BRICKBATS, TIMBER, ROOT AFFECTED SOIL AND EXISTING FILL. STORE TOPSOIL IF REQUIRED.
- GW3. ALL EXCAVATIONS SHALL BE FINISHED CLEAN AND HORIZONTAL AND SHALL NOT UNDERMINE FOOTINGS WALLS etc...
- GW4. PROOF ROLL WITH AN 8 TONNE ROLLER, REPLACE ANY SOFT MATERIAL WITH APPROVED FILL AND RE-COMPACT. GEOTECHNICAL ENGINEER TO APPROVE.
- GW5. THE FILL IS TO BE PLACED AND COMPACTED IN LAYERS OF MAXIMUM LOOSE THICKNESS 300mm.
- GW6. TOP LAYER OF PAVED AREAS TO BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY. GEOTECHNICAL ENGINEER TO VERIFY.
- GW7. ALL PERMANENT EMBANKMENTS TO BE COMPACTED IN 200 mm LAYERS AS PER NOTE GW6 AND AT A MAXIMUM SLOPE OF 1 VERTICAL TO 2.5 HORIZONTAL UNLESS NOTED OTHERWISE. SHOULD DRAINAGE BE REQUIRED THEN SUBMIT DETAILS TO THE ENGINEER.
- GW8. ALL GROUND WORKS SHALL BE TESTED BY AN APPROVED GEOTECHNICAL ENGINEER TO A LEVEL 1 STANDARD IN ACCORDANCE WITH AS 3798 - 1996.
- GW9. ALL EXCAVATIONS TO BE INSPECTED AT REGULAR INTERVALS BY A GEOTECHNICAL ENGINEER.
- GW10. REFER TO ARCHITECTURAL DRAWINGS TO CONFIRM SETOUT OF BUILDINGS, CARPARKS ETC.
- GW11. THE LEVELS SHOWN ARE ONLY RELEVANT TO THE PLAN UPON WHICH THEY ARE SHOWN.
- GW12. ALL CONTOURS AND LEVELS USED TO PRODUCE EARTHWORK DETAILS HAVE BEEN BASED ON SURVEYOR AND ARCHITECTS SURVEY INFORMATION
- GW13. ALL FINISHED FLOOR LEVELS ARE TO BE CONFIRMED BY ARCHITECT.
- GW14. ALL EXISTING SERVICES ARE TO BE CAPPED OFF PRIOR TO ANY WORKS.
- GW15. A PRE-CONSTRUCTION MEETING SHALL BE HELD BETWEEN THE CONTRACTOR, THE GEOTECHNICAL ENGINEER, AND THE EARTHWORKS CONTRACTOR TO UNDERSTAND POTENTIAL DIFFICULTIES AND TO ORGANISE TESTING PROCEDURES. THE CONTRACTOR SHALL CONFIRM TO THE ENGINEER THAT THE MEETING HAS BEEN HELD.

### DRAINAGE NOTES

- D1. PIT LEVELS SHOWN ON STORMWATER DRAINAGE PLANS ARE FOR INFORMATION. EXACT PIT LEVELS TO BE ADJUSTED TO SUIT FALLS IN PAVEMENT/LANDSCAPED AREA.
- D2. PITS GREATER THAN 1.2m DEEP TO BE FITTED WITH STEP IRONS.
- D3. DRAINAGE PIPES SHALL BE BACKFILLED WITH COMPACTED CLEAN SHARP SAND TO 200 ABOVE PIPE OBVERT. ADDITIONAL BACKFILL UNDER ROADS SHALL CONSIST OF CLASS 2 F.C.R. MATERIAL COMPACTED IN 200mm LAYERS TO 98% SMDD. UNDER LANDSCAPED AREAS ADDITIONAL BACKFILL SHALL CONSIST OF GRANULAR MATERIAL COMPACTED IN 200mm LAYERS TO 95% SMDD.
- A 3m LENGTH OF 100 Ø SLOTTED AGRICULTURAL LINE SURROUNDED BY GEOTECH STOCKING SHALL BE PROVIDED ON THE UPSTREAM SIDE OF ALL PITS
- D4. CONCRETE STORMWATER PIPES TO BE CLASS '3' UNDER ROADS AND CLASS '2' IN NON-TRAFFICED AREAS. ALL PIPES GREATER THAN 300Ø ARE TO BE RUBBER RING JOINTS U.N.O.
- D5. CONCRETE PITS GREATER THAN 1.0m DEEP TO BE REINFORCED WITH N12-200 EACH WAY CENTRED, MIN. 300 LAP, CONCRETE - F'c 25MPa
- D6. 150Ø, 225Ø AND 300Ø uPVC PIPES TO BE SEWER GRADE PIPE UNDER TRAFFICABLE PAVEMENT. MIN. 400 COVER UNDER NON-TRAFFICABLE PAVEMENT
- D7. PIT COVERS AND GRATED DRAINS IN TRAFFICABLE PAVEMENT TO BE AS 3996 CLASS D "HEAVY DUTY" AND IN NON-TRAFFICABLE AREAS TO BE AS 3996 CLASS C "LIGHT DUTY".



A D A R E



### UTILITY SERVICES

- S1. CONDUITS TO BE PROVIDED FOR WATER AND ENERGY AUTHORITIES, TELSTRA AND OTHER SERVICES AS REQUIRED.
- S2. THE LOCATIONS OF UNDERGROUND SERVICES SHOWN ON THESE DRAWING'S HAVE BEEN PLOTTED FROM SURVEY AND AUTHORITY INFORMATION. THE SERVICE INFORMATION HAS BEEN PREPARED ONLY TO SHOW THE APPROXIMATE POSITIONS OF ANY KNOWN SERVICES AND MAY NOT BE AS CONSTRUCTED OR ACCURATE.
- VAN DER MEER CANNOT GUARANTEE THAT THE SERVICES INFORMATION S3. SHOWN ON THESE DRAWINGS, ACCURATELY INDICATES THE PRESENCE OR ABSENCE OF SERVICES OR THEIR LOCATION AND WILL ACCEPT NO LIABILITY FOR INACCURACIES IN THE SERVICES INFORMATION SHOWN ARISING FROM ANY CAUSE WHATSOEVER.
- S4. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ESTABLISH THE LOCATION AND LEVEL OF ALL EXISTING SERVICES PRIOR TO THE COMMENCEMENT OF ANY WORK. ANY DISCREPANCIES SHALL BE REPORTED TO THE SUPERINTENDENT. CLEARANCES SHALL BE OBTAINED FROM THE RELEVANT SERVICE AUTHORITY.
- CONTRACTORS SHALL TAKE DUE CARE WHEN EXCAVATING ON SITE S5. INCLUDING HAND EXCAVATION WHERE NECESSARY.
- S6. CONTRACTORS ARE TO CONTACT THE RELEVANT SERVICE AUTHORITY PRIOR TO COMMENCEMENT OF EXCAVATION OR FUTURE WORKS.
- S7 CONTRACTORS ARE TO UNDERTAKE A SERVICES SEARCH PRIOR TO COMMENCEMENT OF WORKS ON SITE. SEARCH RESULTS ARE TO BE KEPT ON SITE AT ALL TIMES.

TELSTRA - DUTY OF CARE NOTE:

TELSTRA'S PLANS SHOW ONLY THE PRESENCE OF CABLES AND PLANT. THEY ONLY SHOW THEIR POSITION RELATIVE TO ROAD BOUNDARIES, PROPERTY FENCES ETC. AT THE TIME OF INSTALLATION AND TELSTRA DOES NOT WARRANT OR UPHOLD THAT SUCH PLANS ARE ACCURATE THEREAFTER DUE TO CHANGES THAT MAY OCCUR OVER TIME. DO NOT ASSUME DEPTH OR ALIGNMENT OF CABLES OR PLANT AS THESE VARY SIGNIFICANTLY.

THE CONTRACTOR HAS A DUTY OF CARE WHEN EXCAVATING NEAR TELSTRA CABLES AND PLANT. BEFORE USING MACHINE EXCAVATORS TELSTRA PLANT MUST FIRST BE PHYSICALLY EXPOSED BY SOFT DIG POT HOLING TO IDENTIFY IT'S LOCATION. TELSTRA WILL SEEK COMPENSATION FOR DAMAGES CAUSED TO IT'S PROPERTY AND LOSSES CAUSED TO TELSTRA AND IT'S CUSTOMERS.

### ELECTRICAL AND GAS NETWORK:

A MINIMUM OF 30 DAYS PRIOR TO COMMENCEMENT OF EXCAVATION WORKS THE SUBCONTRACTOR MUST CONTACT DIAL BEFORE YOU DIG.

### **RETAINING WALL GENERAL**

- GR1. BASE MATERIAL SHALL BE COMPACTED TO MINIMUM 98% STANDARD MAXIMUM DRY DENSITY (SMDD) WITHIN 2% OF STANDARD OPTIMUM MOISTURE CONTENT (SMOC) DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. MINIMUM ALLOWABLE BEARING PRESSURE OF 150 kPa. GEOTECHNICAL ENGINEER EMPLOYED BY CONTRACTOR TO INSPECT AND CONFIRM.
- GR2. DRAINAGE MATERIAL WITHIN AND IMMEDIATELY BEHIND THE WALL SHALL BE 12-20mm CLEAN AGGREGATE. DRAINAGE MATERIAL TO EXTEND A MINIMUM 300mm BEHIND WALL. COMPACT DRAINAGE MATERIAL ALTERNATIVELY, USE NO FINES CONCRETE, AS FOLLOWS:
  - CONCRETE STRENGTH N15.
  - 210kg/m3 PORTLAND CEMENT MAXIMUM AGGREGATE SIZE 20 mm.
  - W/C RATIO 0.45 TO 0.55.
- DENSITY 1600 TO 2000 kg/m3
- GR3. INFILL SOIL SHALL BE CLASS 1 CONTROLLED FILL TO AS4678, OR AS SPECIFIED ON THE DRAWINGS. UNSUITABLE SOILS, SUCH AS HEAVY CLAYS OR ORGANIC SOILS WITH HIGH PLASTICITY, SHALL NOT BE USED IN THE REINFORCED SOIL MASS.
- GR4. SPREAD BACKFILL IN UNIFORM LIFTS OF 200mm UNCOMPACTED THICKNESS. COMPACT TO MINIMUM 95% OF SMDD. COMPACTION WITHIN 1.0 m BEHIND THE WALL SHALL BE ACCOMPLISHED BY USING A HAND-OPERATED PLATE COMPACTOR AND SHALL BEGIN BY RUNNING THE PLATE DIRECTLY ON THE BLOCK. THEN COMPACTING IN PARALLEL PATHS, PROGRESSIVELY AWAY FROM THE WALL FACE.
- GR5. WHERE ROADWAYS OR BUILDING STRUCTURES ARE LOCATED ABOVE THE REINFORCED ZONE, COMPACT TO 98% SMDD WITHIN 2% OF SOMC DETERMINED BY THE STANDARD COMPACTION TEST IN ACCORDANCE WITH CURRENT AUSTRALIAN STANDARD 1289.5.1.1. COMPACTION TESTING SHALL BE TAKEN AT 1.2m BEHIND THE WALL.

### PAVEMENT

- F1. SUBGRADE SHALL BE PREPARED AS OUTLINED IN EARTHWORKS.
- F2. PAVEMENT MATERIAL SHALL CONSIST OF APPROVED OR RIPPED SANDSTONE, NATURAL GRAVEL OR FINE CRUSH ROCK AS PER COUNCIL SPECIFICATION.
- F3. PAVEMENT MATERIALS SHALL BE SPREAD IN LAYERS NOT EXCEEDING 150mm AND NOT LESS THEN 75mm COMPACTED THICKNESS. PAVEMENT MATERIALS SHALL BE SIZED AND OF A STANDARD OUTLINED IN AS1141.
- CRUSHED OR RIPPED SANDSTONE SHALL BE MINUS 75mm NOMINAL SIZE F4. DERIVED FROM SOUND, CLEAN SANDSTONE FREE FROM OVERBURDEN, CLAY SEAMS, SHALE AND OTHER DELETERIOUS MATERIAL
- F5. PAVEMENT MATERIALS SHALL BE COMPACTED BY SUITABLE MEANS TO SATISFY THE FOLLOWING MINIMUM SPECIFICATIONS (AS PER AS1289.52)

DESCRIPTION	MODIFIED DENSITY RATIO
SUB-BASE	98% MDD
BASE COURSE	98% MDD
ASPHALTIC CONCRETE	97% MDD

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AND SUBJECT TO COUNCIL'S CONSTRUCTION SPECIFICATION.

TESTING FOR EACH LAYER SHALL BE UNDERTAKEN BY A N.A.T.A. REGISTERED LABORATORY IN ACCORDANCE WITH AS1289, AT NOT MORE THAN 50m INTERVALS AND A MINIMUM OF TWO PER LAYER. FURTHER FREQUENCY OF TESTING SHALL BE NO LESS THAN THAT REQUIRED BY AS3978-1996.

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### PRIVATE WORKS (SITE CIVIL WORKS)

- PW.01 A COMPLETE SURVEY OF COMPLETED SURFACE INCLUDING SURFACE LEVELS OF STRUCTURES INCLUDING BUT NOT LIMITED TO:
- STORMWATER MANHOLES AND PITS. BIO-RETENTION AREAS, INCLUDING BASE AND TOP OF FILTER TRENCH MATERIAL ANI INSTALLED, IF APPLICABLE.
- WATER SERVICE CONNECTION INCLUDING FITTINGS AND METERS.
- SEWERAGE PROPERTY CONNECTIONS INCLUDING MANHOLES.
- SEWER PUMP STATIONS. IF APPLICABLE
- PW.02 COPIES OF NATA TEST CERTIFICATE RESULTS IN RESPECT OF:
- THE COMPACTION OF FILL INCLUDING COMPACTION OF TRENCH BACKFILL. EARTHWORKS CERTIFICATION FROM GEOTECHNICAL RPEQ INCLUDING LEVEL 1
- CERTIFICATION WHERE REQUIRED. THE SUB-GRADE CBR.
- THE SUB-GRADE COMPACTION.
- THE LOWER SUBBASE (CBR 15) MATERIAL QUALITY.
- THE LOWER SUBBASE (CBR 15) COMPACTION.
- THE SUB-BASE COURSE (CBR 45) MATERIAL QUALITY.
- THE SUB-BASE COURSE (CBR 45) COMPACTION.
- THE BASE COURSE (CBR 80) MATERIAL QUALITY. THE BASE COURSE (CBR 80) COMPACTION.
- THE PRIME OR PRIMER SEAL SPRAY AND APPLICATION RATES.
- THE AC CORE TESTS
- ANY CONCRETE TESTING REQUIRED.
- CCTV VIDEO FOR UNDERGROUND STORMWATER INFRASTRUCTURE WORK.
- PW.03 DURING CONSTRUCTION, DIGITAL PHOTOGRAPHS MUST: BE TAKEN OF COMPLEX CONSTRUCTIONS OR INSTALLATIONS WHICH WILL BE BELOW LEVEL OR NOT VISIBLE AFTER CONSTRUCTION COMPLETION OR AS REQUESTED ON 3
- BE TAKEN PRIOR TO BACKFILLING. INCLUDE A CHAINAGE OR EXACT LOCATION REFERENCE IN THE TITLE OF THE DIGITA FILE.
- BE DATE STAMPED.

### COUNCIL WORKS

## REINFORCED CONCRETE BLOCKWORK

И2.	MINIMUM DURABILITY REQUIREMENTS:

LOCATION	SALT ATTACK RESISTANCE GRADE OF MASONRY UNIT	MORTAR CLASS	DURABILITY CLASS OF WALL TIES AND BUILT-IN COMPONENTS
INTERIOR MASONRY	GENERAL PURPOSE	М3	R3
EXTERIOR MASONRY ABOVE DAMP PROOF COURSE	GENERAL PURPOSE	М3	R3
BELOW DAMP PROOF COURSE OR IN CONTACT WITH GROUND	EXPOSURE	M4	R4

ELEMENT	STRENGTH OF MASONRY UNIT	MORTAR CLASS #
CONCRETE BLOCKWORK (REINF)	f'uc = 15 MPa	M3

- M4. LAY BOTTOM COURSE OF BLOCKS ON FULL MORTAR BED.
- ALL PERPENDS SHALL BE FILLED WITH MORTAR, EXCEPT WEEPHOLES. M5. ALL CORES SHALL BE GROUTED UNLESS NOTED OTHERWISE.
- GROUT FOR CORE FILLING SHALL BE IN ACCORDANCE WITH AS3600, WITH THE FOLL M6. PROPERTIES
- STRENGTH GRADE S20

PARK LAKE ADARE PTY LTD

PO BOX 4107 SPRINGFIELD QLD 4300

ARCHITECT

PROJECT TITLE

STAGE 2

DRAWING TITLE

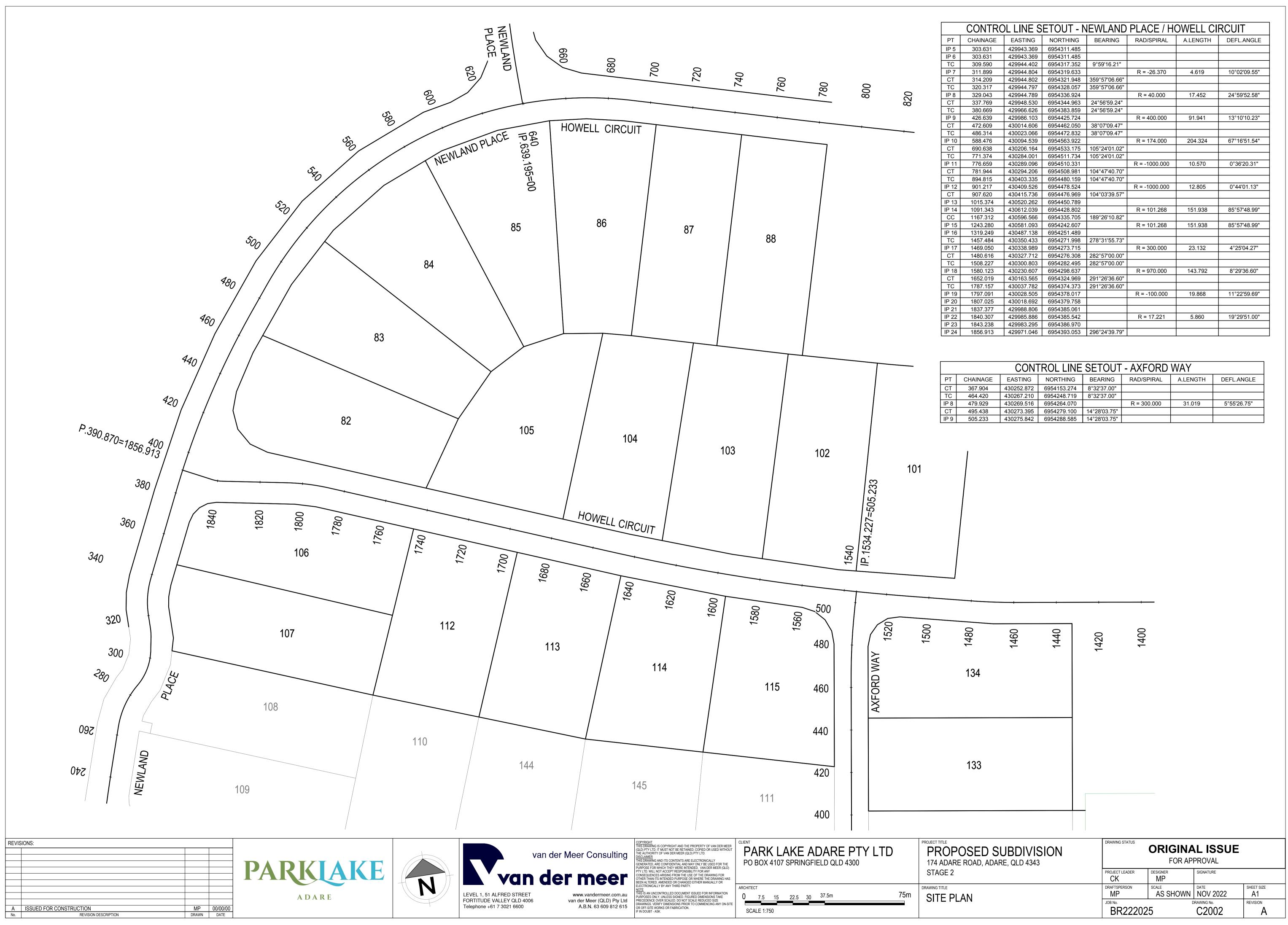
PROPOSE

174 ADARE ROAD,

STANDARD N

AS CONSTR	UCTED				CC	DNCRETE				
PRIVATE WORK	(SITE C	VIL WORK	<u>(S)</u>		C1.	ALL WORKMANSHIP AND M AS 3600 AND OTHER RELE\			NCE WITH	
THE CONTRACTOR SHA MEER PRIOR TO PRACT				IMENTATION TO VAN DER	C2.	CONCRETE SHALL BE SUP ACCORDANCE WITH AS137		APPROVED MANUFAC	CTURER IN	
PW.01 A COMPLETE S STRUCTURES INCLUDIN			CE INCLUDING SURFACE	ELEVELS OF ALL	C3.	CONCRETE SHALL HAVE TH	HE FOLLOWIN	G PARAMETERS:		
<ul> <li>STORMWATER M/</li> <li>BIO-RETENTION A</li> </ul>			OP OF FILTER TRENCH M	IATERIAL AND AREA	ELE	MENT	SLUMP (mm)	AGGREGATE	f'c (MPa)	OTHER REQ
INSTALLED, IF API     WATER SERVICE		NCLUDING FITTIN	IGS AND METERS.		EXT	FERNAL VEHICLE SLAB	+ 80	20	N32	(1)
<ul> <li>SEWERAGE PROF</li> <li>SEWER PUMP ST/</li> </ul>			G MANHOLES.		+ (1)	DENOTES SLUMP AT PLANT DENOTES MAXIMUM BASE		STRAIN 600 x 10 <sup>-6</sup> AT 4	56 DAYS	
PW.02 COPIES OF NA <sup>-</sup> • THE COMPACTION			IN RESPECT OF: ION OF TRENCH BACKFI	ILL.	(1)	(TO AS 1012 PART 13)			DO DATO	
	RTIFICATION F	ROM GEOTECHN	IICAL RPEQ INCLUDING I		C4.	SIZES OF CONCRETE ELEM FINISHES.	IENTS DO NO	T INCLUDE THICKNES	SS OF APPL	.IED
• THE SUB-GRADE	CBR.				C5.	BEAM DEPTHS ARE WRITTE	EN FIRST AND	INCLUDE SLAB THIC	KNESS, IF /	ANY.
<ul> <li>THE SUB-GRADE COMPACTION.</li> <li>THE LOWER SUBBASE (CBR 15) MATERIAL QUALITY.</li> <li>THE LOWER SUBBASE (CBR 15) COMPACTION.</li> </ul>					C6.	HOLES, CHASES OR EMBEE SHALL NOT BE PLACED IN (				
THE SUB-BASE COURSE (CBR 45) MATERIAL QUALITY. THE SUB-BASE COURSE (CBR 45) COMPACTION.					-	OF THE ENGINEER.				
THE BASE COURSE (CBR 80) MATERIAL QUALITY.					C7.	CONDUITS, PIPES AND LIKE COVER, NOR DISPLACE TH			HE CONCR	EIE
THE PRIME OR PRIMER SEAL SPRAY AND APPLICATION RATES. THE AC CORE TESTS.					C8.	CONSTRUCTION JOINTS (C WHERE SHOWN OR SPECIF CONSTRUCTION JOINTS SH	ICALLY APPR	OVED BY THE ENGIN	IEER. ALL	ONLY
<ul> <li>ANY CONCRETE TESTING REQUIRED.</li> <li>CCTV VIDEO FOR UNDERGROUND STORMWATER INFRASTRUCTURE WORK.</li> <li>PW.03 DURING CONSTRUCTION, DIGITAL PHOTOGRAPHS MUST:</li> </ul>					C9.	THE MAXIMUM HEIGHT OF I UNLESS METHOD OF PLACI COLUMNS SHALL NOT BE P	EMENT HAS B	EEN APPROVED BY 1		
BE TAKEN OF COMPLEX CONSTRUCTIONS OR INSTALLATIONS WHICH WILL BE BELOW GROUND LEVEL OR NOT VISIBLE AFTER CONSTRUCTION COMPLETION OR AS REQUESTED ON SITE. BE TAKEN PRIOR TO BACKFILLING. INCLUDE A CHAINAGE OR EXACT LOCATION REFERENCE IN THE TITLE OF THE DIGITAL PHOTO					C10.	CONCRETE SHALL NOT BE P CONCRETE SHALL BE THOP OF MECHANICAL VIBRATIO	ROUGHLY CO		RMS BY ME	ANS
FILE.		LOCATION REFE	ERENCE IN THE TITLE OI	F THE DIGITAL PHOTO	C11.	WHEN THE SHADE TEMPER	ATURE EXCE	,		ACE
BE DATE STAMPED. COUNCIL WORKS						OF CONCRETE SHALL BE S ALIPHATIC ALCOHOL DURIN ACCORDANCE WITH THE M ADEQUATE SUPPLY OF ALI	NG CONCRET	E PLACEMENT AND F ER'S RECOMMENDAT	INISHING II IONS. ENSI	JRING
CW.01 TO PROVIDE AS	CONSTRUCTE	D INFORMATION	AS PER LOCAL COUNCI	L SUBMISSION		CONCRETE WORK.				
GUIDELINES REQUIREM		CRETE BI	LOCKWORK		C12.	CURING OF CONCRETE SH OPERATIONS AND SHALL B AN APPROVED PROPRIETA AS 3799 AND COMPATIBLE PONDING WITH POTABLE W THE CONTRACTOR TO SUB APPROVAL OF THE ENGINE	e Maintaine Ry Curing C With the Pr /Ater. Mit Proposi	D FOR A MINIMUM OF OMPOUND IN ACCOF OPOSED FINISH OR (	7 DAYS US RDANCE WI CONTINUOU	SING TH
M1. CONCRETE BLOCKS SHALL BE BORAL 'CORE FILL BLOCKS', DOUBLE-U TYPE, OR SIMILAR APPROVED.				YPE, OR SIMILAR	C13. ALL CONCRETE DELIVERED TO SITE SHALL BE SUBJECT TO PROJECT ASSESSMENT IN ACCORDANCE WITH AS 1379.					
M2. MINIMUM DURAB			DURABILITY CLASS		C14.	THE CONTRACTOR SHALL I	NOMINATE A (	CONCRETE DELIVER		
LOCATION	RESISTANC GRADE OF	E CLASS	OF WALL TIES AND BUILT-IN			WHO SHALL BE A SUITABLE THE ENGINEER, TO MONITO CONCRETE FOR EACH POU	OR THE DELIV	ERY AND PLACING O	F THE	LOF
INTERIOR MASONRY	MASONRY UI GENERAL		COMPONENTS R3			MANUFACTURER SHALL SA TYPE OF CONCRETE SUPP	MPLE AND TE	EST FOR DRYING SHE	RINKAGE EA	ACH
EXTERIOR MASONRY	PURPOSE GENERAL	M3	R3			COURSE OF THE PROJECT NATA TEST CERTIFICATES	OR FOR EVEN	RY 1000 CUBIC METR RWARDED TO THE EN	ES PLACED	
ABOVE DAMP PROOF COURSE	PURPOSE					RESULTS OF THESE TESTS				
BELOW DAMP PROOF COURSE OR IN CONTACT WITH GROUND	EXPOSUR	E M4	R4		C15.	ARRANGE FOR A NATA REC	GISTERED TES	STING LABORATORY	TO TAKE	
M3. MINIMUM STRENGT		ITS:	1			SAMPLES OF AND TEST CO STRENGTH (SLABS ON GRO			AURAL IEN	NOILE
		STRENGTH OF	MORTAR CLASS #			COMPRESSION TEST SAMP (4 STANDARD CYLINDERS F	OR POST-TE	NSIONED CONCRETE		
CONCRETE BLOCKWO	RK (REINF)	MASONRY UNIT				COMPRESSIVE STRENGTH ONE (1) CYLINDER AT 3 DA				
	· · /		R DURABILITY (REFER N	OTE M2).		ONE (1) CYLINDER AT 3 DAY ONE (1) CYLINDER AT 7 DAY TWO (2) CYLINDERS AT 28 I	YS.			
IAY BOTTOM CO           ALL         PERPENDS SHAL	URSE OF BLOC L BE FILLED W	KS ON FULL MOF	RTAR BED. CEPT WEEPHOLES.			THE MINIMUM NUMBER OF		ES SHALL BE AS FOL	LOWS:	
<ol> <li>ALL CORES SHAI</li> <li>GROUT FOR COF PROPERTIES:</li> </ol>			) OTHERWISE. DANCE WITH AS3600, WI	TH THE FOLLOWING		IN COLUMNS/WALLS:	SAMPLE PEF	R TRUCK		
PRIPPRIPS						ALL OTHER CONCRETE OF 1 TRUCK PER DAY	ANY ONE TYP - 1 SAMPL			
	SIZE TUMM		RTICAL CONTROL JOINT	S IN MASONRY		2 TO 5 TRUCKS PER DAY 6 TO 10 TRUCKS PER DAY 10 TO 20 TRUCKS PER DAY	- 2 SAMPL - 3 SAMPL - 4 SAMPLI	ES ES ES		
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> </ul>	imm TENT 300kg/m <sup>3</sup>	M7. PROVIDE VEI				FOR EACH ADDITIONAL 10				
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON</li> </ul>	imm TENT 300kg/m <sup>3</sup>	M7. PROVIDE VEF JOINT WIDTH	MAX JOINT SPACING			SLUMP: 1 SAMPLE PE		TIME OF POLIRING		
STRENGTH GRADE MAX. AGGREGATE SLUMP 230mm ± 25 MIN. CEMENT CON WALLS AS FOLLOV	imm TENT 300kg/m <sup>3</sup> /S :		MAX JOINT SPACING 12m		C16.			TIME OF POURING.		
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON WALLS AS FOLLOV</li> <li>WALL TYPE</li> <li>CONCRETE BLOCKWO</li> <li>M7. AT CORNERS, CO</li> </ul>	5mm TENT 300kg/m <sup>3</sup> /S : RK (REINF) DNTROL JOINTS	JOINT WIDTH 15mm S SHALL BE WITH	12m IN HALF THE SPECIFIED		C16.					
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON WALLS AS FOLLOW</li> <li>WALL TYPE</li> <li>CONCRETE BLOCKWO</li> <li>M7. AT CORNERS, CO FROM THE CORN PROVIDE JOINTS</li> </ul>	5mm TENT 300kg/m <sup>3</sup> /S : RK (REINF) RK (REINF) DNTROL JOINTS ER. JOINTS SH TO MATCH JO	JOINT WIDTH 15mm S SHALL BE WITH ALL BE SEALED V NTS IN SUPPORT	12m IN HALF THE SPECIFIED WITH AN APPROVED FLE	EXIBLE SEALANT.	C16.					
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON WALLS AS FOLLOV</li> <li>WALL TYPE</li> <li>CONCRETE BLOCKWO</li> <li>M7. AT CORNERS, CO FROM THE CORN PROVIDE JOINTS</li> <li>M8. PROVIDE CLEANG ALL MORTAR PRO BE PROVIDED AE</li> </ul>	INTROL JOINTS FR. (REINF) INTROL JOINTS ER. JOINTS SH TO MATCH JO DUT OPENINGS DTRUSIONS BE OVE EACH HO	JOINT WIDTH 15mm S SHALL BE WITH ALL BE SEALED V NTS IN SUPPORT S AT THE BASE OF FORE GROUTING RIZONTAL POUR	12m IN HALF THE SPECIFIED WITH AN APPROVED FLE FING SLABS. F ALL REINFORCED COR G. ADDITIONAL CLEANOU BREAK.	EXIBLE SEALANT. RES AND REMOVE JT OPENINGS SHALL	C16.					
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON WALLS AS FOLLOV</li> <li>WALL TYPE</li> <li>CONCRETE BLOCKWO</li> <li>M7. AT CORNERS, CO FROM THE CORN PROVIDE JOINTS</li> <li>M8. PROVIDE JOINTS</li> <li>M8. PROVIDE CLEANS ALL MORTAR PRO BE PROVIDED AE</li> <li>M9. MAXIMUM HEIGH AND 0.8m FOR 14</li> </ul>	ITENT 300kg/m <sup>3</sup> /S : RK (REINF) DNTROL JOINTS ER. JOINTS SH TO MATCH JO DUT OPENINGS DTRUSIONS BE OVE EACH HO T OF POUR FO 0 BLOCKWORK	JOINT WIDTH 15mm S SHALL BE WITH ALL BE SEALED V NTS IN SUPPORT AT THE BASE OF FORE GROUTING RIZONTAL POUR R GROUTING SHA	12m IN HALF THE SPECIFIED WITH AN APPROVED FLE FING SLABS. F ALL REINFORCED COR G. ADDITIONAL CLEANOL	EXIBLE SEALANT. RES AND REMOVE JT OPENINGS SHALL OR 190 LOCKWORK,	C16.					
<ul> <li>STRENGTH GRADE</li> <li>MAX. AGGREGATE</li> <li>SLUMP 230mm ± 25</li> <li>MIN. CEMENT CON WALLS AS FOLLOV</li> <li>WALL TYPE</li> <li>CONCRETE BLOCKWO</li> <li>M7. AT CORNERS, CO FROM THE CORN PROVIDE JOINTS</li> <li>M8. PROVIDE JOINTS</li> <li>M8. PROVIDE CLEANS ALL MORTAR PRO BE PROVIDED AE</li> <li>M9. MAXIMUM HEIGH</li> </ul>	Simm TENT 300kg/m <sup>3</sup> /S : RK (REINF) RK (REINF) ONTROL JOINTS ER. JOINTS SH TO MATCH JOI DUT OPENINGS OTRUSIONS BE OVE EACH HOI T OF POUR FOI 0 BLOCKWORK QUENT POUR.	JOINT WIDTH 15mm S SHALL BE WITH ALL BE SEALED V NTS IN SUPPORT AT THE BASE OF FORE GROUTING RIZONTAL POUR R GROUTING SHA C STOP POUR 50	12m IN HALF THE SPECIFIED WITH AN APPROVED FLE FING SLABS. F ALL REINFORCED COR G. ADDITIONAL CLEANOL BREAK. ALL NOT EXCEED 3.6m F0 mm BELOW TOP OF BLO	EXIBLE SEALANT. RES AND REMOVE JT OPENINGS SHALL OR 190 LOCKWORK, ICK TO PROVIDE	C16.					

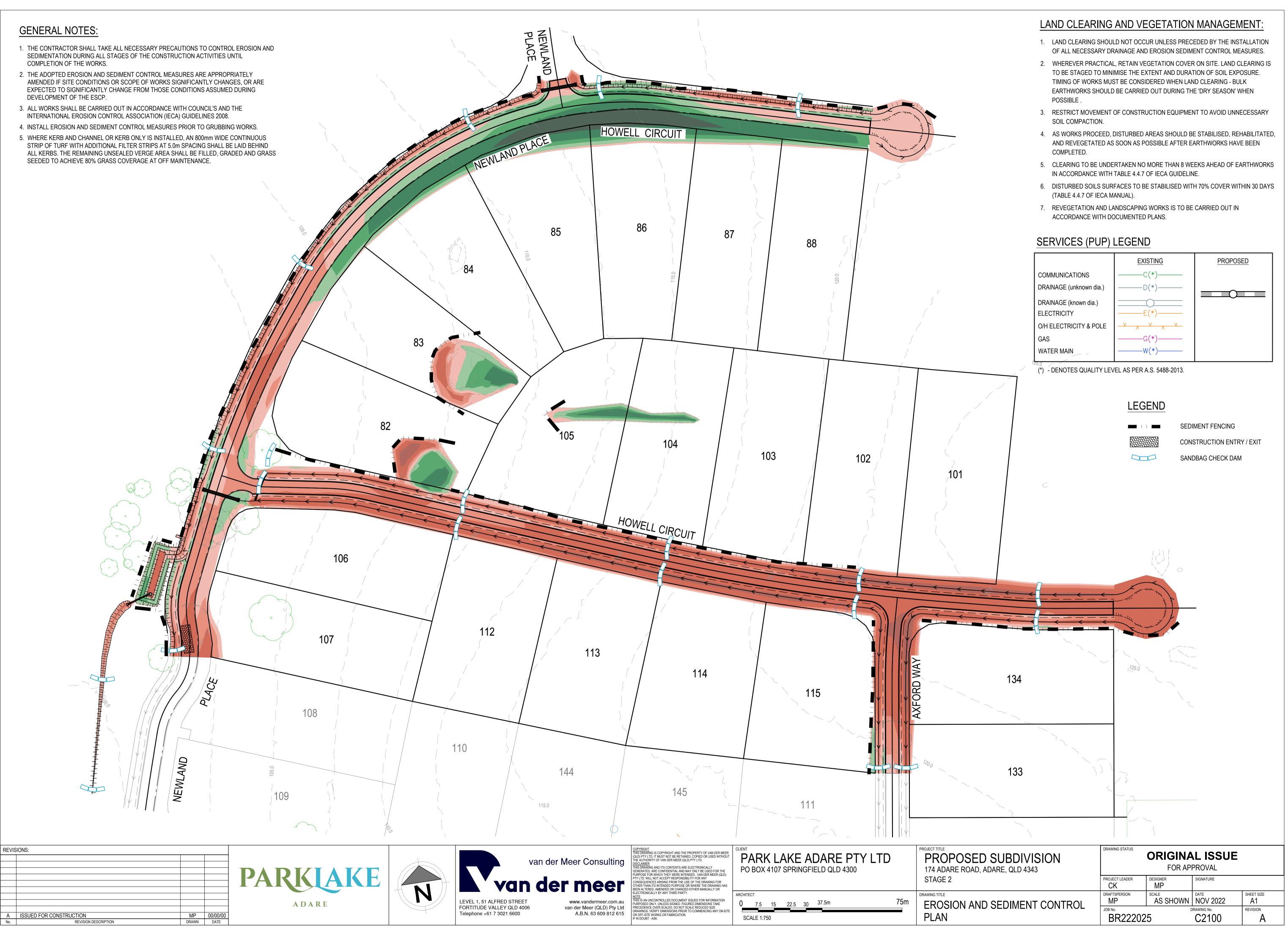
D SUBDIVISION ADARE, QLD 4343	DRAWING STATUS		AL ISSUE PROVAL			
	PROJECT LEADER	designer MP	SIGNATURE			
NOTES	draftsperson	SCALE AS SHOWN	DATE NOV 2022	SHEET SIZE		
NOILO	<sup>ЈОВ №.</sup> BR22202		rawing No.	REVISION		

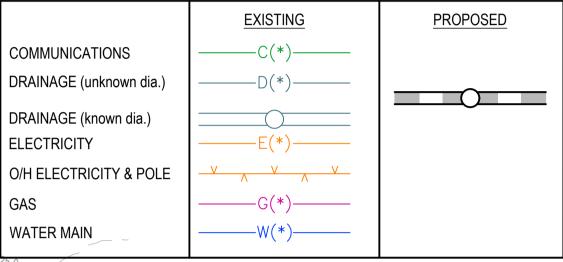


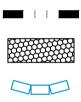
AGE	EASTING	NORTHING	BEARING	RAD/SPIRAL	A.LENGTH	DEFL.ANGLE
631	429943.369	6954311.485				
631	429943.369	6954311.485				
590	429944.402	6954317.352	9°59'16.21"			
399	429944.804	6954319.633		R = -26.370	4.619	10°02'09.55"
209	429944.802	6954321.948	359°57'06.66"			
317	429944.797	6954328.057	359°57'06.66"			
)43	429944.789	6954336.924		R = 40.000	17.452	24°59'52.58"
769	429948.530	6954344.963	24°56'59.24"			
69	429966.626	6954383.859	24°56'59.24"			
639	429986.103	6954425.724		R = 400.000	91.941	13°10'10.23"
609	430014.606	6954462.050	38°07'09.47"			
314	430023.066	6954472.832	38°07'09.47"			
176	430094.539	6954563.922		R = 174.000	204.324	67°16'51.54"
638	430206.164	6954533.175	105°24'01.02"			
374	430284.001	6954511.734	105°24'01.02"			
659	430289.096	6954510.331		R = -1000.000	10.570	0°36'20.31"
944	430294.206	6954508.981	104°47'40.70"			
315	430403.335	6954480.159	104°47'40.70"			
217	430409.526	6954478.524		R = -1000.000	12.805	0°44'01.13"
620	430415.736	6954476.969	104°03'39.57"			
374	430520.262	6954450.789				
343	430612.039	6954428.802		R = 101.268	151.938	85°57'48.99"
312	430596.566	6954335.705	189°26'10.82"			
280	430581.093	6954242.607		R = 101.268	151.938	85°57'48.99"
249	430487.138	6954251.489				
484	430350.433	6954271.998	278°31'55.73"			
050	430338.989	6954273.715		R = 300.000	23.132	4°25'04.27"
616	430327.712	6954276.308	282°57'00.00"			
227	430300.803	6954282.495	282°57'00.00"			
123	430230.607	6954298.637		R = 970.000	143.792	8°29'36.60"
019	430163.565	6954324.969	291°26'36.60"			
157	430037.782	6954374.373	291°26'36.60"			
091	430028.505	6954378.017		R = -100.000	19.868	11°22'59.69"
025	430018.692	6954379.758				
377	429988.806	6954385.061				
307	429985.886	6954385.542		R = 17.221	5.860	19°29'51.00"
238	429983.295	6954386.970				
913	429971.046	6954393.053	296°24'39.79"			

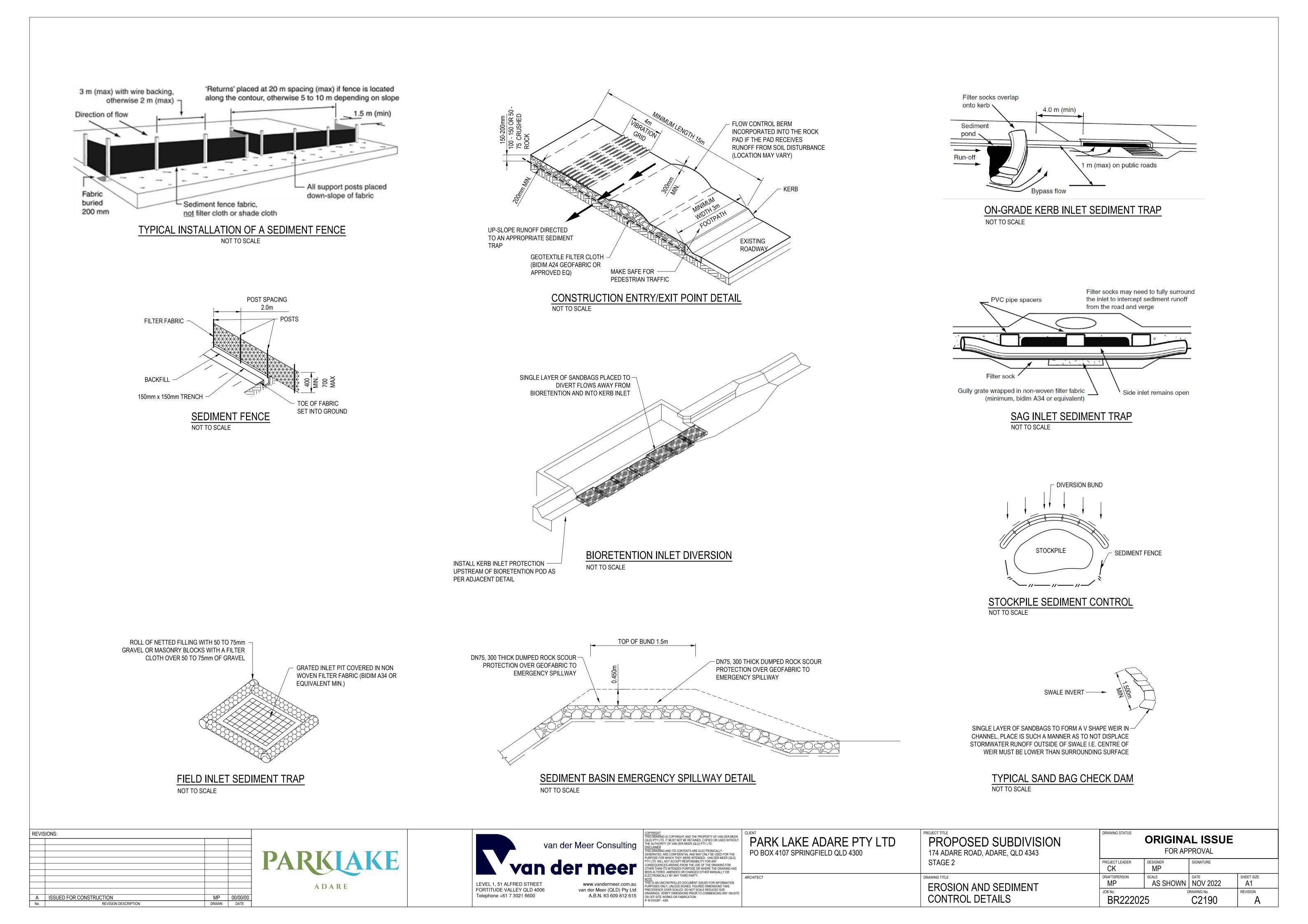
	CONTROL LINE SETOUT - AXFORD WAY								
AGE	EASTING	NORTHING	BEARING	RAD/SPIRAL	A.LENGTH	DEFL.ANGLE			
04	430252.872	6954153.274	8°32'37.00"						
20	430267.210	6954248.719	8°32'37.00"						
29	430269.516	6954264.070		R = 300.000	31.019	5°55'26.75"			
38	430273.395	6954279.100	14°28'03.75"						
33	430275.842	6954288.585	14°28'03.75"						
33	430275.842	6954288.585	14°28'03.75"						

- SEDIMENTATION DURING ALL STAGES OF THE CONSTRUCTION ACTIVITIES UNTIL
- EXPECTED TO SIGNIFICANTLY CHANGE FROM THOSE CONDITIONS ASSUMED DURING DEVELOPMENT OF THE ESCP.
- INTERNATIONAL EROSION CONTROL ASSOCIATION (IECA) GUIDELINES 2008.
- STRIP OF TURF WITH ADDITIONAL FILTER STRIPS AT 5.0m SPACING SHALL BE LAID BEHIND









## SEDIMENT FENCE

### MATERIALS

- 1. FABRIC: POLYPROPYLENE, POLYAMIDE, NYLON, POLYESTER OR POLYETHYLENE WOVEN OR NON-WOVEN FABRIC, AT LEAST 700mm IN WIDTH AND A MINIMUM UNIT WEIGHT OF 140GSM.
- 2. SUPPORT POSTS/STAKES AND STEEL STAR PICKETS SUITABLE FOR ATTACHING FABRIC.

### INSTALLATION

- 1. WHERE POSSIBLE INSTALL SEDIMENT FENCE AT LEAST 2m FROM THE TOE OF ANY FILLING OPERATIONS THAT MAY RESULT IN SHIFTING SOIL/FILL DAMAGING THE FENCE.
- 2. ENSURE THE EXTREME ENDS OF THE FENCE ARE TURNED UP THE SLOPE AT LEAST 1.5m OR AS NECESSARY TO MINIMISE WATER BYPASSING AROUND THE FENCE.
- 3. ENSURE THE SEDIMENT FENCE IS INSTALLED IN A MANNER THAT AVOIDS THE CONCENTRATION OF FLOW ALONG THE FENCE AND THE UNDESIRABLE DISCHARGE OF WATER AROUND THE ENDS OF THE FENCE.
- 4. IF THE SEDIMENT FENCE IS TO BE INSTALLED ALONG THE EDGE OF THE EXISTING TREES, ENSURE CARE IS TAKEN TO PROTECT THE TREES AND THEIR ROOT SYSTEMS DURING INSTALLATION OF THE FENCE.
- 5. UNLESS DIRECTED BY THE SITE SUPERVISOR OR THE APPROVED PLANS, EXCAVATE A 200mm WIDE BY 200mm DEEP TRENCH ALONG THE PROPOSED FENCE LINE, PLACING THE EXCAVATED MATERIAL ON THE UP-SLOPE SIDE OF THE TRENCH.
- 6. ALONG THE LOWER SIDE OF THE TRENCH, APPROPRIATELY SECURE THE STAKES INTO THE GROUND SPACED NO GREATER THAN 3m IF SUPPORTED BY A TOP SUPPORT WIRE OR WEIR MESH BACKING, OTHERWISE NO GREATER THAN 2m.
- 7. WHEREVER POSSIBLE, CONSTRUCT THE SEDIMENT FENCE FROM A CONTINUOUS ROLL OF FABRIC. TO JOIN FABRIC ATTACH EACH END OF TWO OVERLAPPING STAKES WITH THE FABRIC FOLDING AROUND THE ASSOCIATED STAKE ONE TURN AND WITH TWO STAKES TIED TOGETHER WITH THE WIRE METHOD OR OVERLAP THE FABRIC TO THE NEXT ADJACENT SUPPORT POST.
- 8. SECURELY ATTACH THE FABRIC TO THE SUPPORT POSTS USING 25 X 12.5mm STAPLES, OR TIE WIRE AT MAXIMUM 150mm SPACING.
- 9. SECURELY ATTACH THE FABRIC TO THE SUPPORT WIRE/MESH (IF ANY) AT A MAXIMUM SPACING OF 1m.
- 10. ENSURE THE COMPLETED SEDIMENT FENCE IS AT LEAST 450mm, BUT NOT MORE THAN 700mm HIGH. IF A SPILL THROUGH WEIR IS INSTALLED, ENSURE THE CREST OF THE WEIR IS AT LEAST 300mm ABOVE GROUND LEVEL.
- 11. BACKFILL THE TRENCH AND TAMP THE FILL TO FIRMLY ANCHOR THE BOTTOM OF THE FABRIC AND MESH TO PREVENT WATER FROM FLOWING UNDER THE FENCE.
- 12. IF IT IS NOT POSSIBLE TO ANCHOR THE FABRIC IN AN EXCAVATED TRENCH, THEN USE A CONTINUOUS LAYER OF SAND OR AGGREGATE TO HOLD THE FABRIC FIRMLY ON THE GROUND.

### MAINTENANCE

**REVISIONS:** 

A ISSUED FOR CONSTRUCTION

REVISION DESCRIPTION

- 1. INSPECT THE SEDIMENT FENCE AT LEAST WEEKLY AND AFTER ANY SIGNIFICANT RAIN. MAKE NECESSARY REPAIRS IMMEDIATELY.
- 2. REPAIR ANY TORN SECTIONS WITH A CONTINUOUS PIECE OF FABRIC FROM POST TO POST.
- 3. WHEN MAKING REPAIRS, ALWAYS RESTORE THE SYSTEM TO ITS ORIGINAL CONFIGURATION UNLESS AN AMENDED LAYOUT IS REQUIRED OR SPECIFIED.
- IF THE FENCE IS SAGGING BETWEEN STAKES, INSTALL ADDITIONAL SUPPORT POSTS.
- 5. REMOVE ACCUMULATED SEDIMENT IF THE SEDIMENT DEPOSIT EXCEEDS A DEPTH OF 1/3 THE HEIGHT OF THE FENCE.

MP 00/00/00

DATE

DRAWN

6. DISPOSE OF SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

## SEDIMENT BASINS

### GENERAL

- 1. SEDIMENT BASIN TO BE LOCATED ABOVE THE 5YR FLOOD LINE. INSTALL SEDIMENT BASINS FOR ALL CATCHMENTS ACROSS THE PROJECT AREA.
- 2. MATERIALS USED IN THE CONSTRUCTION OF SEDIMENT BASINS SHOULD NOT HAVE AN EMERSON NUMBER OF 3 OR ABOVE (I.E. DISPERSIVE SOILS SUCH AS THE SUBSOILS THAT CAN BE ENCOUNTERED AT THE SITE CANNOT BE USED TO CONSTRUCT SEDIMENT BASINS).
- 3. A " FULL OF SEDIMENT" MARKER MUST BE PLACED IN THE SEDIMENT BASIN TO SHOW THE DESIGN DEPTH OF THE SOIL/STORAGE ZONE VOLUME AND TO INDICATE WHEN REMOVAL OF THE SEDIMENT IS TO BE CARRIED OUT
- 4. CONSTRUCTED SEDIMENT BASINS TO BE FULLY OPERATIONAL THROUGHOUT THE CONSTRUCTION PERIOD AND UNTIL THE BASINS CATCHMENT AREA ACHIEVES 70% GROUND COVER ON ALL SOIL SURFACES.
- 5. FLOCCULATION REQUIREMENTS TO BE IN ACCORDANCE WITH TABLE B17 OF THE IECA GUIDELINES. IN GENERAL 32kg OF GYPSUM TO BE ADDED TO 100m<sup>3</sup> OF STORED WATER.

### MAINTENANCE

- 1. INSPECT THE SEDIMENT BASIN DURING THE FOLLOWING PERIODS AS STATED WITHIN PAGE B.52 OF THE IECA GUIDELINES:
- 1.1. DURING CONSTRUCTION TO DETERMINE WHETHER MACHINERY, FALLING TREES OR CONSTRUCTION ACTIVITY HAS DAMAGED ANY COMPONENT OF THE SEDIMENT BASIN. IF DAMAGE HAS OCCURRED, REPAIR IT.
- 1.2. AFTER EACH RUNOFF EVENT. INSPECT THE EROSION DAMAGE AT FLOW ENTRY AND EXIT POINTS. IF DAMAGE HAS OCCURRED, MAKE THE NECESSARY REPAIRS. 1.3. AT LEAST WEEKLY DURING THE NOMINATED WET SEASON (IF ANY) OTHERWISE AT
- LEAST FORTNIGHTLY. PRIOR TO, AND IMMEDIATELY AFTER, PERIODS OF "STOP WORK" OR SITE "SHUTDOWN"
- 2. CLEAN OUT ACCUMULATED SEDIMENT WHEN IT REACHES THE MARKER BOARD/POST, AND RESTORE THE ORIGINAL STORAGE VOLUME. PLACE SEDIMENT IN A DISPOSAL AREA OR, IF
- APPROPRIATE, MIX WITH DRY SOIL ON THE SITE.
- 3. DO NOT DISPOSE OF SEDIMENT IN A MANNER THAT WILL CREATE AN EROSION OR POLLUTION HAZARD.
- 4. CHECK ALL VISIBLE PIPE CONNECTIONS FOR LEAKS, AND REPAIR AS NECESSARY.
- 5. CHECK FILL MATERIAL IN THE DAM FOR EXCESSIVE SETTLEMENT, SLUMPING OF THE SLOPES OR PIPING BETWEEN THE CONDUIT AND THE EMBANKMENT; MAKE ALL NECESSARY REPAIRS.
- 6. REMOVE ALL TRASH AND OTHER DEBRIS FROM THE BASIN AND RISER.
- 7. SUBMERGED INFLOW PIPES MUST BE INSPECTED AND DE-SILTED (AS REQUIRED) AFTER EACH INFLOW EVENT.

## REMOVAL OR CONVERSION OF SEDIMENT BASIN

- 1. WHEN GRADING AND CONSTRUCTION IN THE DRAINAGE AREA ABOVE A TEMPORARY SEDIMENT BASIN IS COMPLETED AND THE DISTURBED AREAS ARE ADEQUATELY STABILISED, THE BASIN MUST BE REMOVED OR OTHERWISE INCORPORATED INTO THE PERMANENT STORMWATER DRAINAGE SYSTEM. IN EITHER CASE, SEDIMENT SHOULD BE CLEARED AND PROPERLY DISPOSED OF AND THE BASIN AREA STABILISED
- 2. BEFORE STARTING ANY MAINTENANCE WORK ON THE BASIN OR SPILLWAY, INSTALL ALL NECESSARY SHORT-TERM SEDIMENT CONTROL MEASURES DOWNSTREAM OF THE SEDIMENT BASIN.
- 3. ALL WATER AND SEDIMENT MUST BE REMOVED FROM THE BASIN PRIOR TO THE DAM'S REMOVAL. DISPOSE OF SEDIMENT AND WATER IN A MANNER THAT WILL NOT CREATE AN EROSION OR POLLUTION HAZARD.
- 4. BRING THE DISTURBED AREA TO A PROPER GRADE, THEN SMOOTH, COMPACT AND STABILISE OR REVEGETATE AS REQUIRED TO ESTABLISH A STABLE LAND SURFACE.

### MATERIAL STOCKPILING:

- 1. THE CONSTRUCTION CONTRACTOR IS TO ADHERE TO THE FOLLOWING SOIL AND STOCKPILE MANAGEMENT PRACTISES. STOCKPILES OF ERODIBLE MATERIAL THAT HAS THE POTENTIAL TO CAUSE ENVIRONMENTAL HARM IF DISPLACED MUST BE:
- 2. APPROPRIATELY PROTECTED FROM WIND, RAIN, CONCENTRATED SURFACE FLOW AND EXCESSIVE UP-SLOPE STORMWATER SURFACE FLOWS.
- 3. LOCATED AT LEAST 2m FROM ANY HAZARDOUS AREA, RETAINED VEGETATION, OR CONCENTRATED DRAINAGE LINE.
- 4. LOCATED UP-SLOPE OF AN APPROPRIATE SEDIMENT CONTROL SYSTEM.
- 5. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 28 DAYS.
- 6. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 10 DAYS DURING THOSE MONTHS THAT HAVE A HIGH EROSION RISK.
- 7. PROVIDED WITH AN APPROPRIATE PROTECTIVE COVER (SYNTHETIC, MULCH OR VEGETATIVE) IF THE MATERIALS ARE LIKELY TO BE STOCKPILED FOR MORE THAN 5 DAYS DURING THOSE MONTHS THAT HAVE A HIGH EROSION RISK.
- 8. A SUITABLE FLOW DIVERSION SYSTEM MUST BE ESTABLISHED IMMEDIATELY UP-SLOPE OF A STOCKPILE OF ERODIBLE MATERIAL THAT HAS THE POTENTIAL TO CAUSE ENVIRONMENTAL HARM IF DISPLACED, IF THE UP-SLOPE CATCHMENT AREA DRAINING TO THE STOCKPILE EXCEEDS 1500m<sup>2</sup>.





## STABILISED ENTRY/EXIT NOTES

MATERIALS

- ROCK: WELL GRADED, HARD, ANGULAR, EROSION RESISTANT ROCK, NOMINAL DIAMETER OF 50mm TO 75mm (SMALL) DISTURBANCES) OR 100 TO 150mm (LARGE DISTURBANCES). ALL REASONABLE MEASURES MUST BE TAKEN TO OBTAIN ROCK OF NEAR UNIFORM SIZE.
- FOOTPATH STABILISING AGGREGATE: 25 TO 50mm GRAVEL OR AGGREGATE. GEOTEXTILE FABRIC: HEAVY-DUTY, NEEDLE-PUNCHED, NON-WOVEN FILTER CLOTH ('BIDIM' A24 OR EQUIVALENT).

### INSTALLATION

- 1. REFER TO APPROVED PLANS FOR LOCATION AND DIMENSIONAL DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION, DIMENSIONS, OR METHOD OF INSTALLATION, CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.
- 2. CLEAR THE LOCATION OF THE VIBRATION GRID, REMOVING STUMPS, ROOTS AND OTHER VEGETATION TO PROVIDE A FIRM FOUNDATION SO THAT THE ROCK IS NOT PRESSED INTO SOFT GROUND. CLEAR SUFFICIENT WIDTH TO ALLOW PASSAGE OF LARGE VEHICLES, BUT CLEAR ONLY THAT NECESSARY FOR THE EXIT. DO NOT CLEAR ADJACENT AREAS UNTIL THE REQUIRED EROSION AND SEDIMENT CONTROL DEVICES ARE IN PLACE
- 3. IF THE EXPOSED SOIL IS SOFT, PLASTIC OR CLAYEY, PLACE A SUB-BASE OF CRUSHED ROCK OR A LAYER OF HEAVY-DUTY FILTER CLOTH TO PROVIDE A FIRM FOUNDATION.
- 4. ENSURE THAT THE INSTALLATION OF THE VIBRATION GRID INCLUDES ADEQUATE SEDIMENT STORAGE VOLUME UNDER THE GRID. WHERE NECESSARY, INSTALL SUITABLE PRECAST SEDIMENT COLLECTION CHAMBERS
- 5. PLACE A ROCK PAD/RAMP FORMING A MINIMUM 200mm THICK LAYER OF CLEAN. OPEN-VOID ROCK OVER THE ROADWAY BETWEEN THE VIBRATION GRID AND THE SEALED STREET TO PREVENT TYRES FROM PICKING UP MORE SOIL AFTER THEY HAVE BEEN CLEANED.
- 6. IF THE ASSOCIATED CONSTRUCTION SITE IS UP-SLOPE OF THE ROCK PAD, THUS CAUSING STORMWATER RUNOFF TO FLOW TOWARDS THE ROCK PAD. THEN FORM A MINIMUM 300mm HIGH FLOW CONTROL BERM ACROSS THE ROCK PAD TO DIVERT SUCH RUNOFF TO A SUITABLE SEDIMENT TRAP
- 7. THE TOTAL LENGTH OF THE VIBRATION GRIP AND ROCK RAMPS SHOULD BE AT LEAST 15m WHERE PRACTICABLE, AND AS WIDE AS THE FULL WIDTH OF THE ENTRY OR EXIT AND AT LEAST 3m. THE ROCK RAMP SHOULD COMMENCE AT THE EDGE OF THE OFF-SITE SEALED ROAD OR PAVEMENT
- 8. FLARE THE END OF THE ROCK PAD WHERE IT MEETS THE PAVEMENT SO THAT THE WHEELS OF TURNING VEHICLES DO NOT TRAVEL OVER UNPROTECTED SOIL.
- MAINTENANCE 1. INSPECT VIBRATION GRID PRIOR TO FORECAST RAIN, DAILY DURING EXTENDED PERIODS OF RAINFALL, AFTER SIGNIFICANT RUNOFF-PRODUCING RAINFALL, OR OTHERWISE AT FORTNIGHTLY INTERVALS
- 2. IF SAND, SOIL, SEDIMENT OR MUD IS TRACKED OR WASHED ONTO THE ADJACENT SEALED ROADWAY, THEN SUCH MATERIAL MUST BE PHYSICALLY REMOVED, FIRST USING A SQUARE-EDGED SHOVEL, AND THEN A STIFF-BRISTLED BROOM, AND THEN BY A MECHANICAL VACUUM UNIT, IF AVAILABLE.
- 3. IF NECESSARY FOR SAFETY REASONS, THE ROADWAY SHALL ONLY BE WASHED CLEAN AFTER ALL REASONABLE EFFORTS HAVE BEEN TAKEN TO SHOVEL AND SWEEP THE MATERIAL FROM THE ROADWAY.
- 4. WHEN THE VOIDS BETWEEN THE ROCK BECOMES FILLED WITH MATERIAL AND THE EFFECTIVENESS OF THE ROCK RAMPS ARE REDUCED TO A POINT WHERE SEDIMENT IS BEING TRACKED OFF THE SITE, A NEW 100mm LAYER OF ROCK MUST BE ADDED AND/OR THE ROCK PAD MUST BE EXTENDED.
- 5. ENSURE ANY ASSOCIATED DRAINAGE CONTROL MEASURES ARE MAINTAINED IN ACCORDANCE WITH THEIR DESIRED OPERATIONAL CONDITION.
- 6. DISPOSE OF SEDIMENT AND DEBRIS IN A MANNER THAT WILL NOT CREATE AN EROSION OR POLLUTION HAZARD.

### CHECK DAM SEDIMENT TRAPS

### INSTALLATION

- 1. REFER TO APPROVED PLANS FOR LOCATION AND INSTALLATION DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION OR METHOD OF INSTALLATION CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.
- 2. PRIOR TO PLACEMENT OF THE SEDIMENT TRAP, ENSURE THE DRAINAGE CHANNEL IS DEEP ENOUGH TO PREVENT WATER BEING UNSAFELY DIVERTED OUT OF THE DRAIN ONCE THE CHECK DAMS ARE INSTALLED.
- 3. LOCATE EACH CHECK DAM SEDIMENT TRAP AS DIRECTED WITHIN THE APPROVED PLANS. OR OTHERWISE AT SUCH A SPACING TO ACHIEVE THE REQUIRED SEDIMENT TRAPPING OUTCOMES.
- 4. IF THE CHECK DAMS ARE ALSO BEING USED TO CONTROL EROSION WITHIN THE DRAINAGE CHANNEL, THEN LOCATE EACH SUCCESSIVE CHECK DAM SUCH THAT THE CREST OF THE IMMEDIATE DOWNSTREAM DAM IS LEVEL WITH THE CHANNEL INVERT AT THE IMMEDIATE UPSTREAM CHECK DAM.
- 5. ENSURE SAND BAGS EXTEND UP THE CHANNEL BANKS (WHERE PRACTICAL) TO A LEVEL AT LEAST 100mm ABOVE THE CREST LEVEL OF THE CHECK DAM.

### MAINTENANCE

- 1. INSPECT EACH CHECK DAM AND THE DRAINAGE CHANNEL AT LEAST WEEKLY AND AFTER RUNOFF-PRODUCING RAINFALL.
- 2. CORRECT ALL DAMAGE IMMEDIATELY. IF SIGNIFICANT EROSION OCCURS BETWEEN ANY OF THE CHECK DAMS, THEN CHECK THE SPACING OF THE DAMS AND WHERE NECESSARY INSTALL INTERMEDIATE CHECK DAMS OR A SUITABLE CHANNEL LINER.
- 3. CHECK FOR DISPLACEMENT OF THE CHECK DAMS.
- 4. CHECK FOR SOIL SCOUR AROUND THE ENDS OF EACH CHECK DAM. IF SUCH EROSION IS OCCURRING, CONSIDER EXTENDING THE WIDTH OF THE CHECK DAM TO AVOID SUCH PROBLEMS.
- 5. IF SEVERE SOIL EROSION OCCURS EITHER UNDER OR AROUND THE CHECK DAMS. THEN SEEK EXPERT ADVICE ON AN ALTERNATIVE TREATMENT MEASURE.
- DE-SILT SEDIMENT TRAP IF THE SEDIMENT LEVEL EXCEEDS 1/3 THE CREST HEIGHT.
- 7. DISPOSE OF COLLECTED SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

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PROJECT TITLE PROPOSED 174 ADARE ROAD, A STAGE 2

DRAWING TITLE **EROSION AND** CONTROL NO

## KERB INLET SEDIMENT TRAPS

FROM ICEA (INTERNATIONAL EROSION SEDIMENT ASSOCIATION) AUSTRALASIA STANDARD DRAWING ESC-03 (DEC 09).

### MATERIALS

1. SOCKS: MINIMUM 200mm DIAMETER SYNTHETIC OR BIODEGRADABLE TUBES MANUFACTURED FROM NON-WOVEN OR COMPOSITE FABRIC SUITABLE FOR THE 'FILTRATION' OF COARSE SEDIMENTS. 2. FILL MATERIAL: STRAW, CANE MULCH, COMPOSITE MATERIAL (AS4454), COARSE SAND, OR CLEAN AGGREGATE.

3. STAKES: MINIMUM 25 x 25mm TIMBER.

### INSTALLATION

1. REFER TO APPROVED PLANS FOR LOCATION AND INSTALLATION DETAILS. IF THERE ARE QUESTIONS OR PROBLEMS WITH THE LOCATION, DIMENSIONS OR METHOD OF INSTALLATION CONTACT THE ENGINEER OR RESPONSIBLE ON-SITE OFFICER FOR ASSISTANCE.

2. ENSURE THE SOCKS ARE PLACED INDIVIDUALLY OR COLLECTIVELY (AS A SINGLE SEDIMENT TRAP) SUCH THAT:

(i) LEAKAGE AROUND OR UNDER THE SOCKS IS MINIMISED

(ii) ADJOINING SOCKS ARE TIGHTLY BUTTED OR OVERLAPPED AT LEAST 450mm. (iii) THE SURFACE AREA OF POTENTIAL WATER PONDING UP-SLOPE OF EACH SEDIMENT TRAP IS MAXIMISED.

(iv) TO THE MAXIMUM DEGREE PRACTICAL, ALL SEDIMENT-LADEN WATER WILL PASS THROUGH THE FORMED POND BEFORE FLOWING OVER THE DOWN-SLOPE END OF THE SEDIMENT TRAP.

WHEN PLACED ACROSS THE INVERT OF MINOR DRAINS, ENSURE THE SOCKS ARE PLACED SUCH THAT (i) THE CREST OF THE DOWNSTREAM SOCK IS LEVEL WITH THE CHANNEL INVERT AT THE IMMEDIATE UPSTREAM SOCK (IF ANY);

(ii) EACH SOCK EXTENDS UP THE CHANNEL BANKS SUCK THAT THE CREST OF THE SOCK AT ITS LOWEST POINT IS LOWER THAN GROUND LEVEL AT EITHER END OF THE SOCK.

4. IF STAKES ARE REQUIRED TO ANCHOR THE SOCKS, THEIR SPACING DOES NOT EXCEED 1.2m OR SIX TIMES THE SOCK DIAMETER (WHICHEVER IS THE LESSER). A MAXIMUM STAKE SPACING OF 0.3m APPLIES WHEN USED TO FORM CHECK DAMS.

### MAINTENANCE

1. INSPECT ALL FILTER SOCKS PRIOR TO FORECAST RAIN, DAILY DURING EXTENDED PERIODS OF RAINFALL, AFTER SIGNIFICANT RUNOFF PRODUCING STORMS OR OTHERWISE AT WEEKLY INTERVALS.

2. REPAIR OR REPLACE DAMAGED SOCKS.

3. THE BULK OF THE SEDIMENT COLLECTED BEHIND THE FILTER SOCKS SHOULD BE REMOVED BY SHOVEL AFTER EACH STORM EVENT.

4. REMOVE COLLECTED SEDIMENT AND DISPOSE OF IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

### REMOVAL

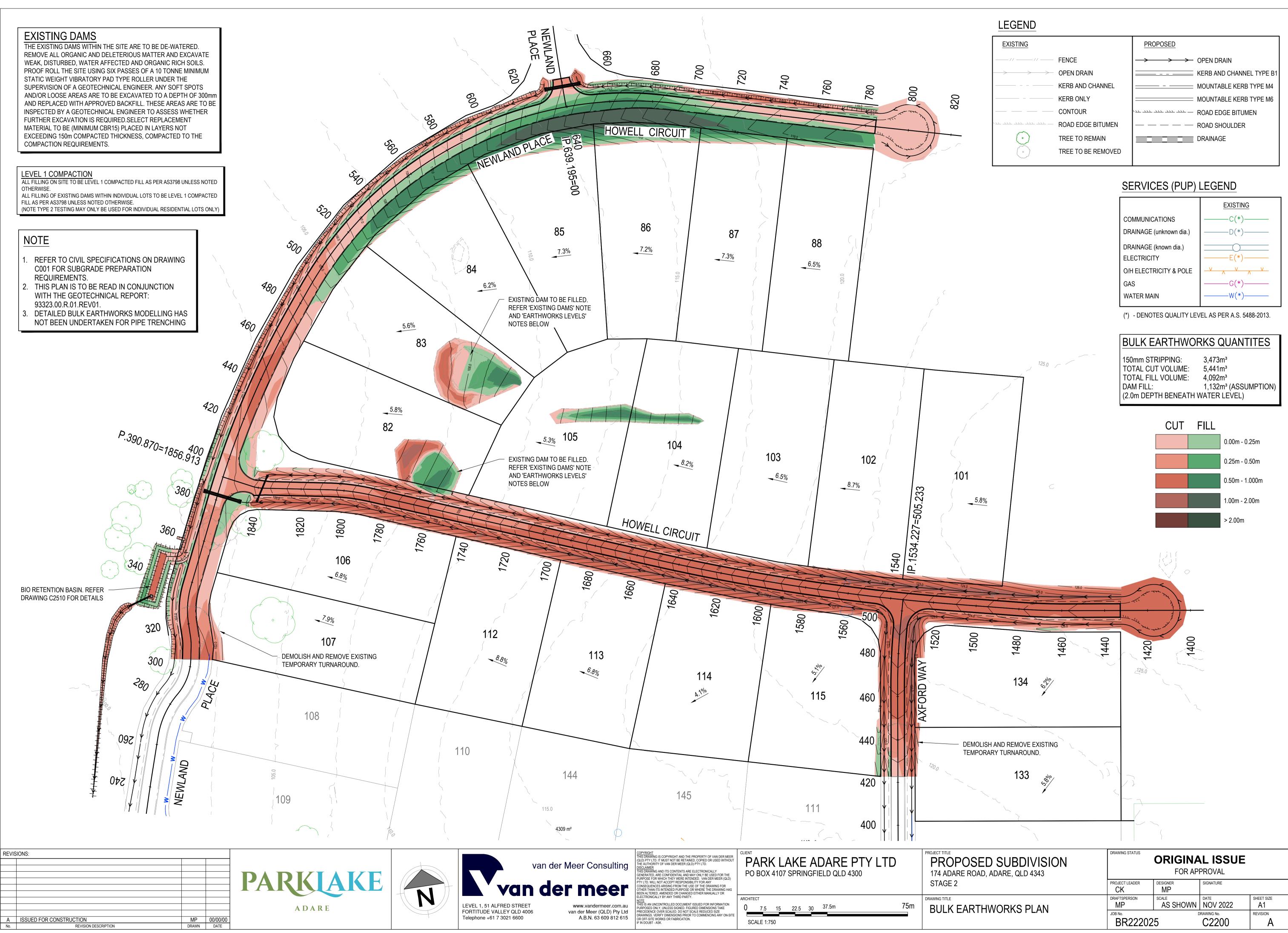
1. ALL SAND, SOIL, SEDIMENT OR MUD MUST BE PHYSICALLY REMOVED FROM SEALED SURFACES, FIRST USING A SQUARE-EDGED SHOVEL, AND THEN A STIFF-BRISTLED BROOM, AND THEN BY A MECHANICAL VACUUM UNIT, IF AVAILABLE.

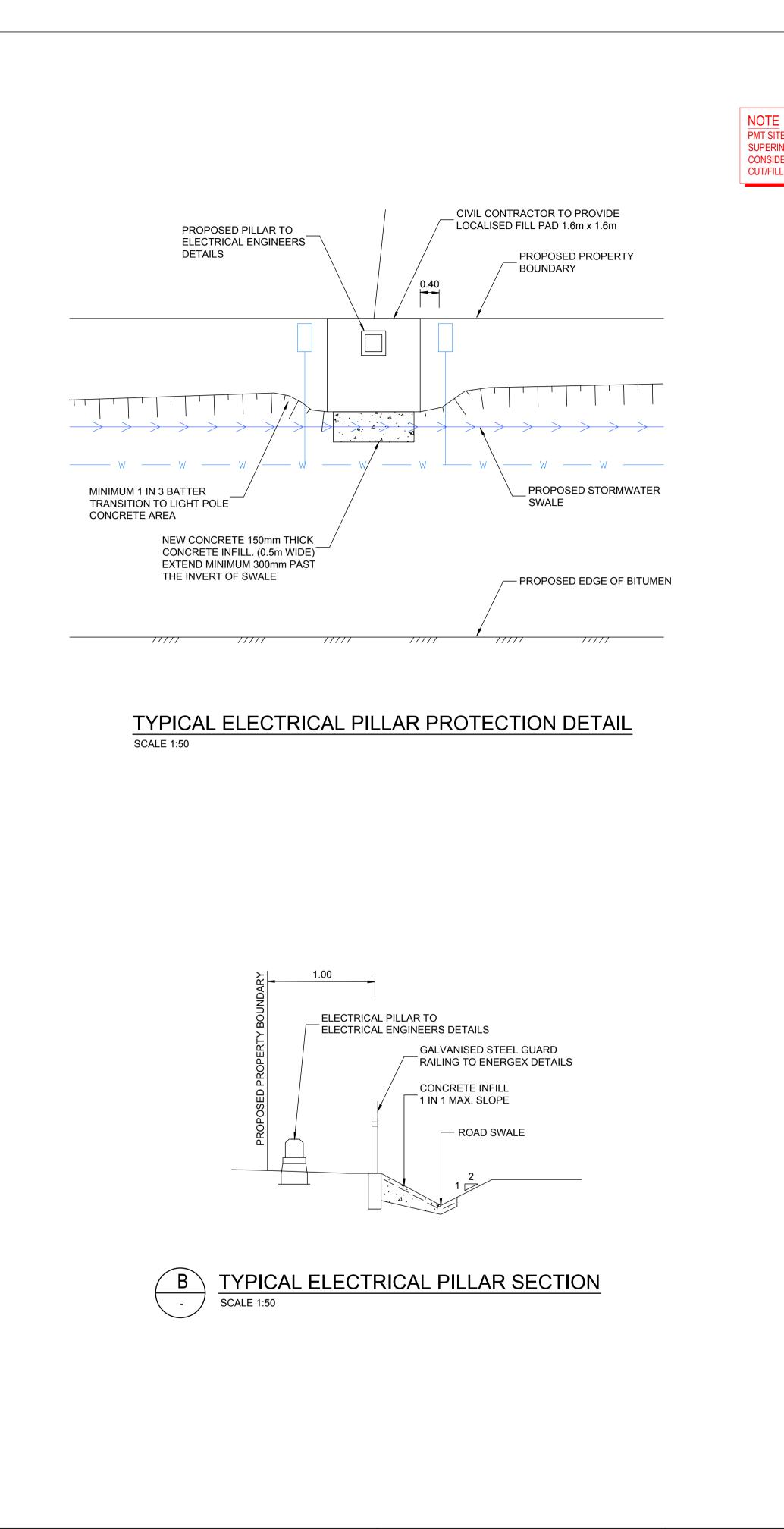
2. IF NECESSARY FOR SAFETY REASONS, THE SEALED SURFACE SHALL ONLY BE WASHED CLEAN AFTER ALL REASONABLE EFFORTS HAVE BEEN TAKEN TO SHOVEL AND SWEEP THE MATERIAL FROM THE SURFACE.

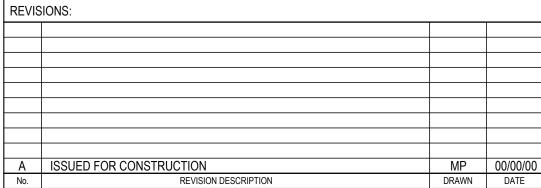
3. DISPOSE OF COLLECTED SEDIMENT IN A SUITABLE MANNER THAT WILL NOT CAUSE AN EROSION OR POLLUTION HAZARD.

4. ALL SYNTHETIC (PLASTIC) MESH OR OTHER NON READILY BIODEGRADABLE MATERIAL MUST BE REMOVED FROM THE SITE ONCE THE SLOPE OR DRAIN IS STABILISED, OR THE SOCKS HAVE DETERIORATED TO A POINT WHERE THEY ARE NO LONGER PROVIDING THEIR INTENDED DRAINAGE OR SEDIMENT CONTROL FUNCTION.

SUBDIVISION ADARE, QLD 4343	DRAWING STATUS ORIGINAL ISSUE FOR APPROVAL					
	PROJECT LEADER	designer MP	SIGNATURE			
) SEDIMENT	draftsperson	SCALE AS SHOWN	DATE NOV 2022	SHEET SIZE		
TES	JOB №. BR22202		DRAWING No. C2191	REVISION		

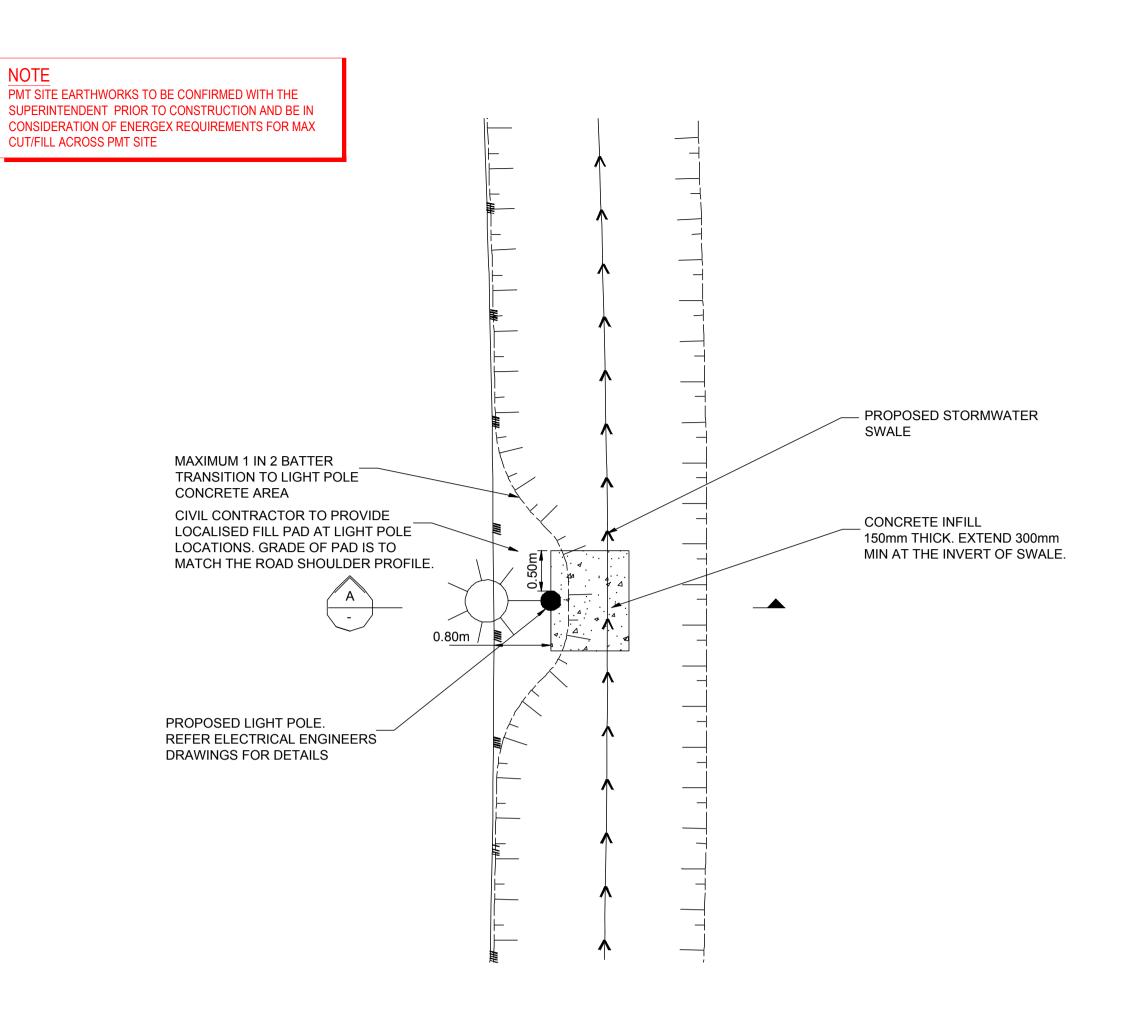






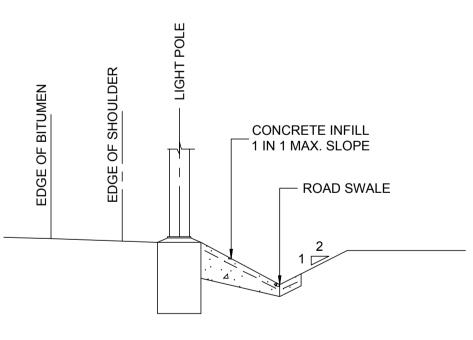




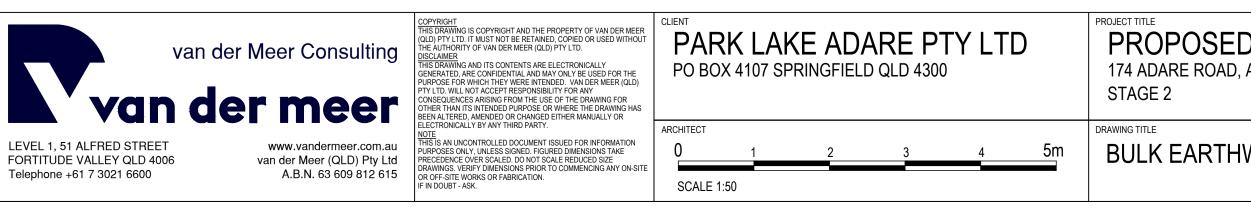


## **TYPICAL LIGHT POLE PROTECTION DETAIL**

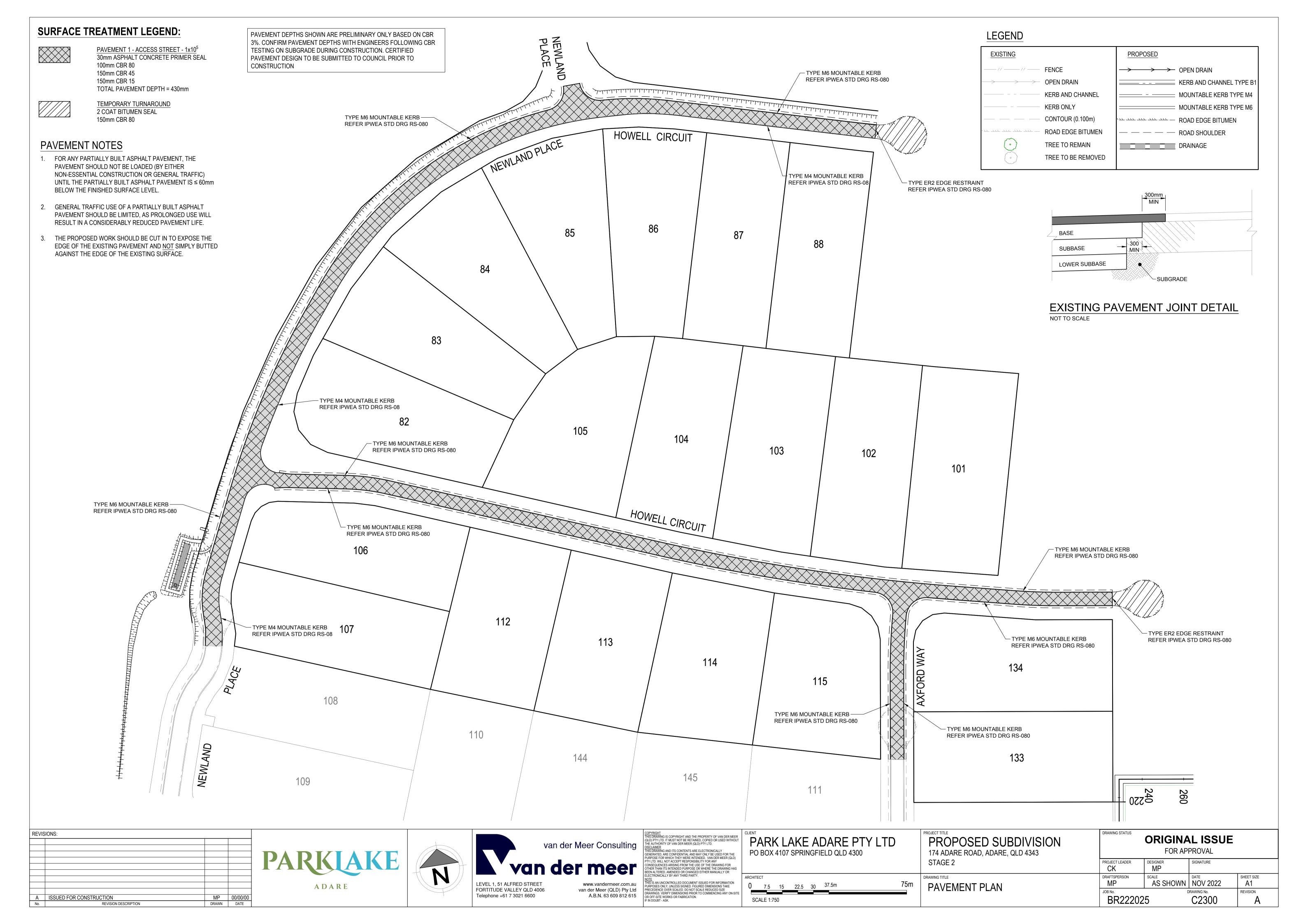
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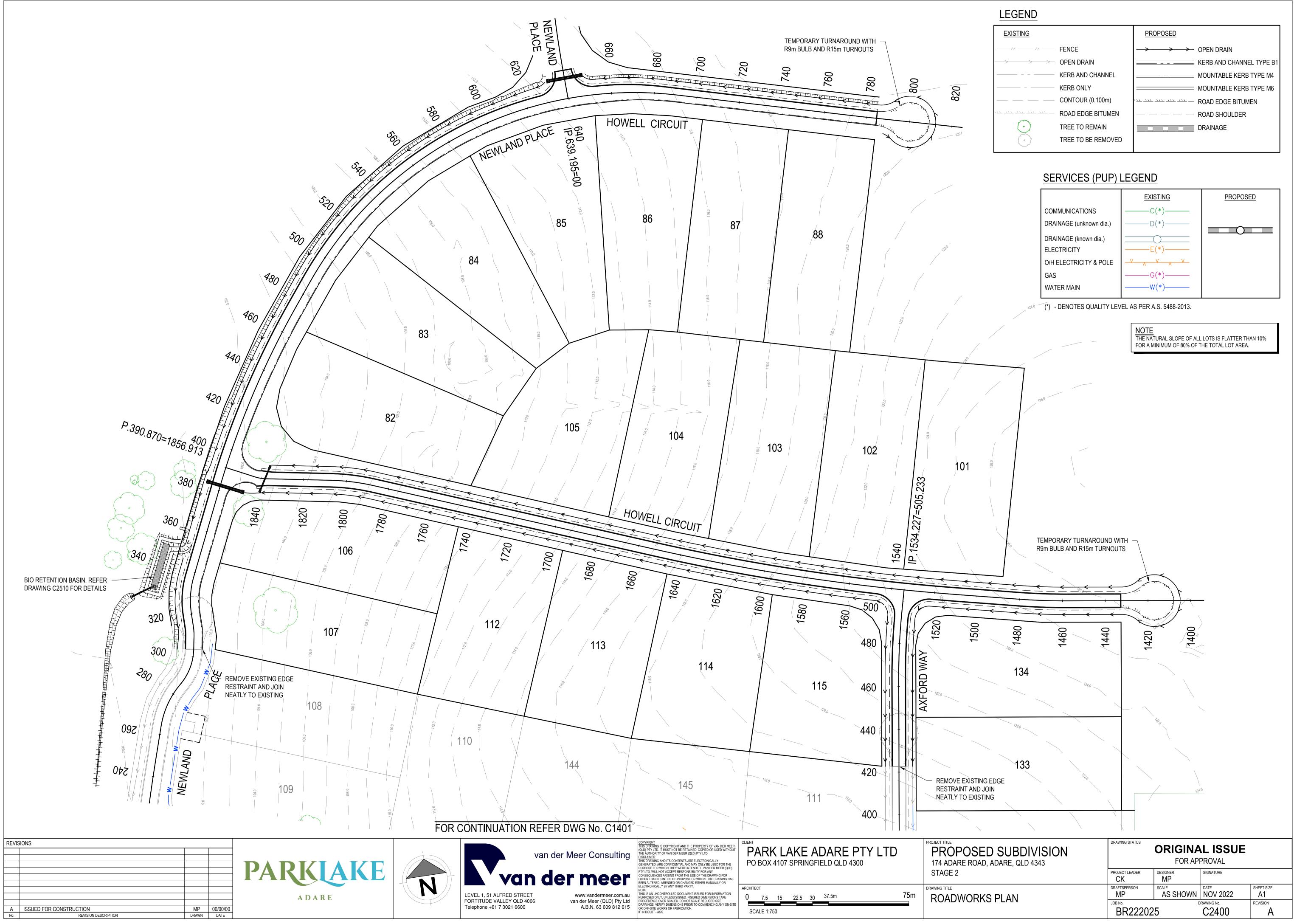


### **TYPICAL LIGHT POLE SECTION A** SCALE 1:50

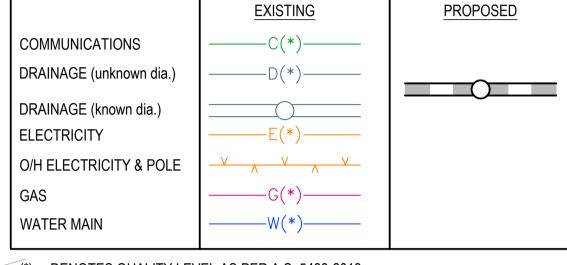


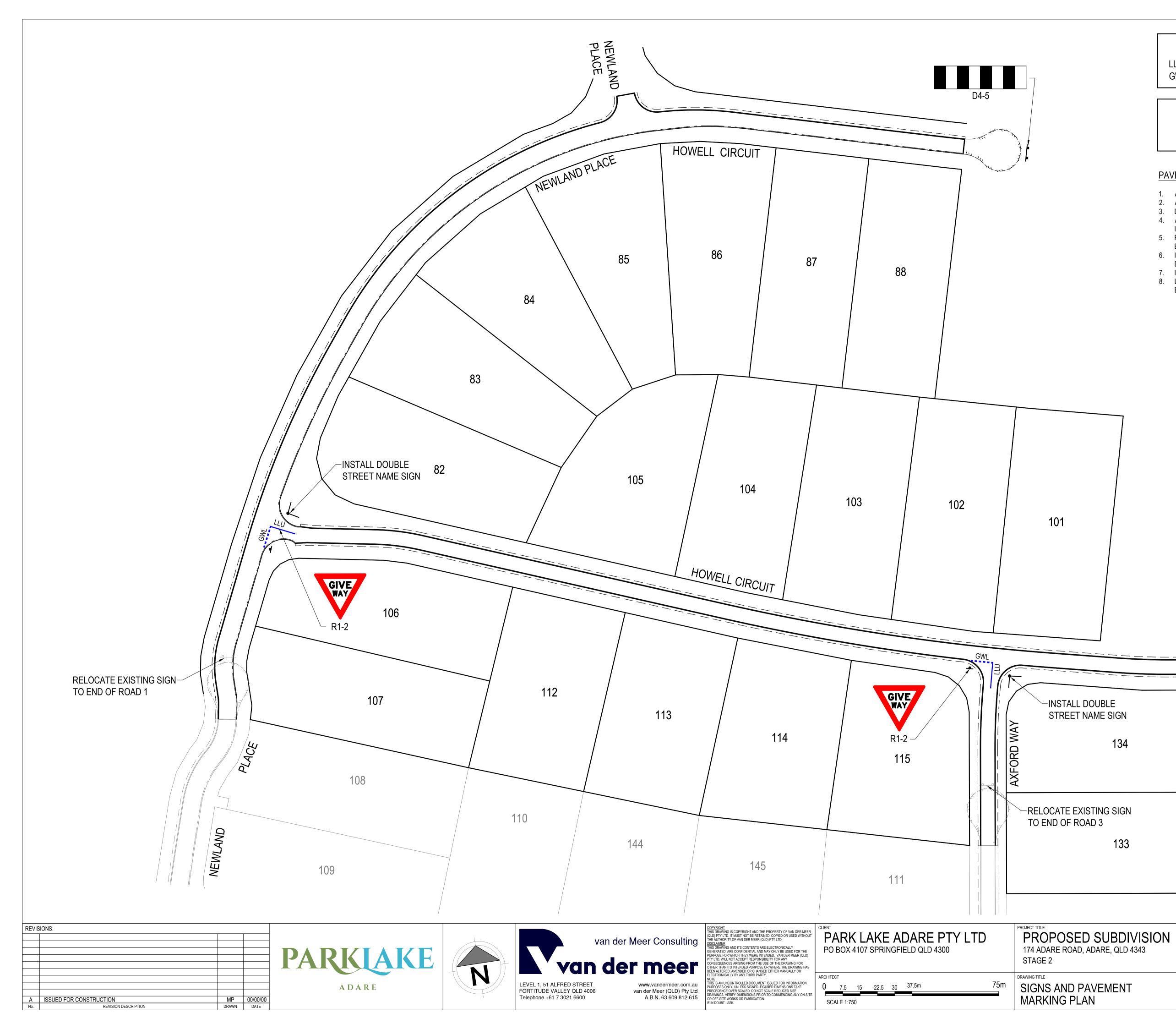
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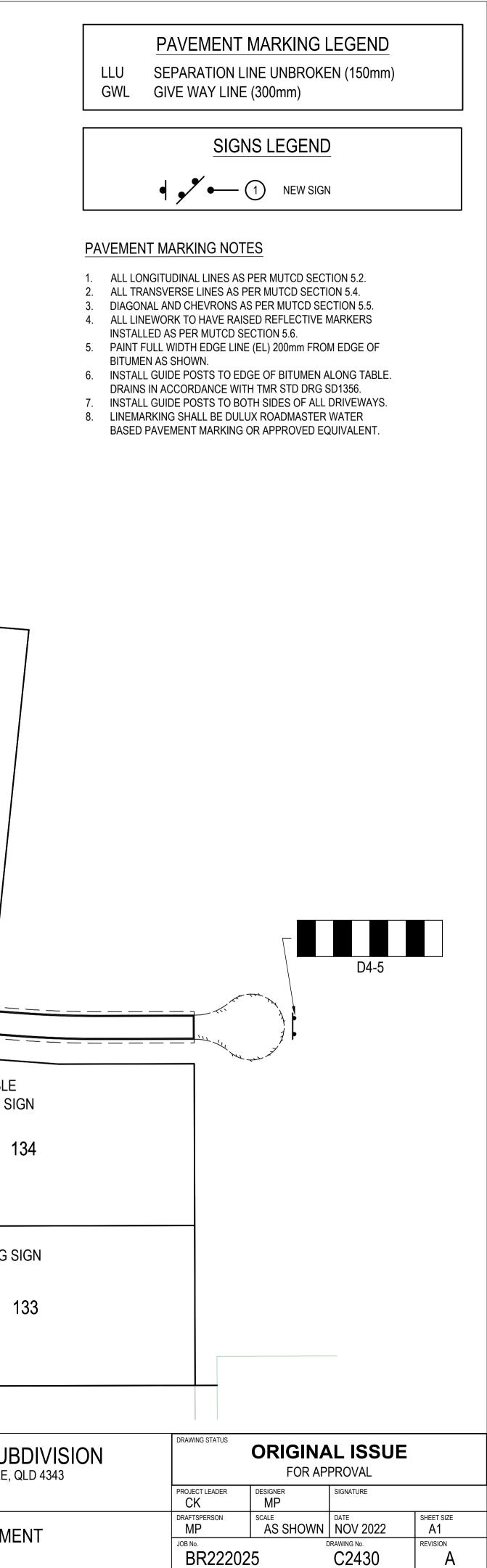


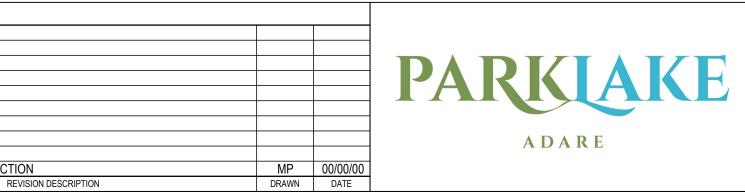


EXISTING		PROPOSED	
////	FENCE	$\rightarrow \rightarrow \rightarrow \rightarrow$	OPEN DRAIN
$\longrightarrow$	OPEN DRAIN		KERB AND CHANNEL TYPE B1
	KERB AND CHANNEL		MOUNTABLE KERB TYPE M4
	KERB ONLY		MOUNTABLE KERB TYPE M6
	CONTOUR (0.100m)		ROAD EDGE BITUMEN
· · · · · · · · · · · · · · · · · · ·	ROAD EDGE BITUMEN		ROAD SHOULDER
$\odot$	TREE TO REMAIN		DRAINAGE
$\bigcirc$	TREE TO BE REMOVED		









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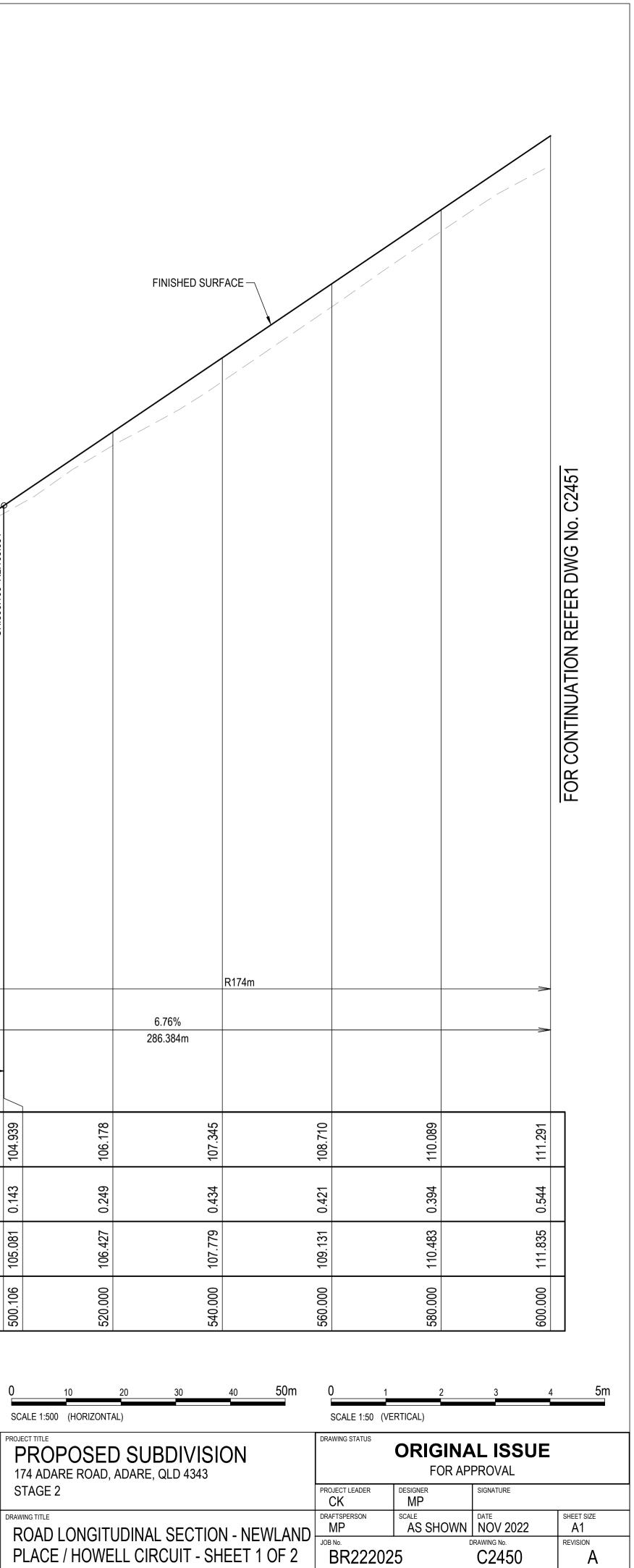
		R40m		STAGE 1 9000 NEATLY TO EXISTING DOIN NEATLY TO EXISTING B CH.303.631 RL.101.125			R40m				CH.400.995 CH.420.106 RL.102.057	R400m	IP CH.460.106 RL.102.377				CH.500.106 RL.105.081	
Horiz Curve Data Vertical Geometry Grade (%)		0.93%	~~~		R- <u>26.3</u> 7		>			0.8%				~~~~				
Vertical Grade Length (m) Vertical Curve Length (m)		274.906m		>	<					156.474m			80m VC					
Vertical Curve Radius (m) DATUM R.L.94.000													1342m				>	
NATURAL SURFACE LEVELS	100.699	100.941	101.094	101.529 101.530	101.506 101.473	101.403 101.400	101.361 101.361	101.525	101.512 101.508	101.760	102.083 102.085	102.491	103.024 103.028	103.536	103.869	104.180	104.933 104.939	106.178
CUT / FILL DEPTH	0.022	-0.035	-0.135	-0.437 -0.405	-0.333 -0.263	-0.147 -0.141			0.224 0.234	0.136	-0.027 -0.028	-0.128	-0.055	-0.032	0.004	0.040	0.142 0.143	
DESIGN SURFACE LEVELS	100.721	100.906	100.959	101.092 101.125	101.173 101.210	101.256 101.259	101.398 101.416	101.576	101.736 101.742	101.896	102.056 102.057	102.364	102.969 102.973	103.504	103.873	104.220	105.074 105.081	106.427
CHAINAGE	260.000 263.877	280.000	285.676	300.000 303.631	309.590 314.209	320.000 320.317	337.769 340.000	360.000	380.000 380.669	400.000	420.000 420.106	440.000	460.000 460.106	472.609	480.000	486.314	500.000 500.106	520.000

## LONGITUDINAL SECTION - NEWLAND PLACE / HOWELL CIRCUIT

HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:50

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Horiz Curve Data	CONTINUATION REFER DWG No. C2450		CH 601 400 EI 111 036		R174m		NG SURFACE
Vertical Geometry Grade (%)	ILLNC	<del></del>					6.76%
Vertical Grade Length (m) Vertical Curve Length (m)	FOR C(						286.384m
Vertical Curve Radius (m) DATUM R.L.103.000	Щ			-			
NATURAL SURFACE LEVELS	110.089	111.291	111.370	112.439	44.0 EOE	114.442	
CUT / FILL DEPTH	0.394 1	0.544		0.701			
DESIGN SURFACE	110.483 0.	111.835 0.		113.140 0.	0000		
LEVELS							
CHAINAGE	580.000	600.000	601.490	620.000		000.066	

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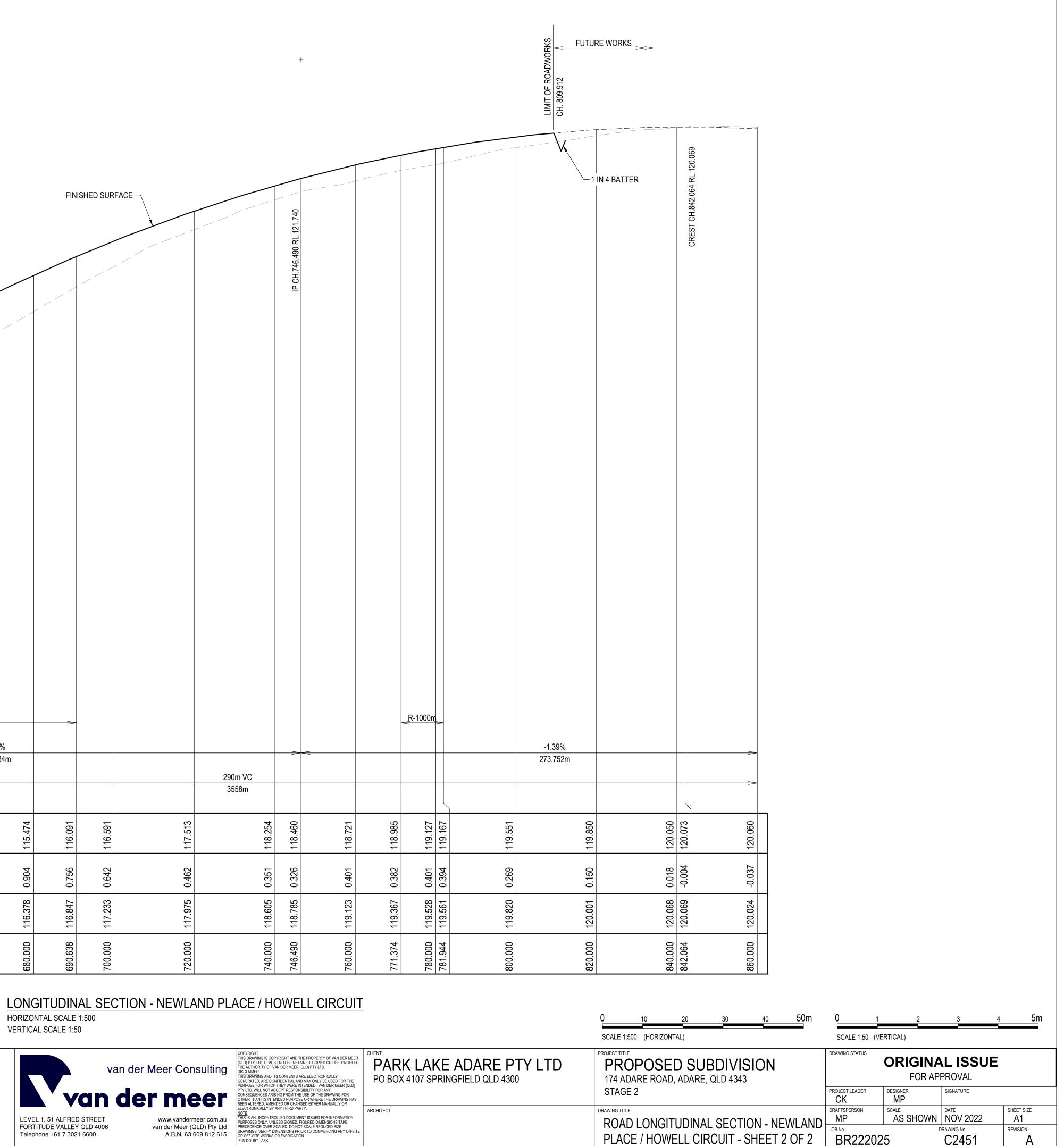
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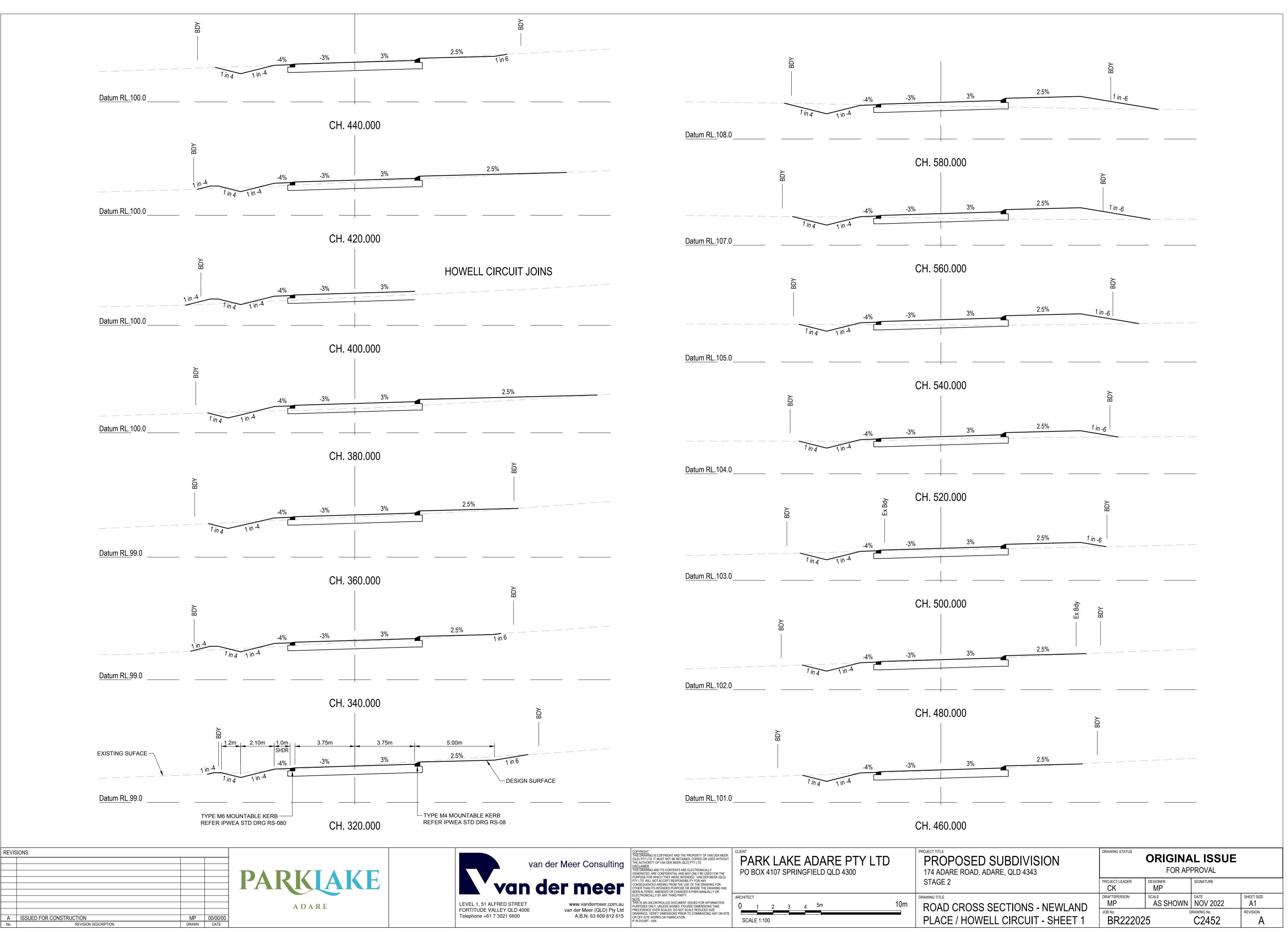


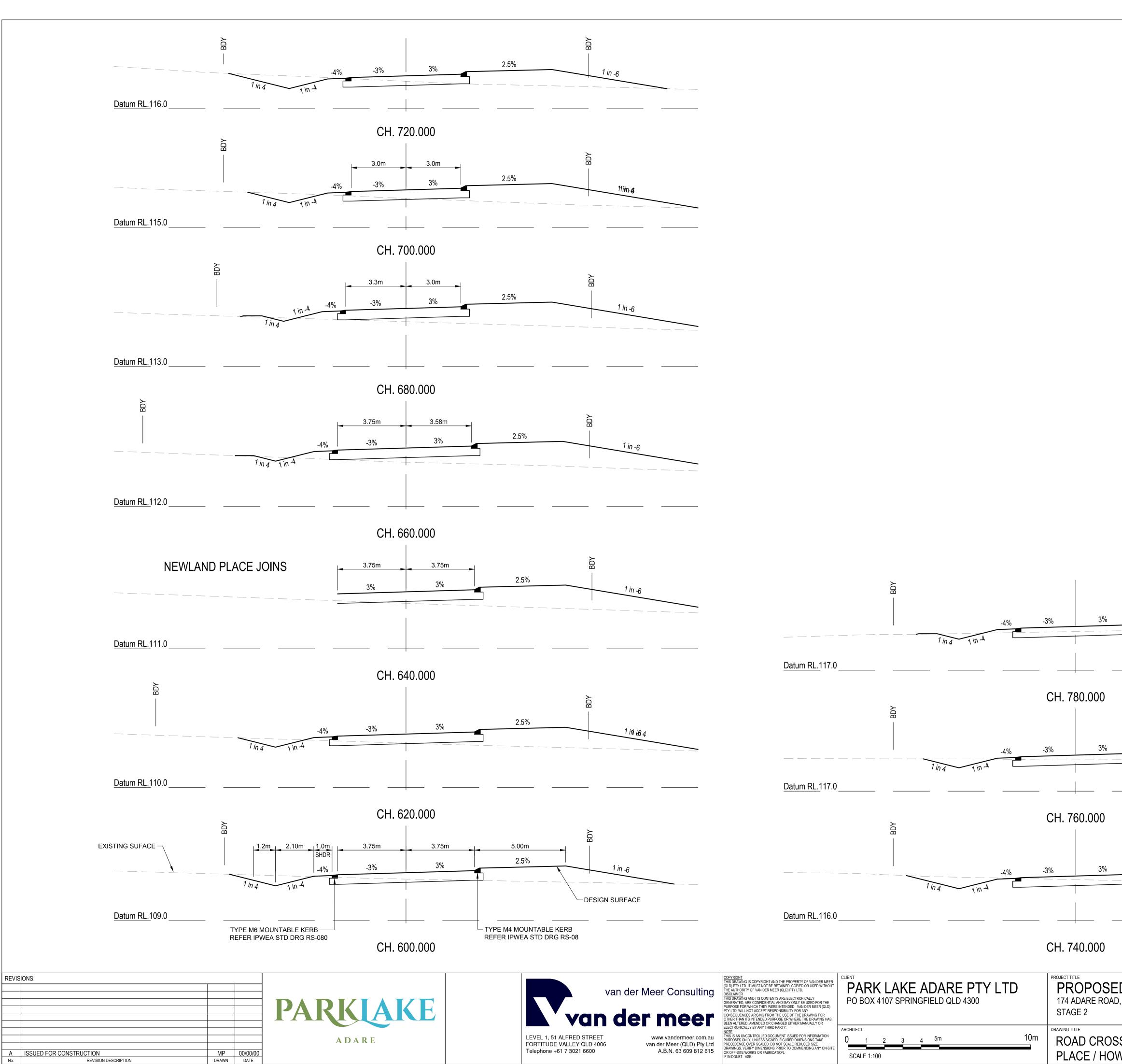
PLACE / HOWELL CIRCUIT - SHEET 2 OF 2

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2.5% 1 in -6				
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SS SECTIONS - NEWLAND WELL CIRCUIT - SHEET 2	CK DRAFTSPERSON MP JOB No. BR22202	MP SCALE AS SHOWN	DATE	SHEET SIZE A1 REVISION A

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		CREST CH.1385.550 RL.126.249		EUTURE N			IP CH.1472.260 RL.128.916				INTERSECTION - AXFORD WAY CH.1533.921	FIN	NISHED SURFACE				
Horiz Curve Data							R300m							<u>R970m</u>		CH.1632.260 RL.117.271	(CE
Vertical Geometry Grade (%)					2.16%									-7.28%			
Vertical Grade Length (m) Vertical Curve Length (m) Vertical Curve Radius (m) DATUM R.L.108.000	V				232.473m					320m VC 3390m				308.742m		>	
	126.320	126.266	126.001	126.199	125 962	പ്പ	125.302	125.178 125.168	124.680	124.391 123.897	123.110	122.136	121.084	119.661	118.365		
CUT / FILL DEPTH		-0.017	0.217	-0.126		121		-0.245 -0.252		-0.362 -0.314		-0.376	-0.413	-0.196	-0.224		
DESIGN SURFACE LEVELS	126.244	126.249	126.218	126.074	125,812	125.486	125.140	124.933 124.916	124.317	124.029 123.583	122.730	121.760	120.672	119.466	118.141	117.271	
CHAINAGE	1380.000	1385.550	1400.000	1420.000	1440 000	457.4 460.0	1472.260	1480.000 1480.616	1500.000	1508.227 1520.000	1540.000	1560.000	1580.000	1600.000	1620.000	1632.260	

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### LONGITUDINAL SECTION - HOWELL CIRCUIT HORIZONTAL SCALE 1:500

VERTICAL SCALE 1:50

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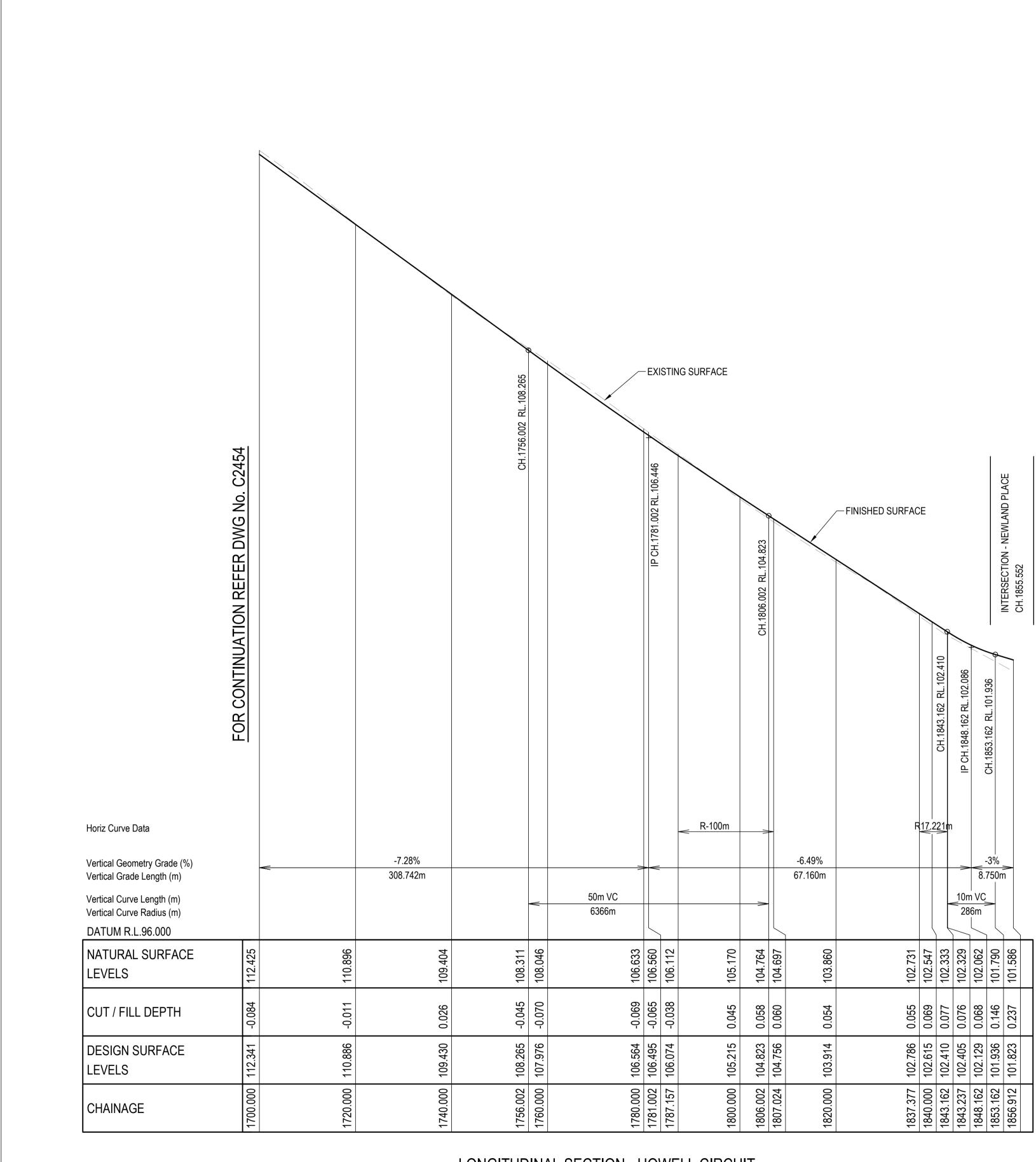
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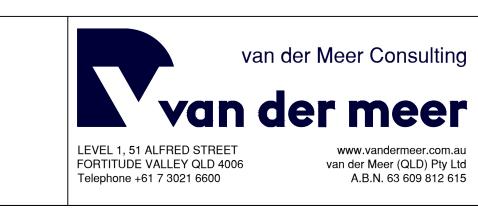
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LONGITUDINAL SECTION - HOWELL CIRCUIT HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:50



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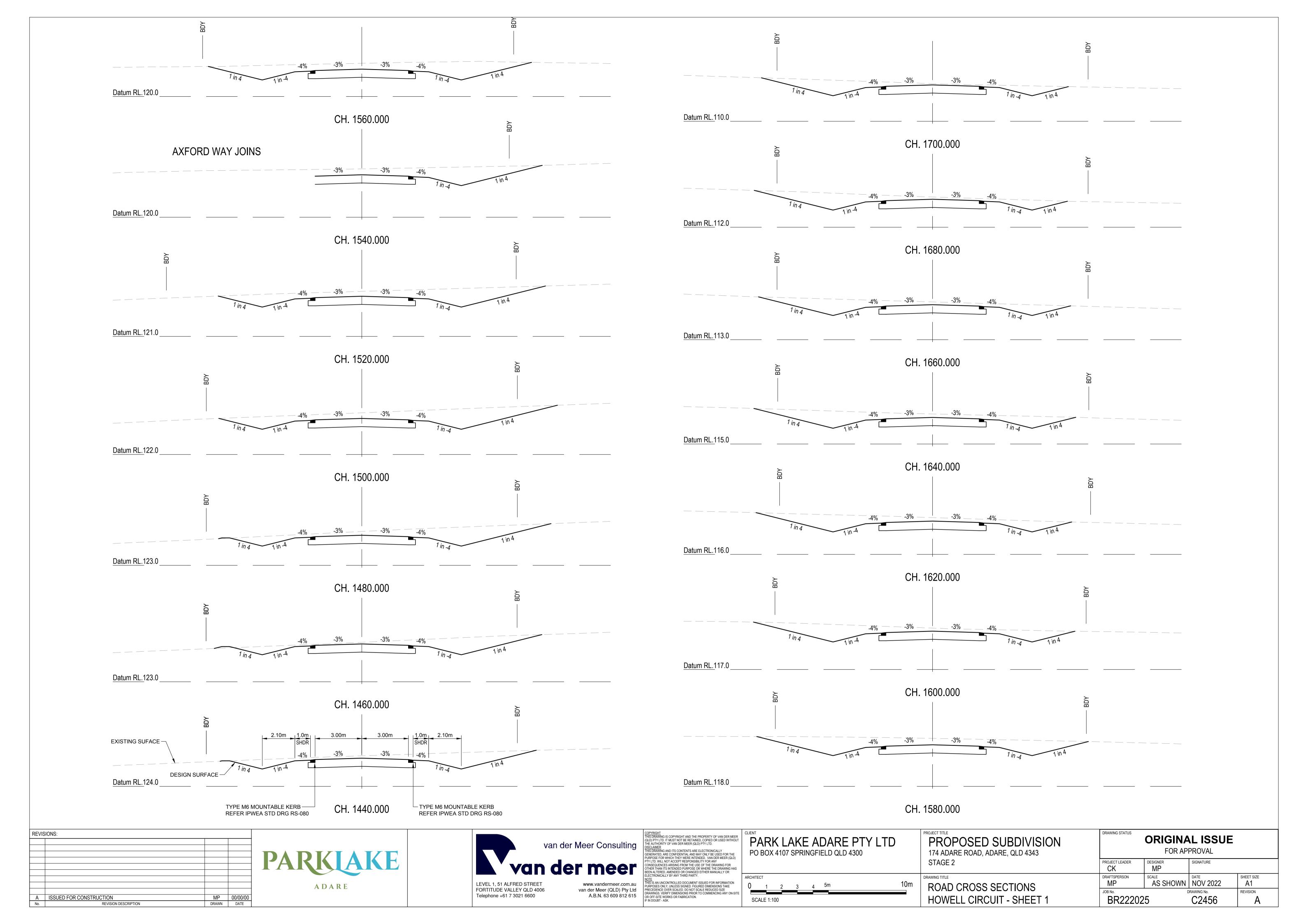
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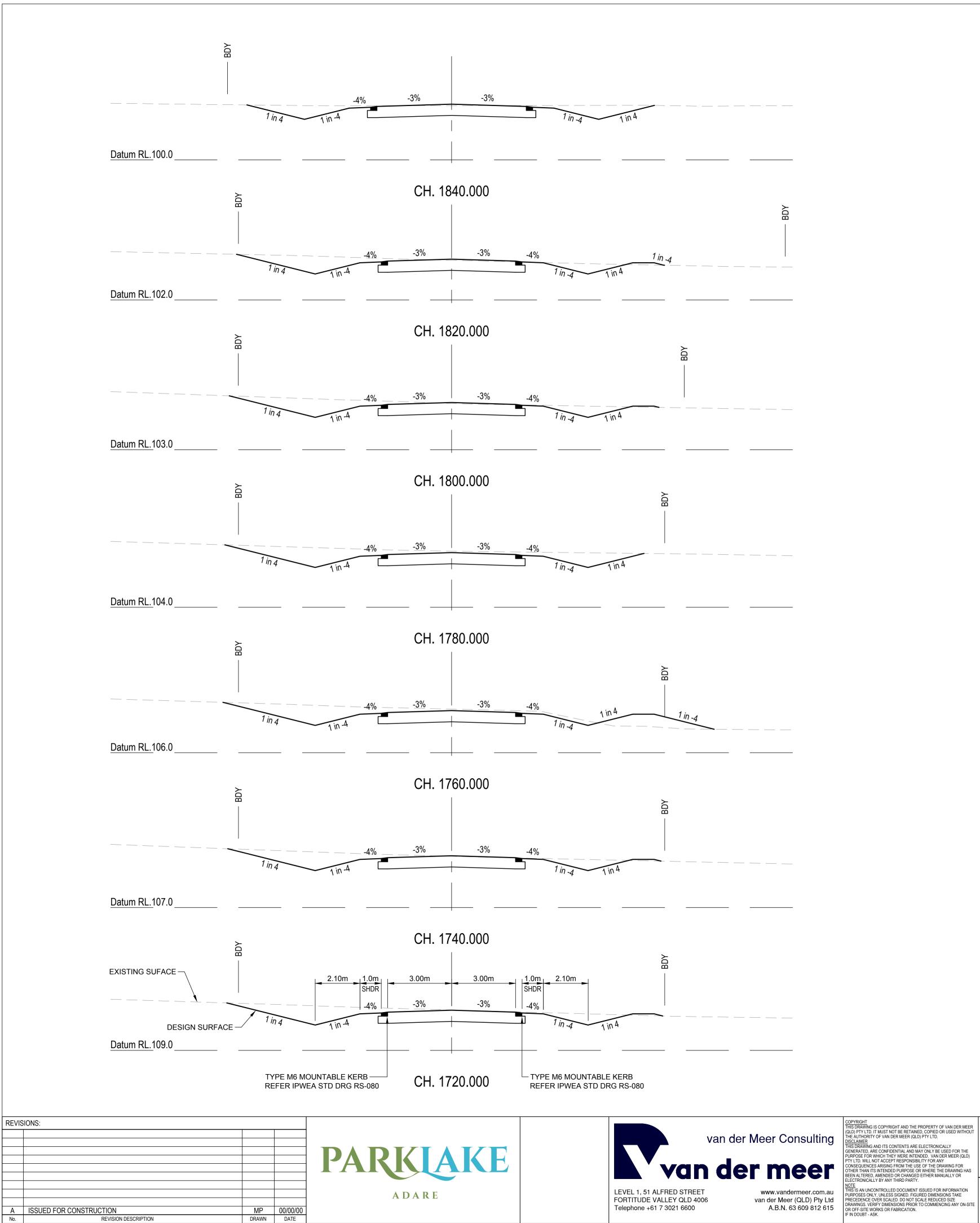
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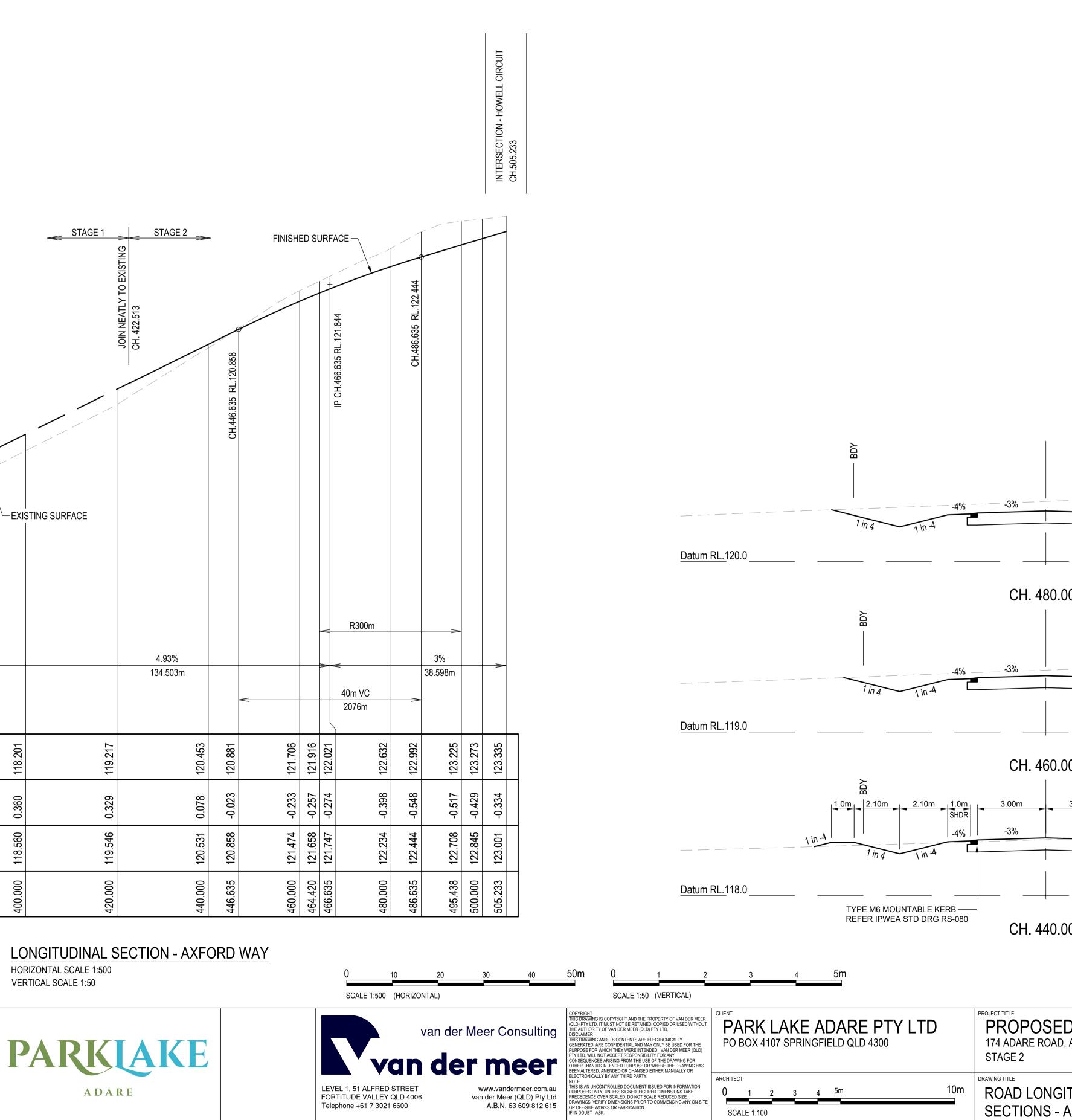
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•					DARE IELD QLD 4	PTY LTD		PROJECT TITLE <b>PROPOSED</b> 174 ADARE ROAD, A STAGE 2
ARCHIT	ECT						40	DRAWING TITLE
0	1	2	3	4	5m		10m	ROAD CROSS
SC	CALE 1:1	00						HOWELL CIRC

POSED SUBDIVISION ARE ROAD, ADARE, QLD 4343	DRAWING STATUS ORIGINAL ISSUE FOR APPROVAL						
2	PROJECT LEADER	designer MP	SIGNATURE				
CROSS SECTIONS	DRAFTSPERSON	AS SHOWN	DATE NOV 2022	SHEET SIZE			
ELL CIRCUIT - SHEET 2	JOB NO. BR22202	-	0. C2457				

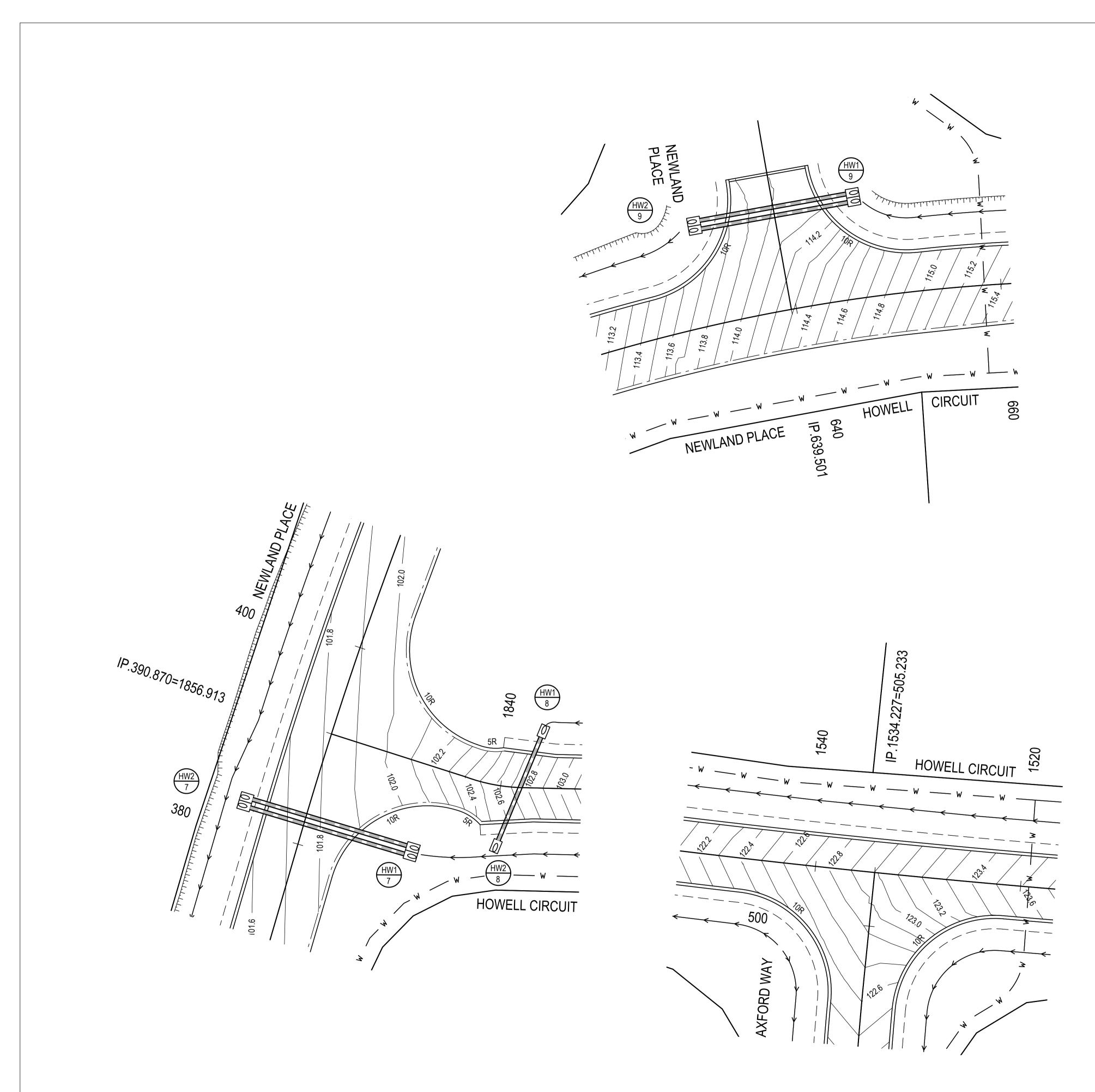
			STAGE 1	JOIN NEATLY TO EXISTING V CH. 422.513	RL.120.858	FINISH	ED SI	IP CH.466.635 RL.121.844
		EXIS	STING SURFACE		CH.446.635 RL.12			IP CH.46
Horiz Curve Data								F
Vertical Geometry Grade (%) Vertical Grade Length (m)	-	<		4.93% 134.503m				><
Vertical Curve Length (m) Vertical Curve Radius (m) DATUM R.L.112.000						<		40r 20
NATURAL SURFACE LEVELS	117.213	118.201	119.217	100 153	120.881	121.706	121.916	122.021
CUT / FILL DEPTH	0.362	0.360	0.329	0.078	-0.023	-0.233	-0.257	-0.274
DESIGN SURFACE LEVELS	117.575	118.560	119.546	100 £34	120.858	121.474	121.658	121.747
CHAINAGE	380.000	400.000	420.000		446.635	460.000	464.420	466.635

### LONGITUDINAL SECTION - AXFORD WAY HORIZONTAL SCALE 1:500 VERTICAL SCALE 1:50



REVIS	IONS:		
A	ISSUED FOR CONSTRUCTION	MP	00/00/00
No.	REVISION DESCRIPTION	DRAWN	DATE

- BDY				
-3% -4%	1 in 4			
D00 合				
-3% -4%	1 in 4			
DOO				
3.00m         1.0m         2.10m           SHDR         -3%         -4%		EXIST	ING SUFACE	
1 in -4 1 in -4	4 DESI	GN SURFACE		
TYPE M6 MOUNTABLE KERI REFER IPWEA STD DRG RS				
000	-000			
D SUBDIVISION , ADARE, QLD 4343	DRAWING STATUS	ORIGINA FOR AP	AL ISSUE	
, , ,	PROJECT LEADER CK DRAFTSPERSON	designer MP scale	SIGNATURE	SHEET SIZE
ITUDINAL AND CROSS AXFORD WAY	МР JOB №. BR22202	AS SHOWN		A1 REVISION
		0	02700	A



REVIS	SIONS:					
				PARKLAKE		
					+	
				A D A R E		LEV
						Tele
A	ISSUED FOR CONSTRUCTION	MP	00/00/00		·	
No.	REVISION DESCRIPTION	DRAWN	DATE			



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CLIENT

PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300 ARCHITECT 0 1 2 3 4 5 20m 10m SCALE 1:200

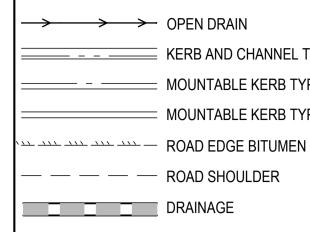
PROJECT TITLE <b>PROPOSED</b> 174 ADARE ROAD, AI STAGE 2
DRAWING TITLE

## LEGEND

### EXISTING

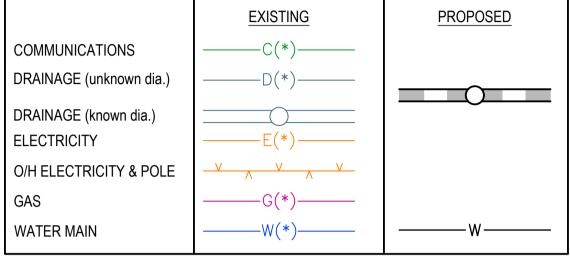
////	FENCE
>>	OPEN DRAIN
	KERB AND CHANNEL
	KERB ONLY
	CONTOUR (0.100m)
···· ··· ··· ··· ··· ···	ROAD EDGE BITUMEN
$\bigcirc$	TREE TO REMAIN
0	TREE TO BE REMOVED

### PROPOSED



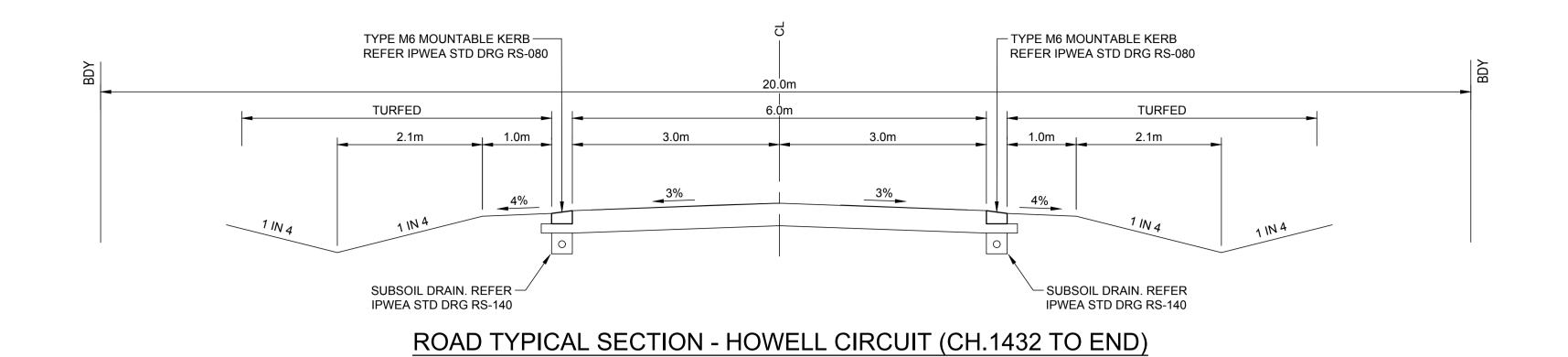
\_\_\_\_ KERB AND CHANNEL TYPE B1 — – — MOUNTABLE KERB TYPE M4 MOUNTABLE KERB TYPE M6

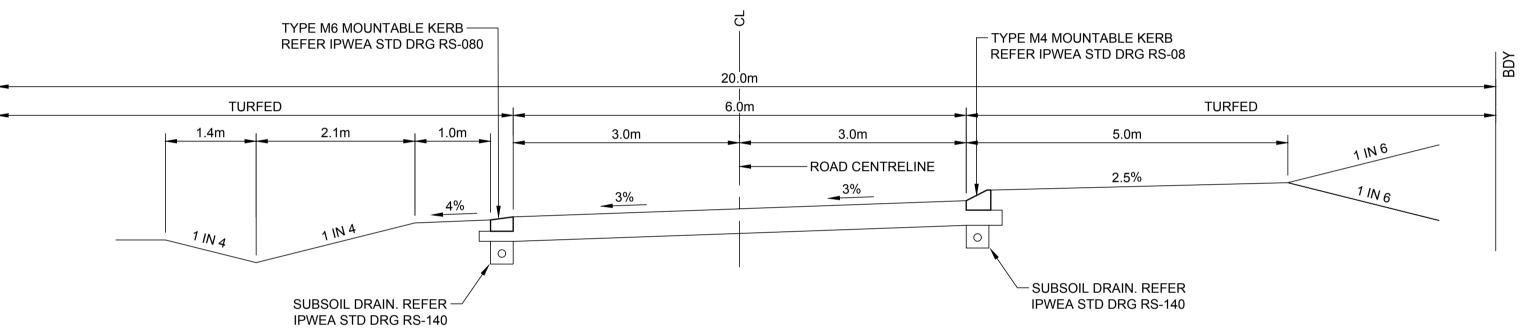
## SERVICES (PUP) LEGEND

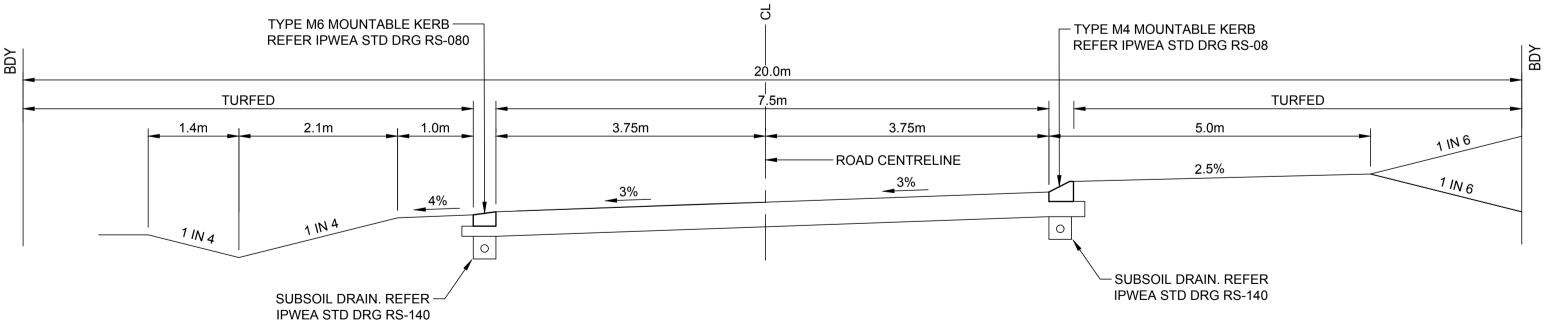


(\*) - DENOTES QUALITY LEVEL AS PER A.S. 5488-2013.

O SUBDIVISION ADARE, QLD 4343	DRAWING STATUS ORIGINAL ISSUE FOR APPROVAL					
	PROJECT LEADER		SIGNATURE			
N DETAILS	DRAFTSPERSON	AS SHOWN	date NOV 2022	SHEET SIZE		
N DETAILS	JOB №. BR22202		DRAWING No. REVISION			











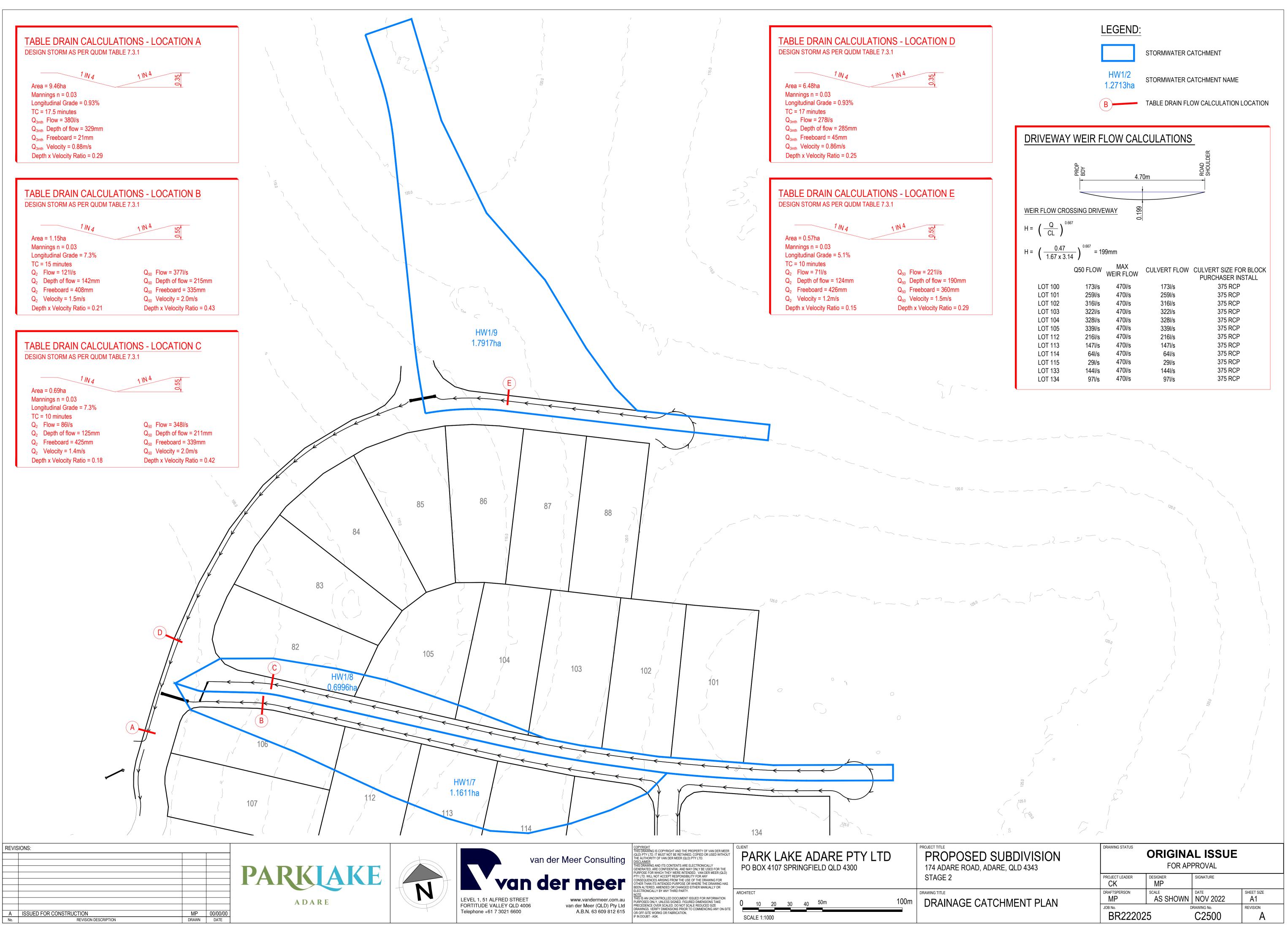
	REVIS	IONS:		
	Α	ISSUED FOR CONSTRUCTION	MP	00/00/00
I	No.	REVISION DESCRIPTION	DRAWN	DATE

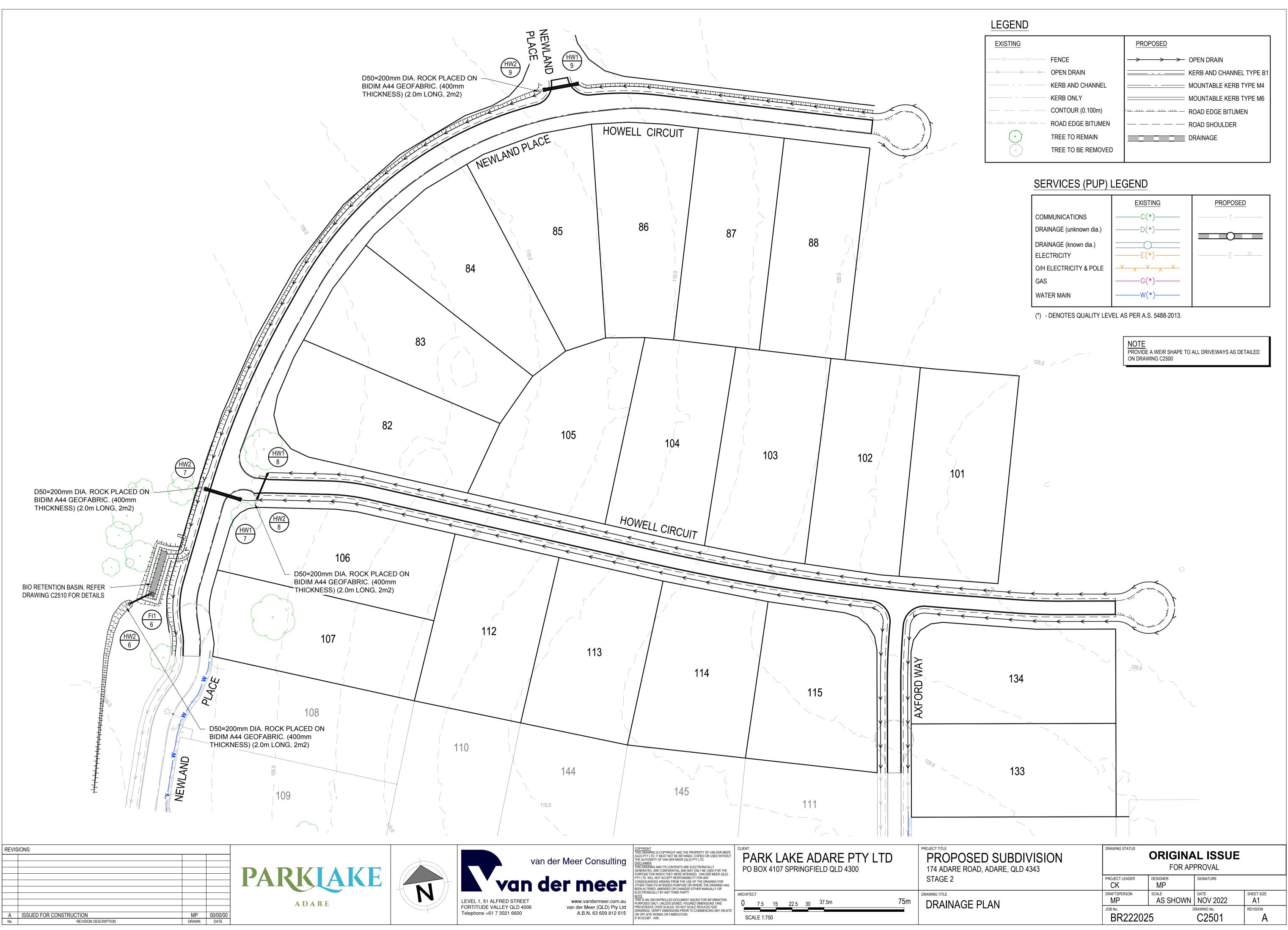
# ROAD TYPICAL SECTION - HOWELL CIRCUIT (CH.660 TO CH.785) SCALE 1:50

# ROAD TYPICAL SECTION - NEWLAND PLACE (CH.303 TO CH.640) SCALE 1:50

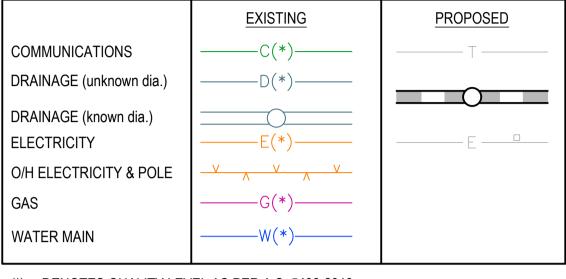
Van der meer	COPYRIGHT THIS DRAWING IS COPYRIGHT AND THE PROPERTY OF VAN DER MEER (QLD) PTY LTD. IT MUST NOT BE RETAINED, COPIED OR USED WITHOUT THE AUTHORITY OF VAN DER MEER (QLD) PTY LTD. DISCLAIMER THIS DRAWING AND ITS CONTENTS ARE ELECTRONICALLY GENERATED, ARE CONFIDENTIAL AND MAY ONLY BE USED FOR THE PURPOSE FOR WHICH THEY WERE INTENDED. VAN DER MEER (QLD) PTY LTD. WILL NOT ACCEPT RESPONSIBILITY FOR ANY CONSEQUENCES ARISING FROM THE USE OF THE DRAWING FOR OTHER THAN ITS INTENDED PURPOSE OR WHERE THE DRAWING HAS BEEN ALTERED, AMENDED OR CHANGED EITHER MANUALLY OR			<b>KE ADA</b> PRINGFIELI		—	C	PROJECT TITLE <b>PROPOSE</b> 174 ADARE ROAD, STAGE 2
I, 51 ALFRED STREET www.vandermeer.com.au UDE VALLEY QLD 4006 van der Meer (QLD) Pty Ltd ne +61 7 3021 6600 A.B.N. 63 609 812 615	BEEN AL LERED, AMENDED OR CHANGED EITHER MANUALLY OR ELECTRONICALLY BY ANY THIRD PARTY. NOTE THIS IS AN UNCONTROLLED DOCUMENT ISSUED FOR INFORMATION PURPOSES ONLY. UNLESS SIGNED. FIGURED DIMENSIONS TAKE PRECEDENCE OVER SCALED. DO NOT SCALE REDUCED SIZE DRAWINGS. VERIFY DIMENSIONS PRIOR TO COMMENCING ANY ON-SITE OR OFF-SITE WORKS OR FABRICATION. IF IN DOUBT - ASK.	ARCHITECT 0 SCALE 1:	1 50	2	3	4	5m	DRAWING TITLE ROADWORKS

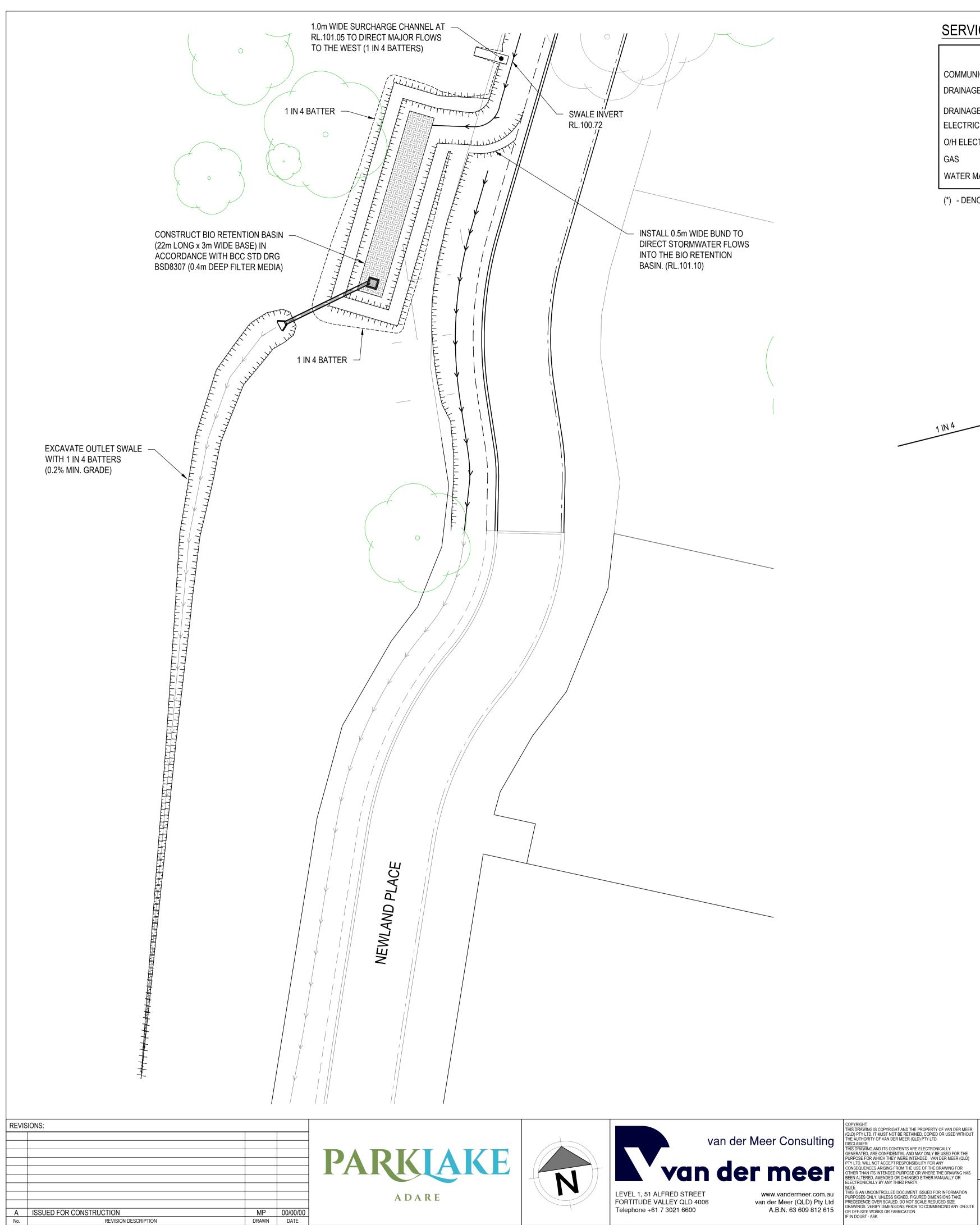
D SUBDIVISION , ADARE, QLD 4343	DRAWING STATUS ORIGINAL ISSUE FOR APPROVAL				
	PROJECT LEADER	designer MP	SIGNATURE		
S DETAILS	DRAFTSPERSON	SCALE AS SHOWN	DATE NOV 2022	SHEET SIZE	
	JOB No. BR222025		DRAWING No. C2490		



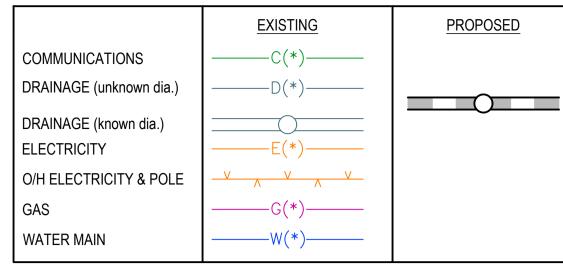


EXISTING		PROPOSED	
// FE	INCE	$\rightarrow \rightarrow \rightarrow \rightarrow$	OPEN DRAIN
OF	PEN DRAIN		KERB AND CHANNEL TYPE B1
———— КЕ	ERB AND CHANNEL		MOUNTABLE KERB TYPE M4
———————— КЕ	ERB ONLY		MOUNTABLE KERB TYPE M6
CC	ONTOUR (0.100m)		ROAD EDGE BITUMEN
RC	DAD EDGE BITUMEN		ROAD SHOULDER
AT 🕤	REE TO REMAIN		DRAINAGE
AT O	REE TO BE REMOVED		
1			

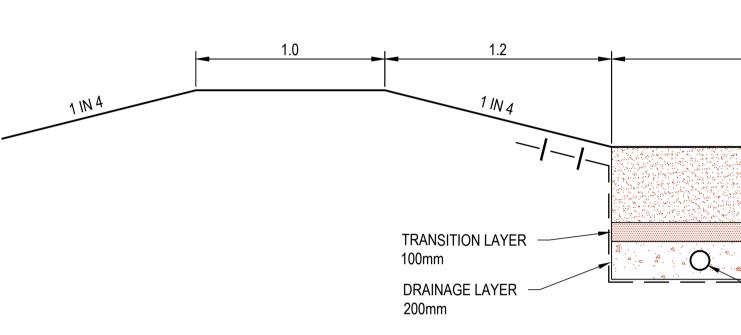








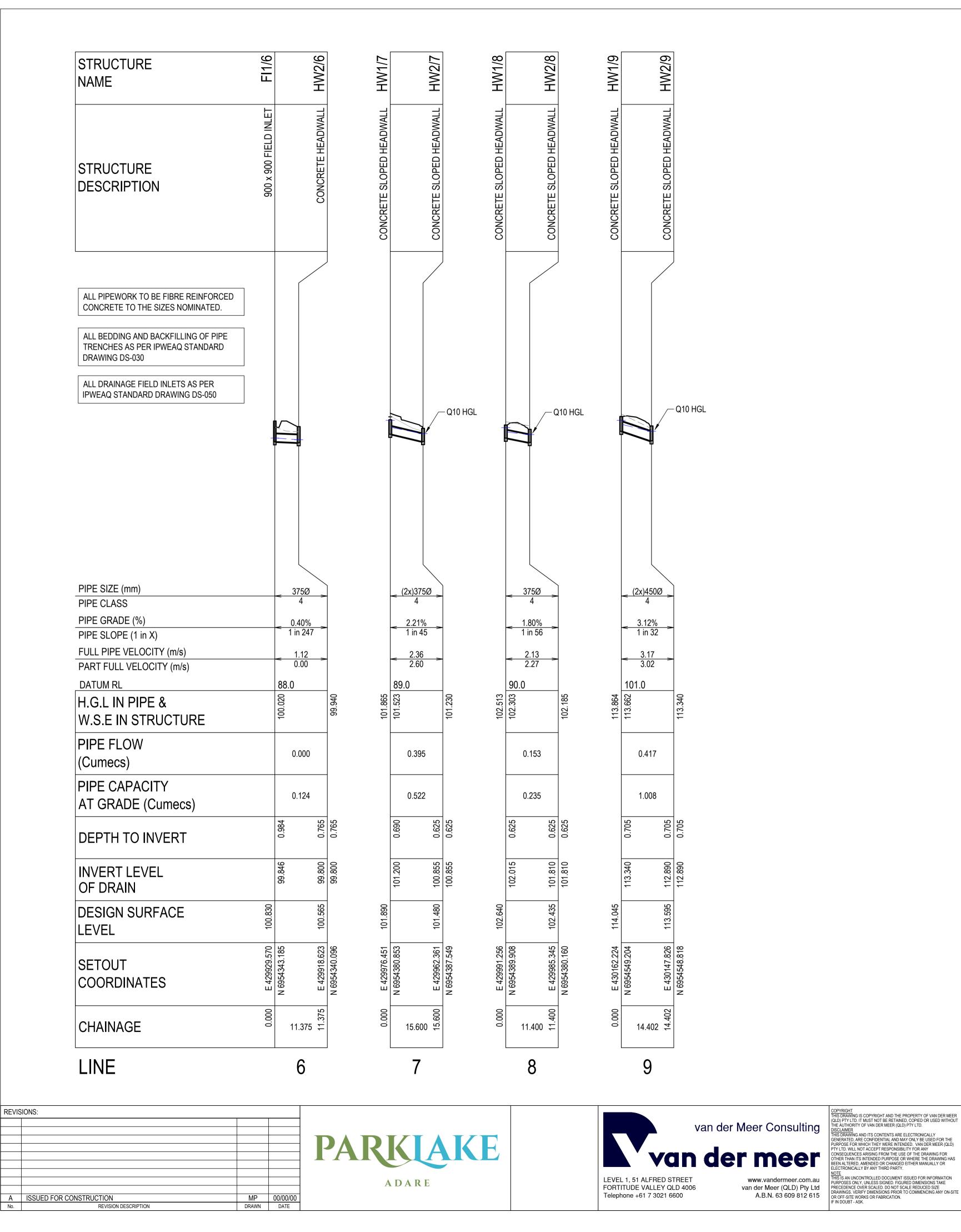
(\*) - DENOTES QUALITY LEVEL AS PER A.S. 5488-2013.



TYPICAL BIORE SCALE 1:20

PARK LAKE A PO BOX 4107 SPRINGF	PROJECT TITLE <b>PROPOSED S</b> 174 ADARE ROAD, ADA STAGE 2			
ARCHITECT 0 1 2 3 4 5 SCALE 1:250	10m	15	20m	DRAINAGE BIO F BASIN DETAILS

EXISTING		PROPO	SED			
// //	FENCE	<b> </b> →	$\rightarrow \rightarrow \rightarrow$	OPEN DF	RAIN	
$\rightarrow \rightarrow \rightarrow$	OPEN DRAIN			KERB AN	ID CHANNEL T	YPE B1
	KERB AND CHANNEL		<u> </u>	MOUNTA	ABLE KERB TYP	PE M4
	KERB ONLY				BLE KERB TYP	PE M6
	<ul> <li>CONTOUR (0.100m)</li> <li>ROAD EDGE BITUMEN</li> </ul>	***				
$\left( \begin{array}{c} \\ \end{array} \right)$	TREE TO REMAIN			DRAINA(	HOULDER	
$(\circ)$	TREE TO BE REMOVE	D				
3.0		1.2				
4		1 111	1			
300	RL.100.60	1 111				
•	F	- - -				
400						
			ים דאו ואד			22m
		LON	IG x 3m WID	E BASE) I	TION SWALE (2 N ACCORDAN( 20207	CE
	2 00mm DIA. SLOTTED PIPE		H BCC STD	DKG BSD	0307	
RETENTION E	BASIN SECTION	J				
		<u> </u>				
		<u> </u>				
		<u> </u>				
		<u>×</u>				
		<u>v</u>				
		<u>v</u>				
		<u>v</u>				
		<u>v</u>				
		0 .1 .2 .3 .4	.5	1m	1.5	2
			.5	<u>1m</u>	1.5	2
		0 .1 .2 .3 .4			1.5 15 20	
		0 .1 .2 .3 .4 SCALE 1:20				
		0 .1 .2 .3 .4 SCALE 1:20 0 1 2 3 4 5	10r	m		
DSUBDIVI	SION	0 .1 .2 .3 .4 SCALE 1:20 0 1 2 3 4 5 SCALE 1:250	<sup>10r</sup> ORIG	m	15 20 . <b>ISSUE</b>	
DSUBDIVI	SION	0 .1 .2 .3 .4 SCALE 1:20 0 1 2 3 4 5 SCALE 1:250	<sup>10r</sup> ORIG	n INAL PR APPR	15 20 . <b>ISSUE</b>	
D SUBDIVI D, ADARE, QLD 434	SION	0 .1 .2 .3 .4 SCALE 1:20 0 1 2 3 4 5 SCALE 1:250 DRAWING STATUS PROJECT LEADER	10r ORIG FO DESIGNER	n INAL IR APPR	15 20 ISSUE OVAL SNATURE	



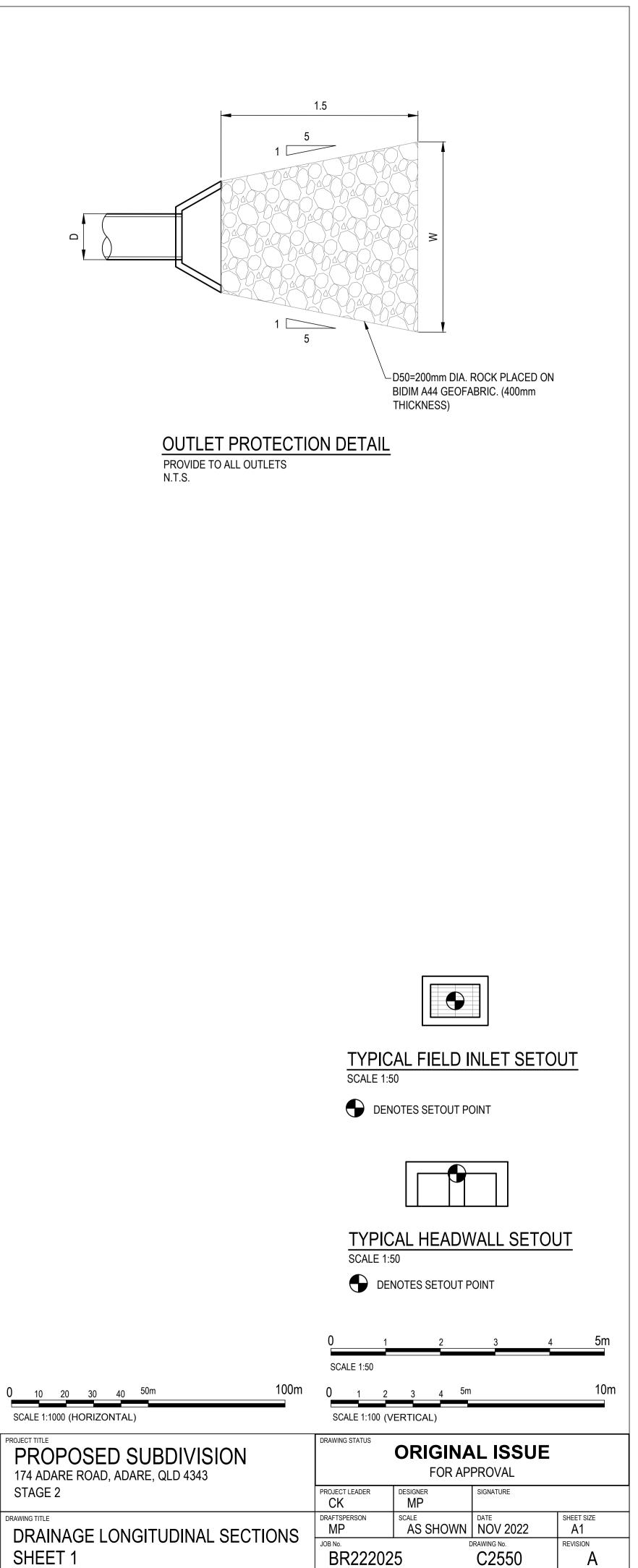
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PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300

ARCHITECT

STAGE 2 DRAWING TITLE SHEET 1

PROJECT TITLE



	LOCA	ATION			SU	UB-CATCHN	MENT RUI	NOFF										11	NLET DESIGN											DRAIN DESI	GN								HEAD LOSS	S				PART FULL				DESIGN LEV	VELS		
				Тс	1		А	CA	Qc	Q	a												Qg	Qb	Т	c I	CA	Qrat	Q	L	S	Vf=Q	/A Qcap	Vcap	Vt		Vf²/2g	Ku	hu	Kw	hw	Sf	hf	dn Vr	l l						
STRUCTURE No.	CATCHMENT CONTRIBUTING		DRAIN SECTION	SUB-CATCHMENT TIME OF CONC.	RAINFALL INTENSITY	CO-EFFICIENT OF RUNOFF	SUB-CATCHMENT AREA	EQUIVALENT AREA	SUB-CATCHMENT DISCHARGE	FLOW IN K&C (INC.	BYPASS)	HALF ROAD CAPACITY	FLOW WIDTH	FLOW DEPTH	FLOW DxV	ROAD GRADE AT INLET	ROAD XFALL AT INLET		INLET TYPE		BLOCKAGE FACTOR	INLET CURVE	FLOW INTO INLET	BYPASS FLOW	BYPASS STRUCTURE No. CRITICAL TIME OF	CONC. RAINFALL INTENSITY	TOTAL (C × A)	PEAK FLOW	PIPE FLOW	REACH LENGTH	PIPE GRADE	PIPE SIZE FULL PIPE VELOCITY	CAPACITY FLOW	CAPACITY VELOCITY	TRAVEL VELOCITY	CHART(S) USED	VELOCITY HEAD	U/S HEAD LOSS COEFFICIENT	U/S HEAD LOSS	W.S.E COEFFICIENT	CHANGE IN W.S.E	PIPE FRICTION SLOPE	PIPE FRICTION HEAD LOSS	NORMAL DEPTH NORMAL DEPTH VEL.	PIPE U/S I.L	PIPE D/S I.L	PIPE U/S H.G.L	PIPE D/S H.G.L	W.S.E	SURFACE LEVEL FREEBOARD	STRUCTURE No.
				min	mm/h		ha	ha	L/s	L,	/s l	L/s	m	m	m^2/s	%	%						L/s	L/s	m	in mm/hr	ha	L/s	L/s	m	% n	nm m/s	s L/s	m/s	m/s		m		m		m	%	m	m m/	s m	m	m	m	m	m	
FI1/6	FI1/6	FI 1/6 1	to HW2/6																900 x 900 FIELD INLE	ET	0.5				5	182				11.3746 0.4	0441 3	75	111.545	4 1.009949	2	Inlet Control					0	.40441	0.046		99.846	99.8	99.846	99.8 9	99.846 1	100.83 0.984	FI1/6
HW2/6	HW2/6																		CONCRETE HEADWA	LL																													99.8 10	00.565	HW2/6
HW1/7	HW1/7	HW1/7	7 to HW2/7	14.96441	1 125.1637	0.6	1.16113	31 0.6966	579 242.219	91 242.	2191						-0.42363		CONCRETE HEADWA	LL	0.8	2	42.2191		14.9	5441 125.163	7 0.696679	395.2191	395.2191 1	.5.60001 2.53	32049 (2x	375 1.789	186 558.222	1 2.527113	2	Inlet Control	0.16332	6 2.091178	0.341543	С	.341543 2.	197557 0.	.374294 0.2	33014 2.739	944 101.25	100.855	101.5728	101.23 10	)1.9144 1	0.0256	37 HW1/7
HW2/7	HW2/7																		CONCRETE HEADWA	LL																												1	.01.23 1	101.48	HW2/7
HW1/8	HW1/8	HW1/8	8 to HW2/8	13.66939	9 131.1208	0.6	0.69958	82 0.4197	49 152.882	29 152.	8829						4		CONCRETE HEADWA	LL	0.8	1	52.8829		13.6	5939 131.120	8 0.419749	152.8829	152.8829 1	1.39987 1.79	98267 3	75 1.384	225 235.216	6 2.129686	2	Inlet Control	0.09775	9 2.147085	0.209897	С	0.209897 1	.03591 0.	.164186 0.2	20283 2.266	471 102.015	101.81	102.3031 1	102.185 10	02.513 1	102.64 0.1270	11 HW1/8
HW2/8	HW2/8																		CONCRETE HEADWA	LL																												10	02.185 10	02.435	HW2/8
HW1/9	HW1/9	HW1/9	9 to HW2/9	11.79067	7 139.7629	0.6	1.7916	57 1.0750	002 417.348	83 417.	3483						20.17928		CONCRETE HEADWA	LL	0.8	4	17.3483		11.7	9067 139.762	9 1.075002	417.3483	417.3483 1	4.40237 3.12	24486 (2x	)450 1.312	1008.34	7 3.170044	2	Inlet Control	0.08783	2 2.301312	0.202128	C	.202128 2	.23517 0.	.383977 0.2	01822 3.019	444 113.34	112.89	113.6619	113.34 11	13.864 11	14.045 0.0809	54 HW1/9
HW2/9	HW2/9																		CONCRETE HEADWA	LL																												1	13.34 11	13.595	HW2/9

REVIS	IONS:			
				PAR
Α	ISSUED FOR CONSTRUCTION	MP	00/00/00	
No.	REVISION DESCRIPTION	DRAWN	DATE	





## STORMWATER CALCULATIONS TABLE (Q10)



PARK LAKE ADARE PTY LTD PO BOX 4107 SPRINGFIELD QLD 4300

PROJECT TITLE PROPOSED 174 ADARE ROAD, A STAGE 2

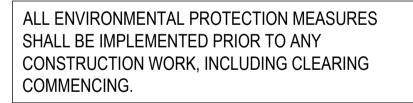
CLIENT

ARCHITECT

D SUBDIVISION ADARE, QLD 4343	DRAWING STATUS	ORIGINA FOR AP	AL ISSUE	
	PROJECT LEADER	designer MP	SIGNATURE	
ALCULATIONS	draftsperson	SCALE AS SHOWN	DATE NOV 2022	SHEET SIZE
	JOB NO. BR22202		DRAWING No. C2560	

	ASSET	REGISTER -	WATER	RETI	CULATION								
ESTATE/	STAGE	STAGE 2											
SITE ADI	DRESS	174 ADAF	RE ROAL	D, AD	ARE								
FILE/APF	PLICATIO	N											
DELEGA	TES APP	. DATE											
CLIENT		PAR	K LAKE /	ADAR	E PTY LTD								
DRAWIN	G/PLAN I	No.											
MAINS	DIA.	MATE DESIGN		T.	LEN DESIGN	IGTH CONST.							
	DN125	PE			575m								
	DN180	PE			406m								
SERVICES	DIA.	MATE DESIGN	RIAL CONS	T.	LEN DESIGN	IGTH CONST.							
	20mm												
	25mm												
	32mm	PE100 PN16			6m								
	40mm	PE100 PN16			19m								
METERS	DIA.	NUM	BER										
	20mm	20											
	25mm												
	32mm												
	150mm												
	50mm												

[			SERVICE DETAILS
	NO	SIZE	LOT NUMBERS
	16	DN25 PE	82-88, 101-107, 133 AND 134
	4	DN32 PE	112-115

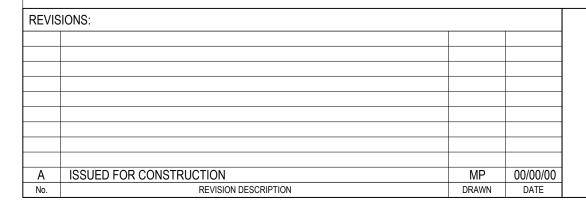


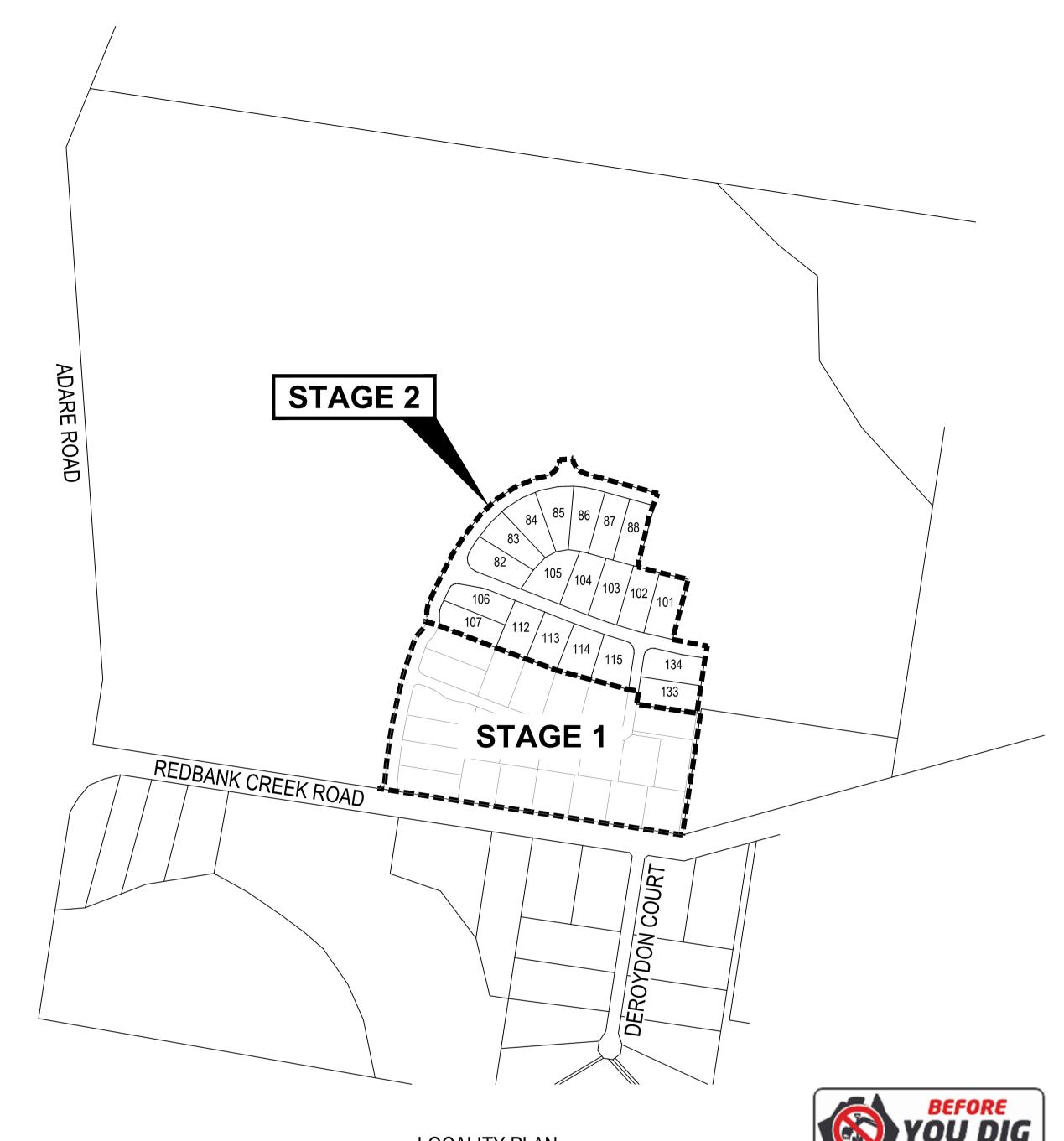
ALL WATER AND SEWER CONSTRUCTION WORK SHALL COMPLY WITH THE REQUIREMENTS OF THE QUEENSLAND WORKPLACE HEALTH AND SAFETY ACT 2011.

CONTACT THE DIVISION OF WORKPLACE HEALTH AND SAFETY FOR INFORMATION PHONE : 1300 362 128

SEQ CODE STANDARD DRAWING SCHEDULE

SOIL CLASSIFICATION	SEQ-WAT-1200-1
EMBEDMENT AND TRENCH FILL	SEQ-WAT-1200-2
THRUST BLOCK DETAILS	SEQ-WAT-1205-1
VALVE THRUST BLOCKS	SEQ-WAT-1206-1
IDENTIFICATION MARKERS	SEQ-WAT-1300-1,2





DN125 PE100 PN16

EXISTING DN125 PE

REMOVE END CAP AND INSTALL DN125 APPROVED CONNECTOR

LIVE CONNECTION DETAIL '2'

DIAGRAMMATIC

DN180 PE100 PN16 EXISTING DN180 PE

> REMOVE END CAP AND INSTALL DN180 APPROVED CONNECTOR

LIVE CONNECTION DETAIL '1' DIAGRAMMATIC

**ADARE** 



## **GENERAL NOTES**

- g NO.SEQ-WAT-1410-1.

## WATER MAIN CONSTRUCTION NOTES

- 3
- UTILITIES. 4
- 5

## **VEGETATION PROTECTION**

2.

4

- FURTHER ADVICE.

## SOIL

- 3

## REHABILITATION

- 2

- 6.
- ALL LIVE WORKS ON AC MAIN SHALL INCLUDE REMOVAL OF EXISTING AC MAIN FROM COLLAR

www.byda.com.au

TO COLLAR.

### VALVE MARKERS, HYDRANT M MARKINGS ARE TO BE INSTALL WITH SEQ-WAT-1300-1

## LIVE WORKS CONNECTION 1

STREET :	NEWLAND PLACE
INSTALLATION :	
TYPE OF MAIN :	EXISTING DN180 PE
-	
DATE COMMENCED :	DATE COMPLETED :
-	

SIGNATURE : \_\_\_\_\_

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WILL NOT ACCEPT RESPONSIBILITY FOR ANY DUENCES ARISING FROM THE USE OF THE DRAWING FOR THAN ITS INTENDED PURPOSE OR WHERE THE DRAWING HAS TERED, AMENDED OR CHANGED EITHER MANUALLY OR							STAGE 2	PROJECT LEADER	designer MP	SIGNATURE	
ONICALLY BY ANY THIRD PARTY. AN UNCONTROLLED DOCUMENT ISSUED FOR INFORMATION SES ONLY, UNLESS SIGNED. FIGURED DIMENSIONS TAKE		100	200	300	400	500m	DRAWING TITLE WATER RETICULATION COVER SHEET	DRAFTSPERSON	AS SHOWN	DATE NOV 2022	SHEET SIZE
JENCE OVER SCALED. DO NOT SCALE REDUCED SIZE IGS. VERIFY DIMENSIONS PRIOR TO COMMENCING ANY ON-SITE -SITE WORKS OR FABRICATION. UBT - ASK.	SCALE 1:50	000						JOB NO. BR22202		drawing №.	

LOCALITY PLAN SCALE 1:5000



IS DRAWING AND ITS CONTENTS ARE ELECTRONIC, NERATED, ARE CONFIDENTIAL AND MAY ONLY BE L RPOSE FOR WHICH THEY WERE INTENDED. VAN DI

LTD. WILL NOT ACCEPT RESPONSIBILITY FOR AN

1. ALL WORK AND MATERIALS SHALL BE IN ACCORDANCE WITH CURRENT SOUTH EAST QUEENSLAND WATER SUPPLY CODE SPECIFICATIONS AND STANDARDS.

2. UNLESS SPECIFIED OTHERWISE ALL MATERIALS AND WORK SHALL COMPLY WITH THE RELEVANT AUSTRALIAN STANDARDS.

3. ADOPT LIP OF KERB OR SHOULDER OF ROAD AS PERMANENT LEVEL.

COVER ON MAINS FROM PERMANENT LEVEL TO BE AS SHOWN IN SEQ-WAT-1200-2.

CONDUITS TO BE INSTALLED IN ACCORDANCE WITH THE STANDARD DRAWINGS. A WATER METER SUPPLIED AT THE DEVELOPER'S COST, IS TO BE INSTALLED AT THE SERVICE POINT OF EACH LOT IN ACCORDANCE WITH THE STANDARD DRAWING FOR THE SEQ-SP.

7. ALL MATERIALS USED IN THE WORKS SHALL COMPLY WITH THE SEQ-SP'S ACCEPTED PRODUCTS AND

MATERIALS LIST OR BE APPROPRIATELY SHOWN. LISTED AND DEFINED IN THE ENGINEERING SUBMISSION SO THAT THE ALTERNATIVE PRODUCT OR MATERIAL CAN BE ASSESSED AND IF APPROPRIATE, APPROVED BY THE SEQ-SP.

8. ALL CONCRETE FOOTPATHS TO BE CLEAR OF WATER MAINS.

TEST/CHLORINATION POINTS TO BE INSTALLED IN ACCORDANCE WITH STANDARD DRAWING

10. THE CONSTRUCTION OF THE WATER RETICULATION WORK SHOWN ON THIS DRAWING MUST BE SUPERVISED BY AN ENGINEER WHO HAS RPEQ REGISTRATION. WORKS NOT COMPLYING WITH THIS REQUIREMENT WILL NOT BE PERMITTED TO CONNECT TO THE RETICULATION SYSTEM.

11. WATER MAIN ROAD CROSSING AND VALVE PAVEMENT MARKERS TO BE INSTALLED AS PER SEQ-WAT-1300-1.

1. DURING ANY CONSTRUCTION ACTIVITY AT LEAST ONE PERSON ON SITE MUST HAVE COMPLETED A PIPE LAYING TRAINING COURSE APPROVED BY THE PIPE SUPPLIER OR MANUFACTURER AND APPROPRIATE TO THE PIPELINE UNDER CONSTRUCTION. THE TRAINING COURSE MUST HAVE BEEN COMPLETED WITHIN THE LAST TEN YEARS.

ALL SITE AND FACTORY PE WELDING SHALL BE CARRIES OUT BY A PERSON WHO HAS COMPLETED RELEVANT NATIONALLY ACCREDITED TRAINING COURSES FOR BUTT WELDING/ELECTROFUSION AND HOLD A VALID WELDING CERTIFICATE IN ACCORDANCE WITH AS/NZS 2033.

THE CONTRACTOR SHALL PROVIDE DOCUMENTED EVIDENCE OF ACCEPTABLE QUALIFICATIONS TO URBAN

INSTALL DETECTABLE MARKER TAPE ON ALL WATER MAINS AND PROPERTY SERVICES.

CONSTRUCT FIRE HYDRANTS AND STOP VALVES TO SEQ-WAT-1301-1, 1302-1, 1303-2, 1305-1, 1306-1 & 1409-1. ALL LIVE WORKS SHALL BE UNDERTAKEN BY THE DEVELOPER'S LICENSED CONTRACTOR IN ACCORDANCE WITH A VALID URBAN UTILITIES NETWORK ACCESS PERMIT, UNDER SUPERVISION OF URBAN UTILITIES, AT THE DEVELOPER'S EXPENSE.

7. PROPOSED WORKS ARE LOCATED WITHIN FIRE ANT BIOSECURITY ZONE 2. ALL WORKS ARE TO BE TO DAFF REQUIREMENTS.

1. TREES LOCATED ALONG THE FOOTPATH SHALL BE, TRANSPLANTED PRIOR TO CONSTRUCTION, OR REPLACED IF DESTROYED.

WHEN WORKING WITHIN 4m OF TREES, RUBBER OR HARDWOOD GIRDLES SHALL BE CONSTRUCTED WITH 1.8m BATTENS CLOSELY SPACED AND ARRANGED VERTICALLY FROM GROUND LEVEL. GIRDLES SHALL BE STRAPPED TO TREES PRIOR TO CONSTRUCTION AND REMAIN UNTIL COMPLETION.

3. TREE ROOTS SHALL BE TUNNELLED UNDER, RATHER THAN SEVERED. IF ROOTS ARE SEVERED THE DAMAGED AREA SHALL BE TREATED WITH A SUITABLE FUNGICIDE. CONTACT RELEVANT COUNCIL ARBORIST FOR

ANY TREE LOPPING REQUIRED SHOULD BE UNDERTAKEN BY AN APPROVED ARBORIST.

TOPSOIL AND SUBSOIL SHALL BE STOCKPILED SEPARATELY.

CARE SHALL BE TAKEN TO PREVENT SEDIMENT FROM ENTERING THE STORMWATER SYSTEM. THIS MAY

INVOLVE PLACING APPROPRIATE SEDIMENT CONTROLS AROUND STOCKPILES. IF ACID SULPHATE SOILS EXIST IN THE WORKS AREA THE OUTPUTS FROM THE RISK ASSESSMENT BASED ON THE QUEENSLAND ACID SULPHATE SOIL TECHNICAL MANUAL SHALL BE ADHERED TO.

PRE-DISTURBANCE SOIL PROFILES AND COMPACTION LEVELS SHALL BE REINSTATED.

PRE-DISTURBANCE VEGETATION PATTERNS SHALL BE RESTORED.

ALL DISTURBED AREAS ASSOCIATED WITH CONSTRUCTION SHALL BE REHABILITATED. HEAVILY COMPACTED AREAS SHOULD BE RIPPED PRIOR TO TREATMENT

ALL DISTURBED AREAS ARE TO BE LEFT IN STABLE CONDITION.

ALL PLANTING/RE-VEGETATION WILL NEED TO BE MAINTAINED THROUGHOUT THE MAINTENANCE PERIOD. TRENCH REINSTATEMENT IS TO BE UNDERTAKEN IN ACCORDANCE WITH SEQ STANDARD REQUIREMENTS FOR EMBEDMENT AND TRENCH FILL. STANDARD DRAWINGS HAVE BEEN NOTED ON THIS PLAN.

		AS CONSTRU	CTED	
ARKERS AND PAVEMENT .ED/REINSTATED IN ACCORE	DANCE	ACCORDANCE V	ED DOCUMENTATION IS TO BE PROVIDED IN VITH REQUIREMENTS AS SPECIFIED IN THE SEQ USIVE OF FULL ADAC COMPLAINT SURVEY	
	LIVE W	ORKS CONN	IECTION 2	
<u>E</u>	STREET :		AXFORD WAY	
	INSTALLA	TION :		
PE	TYPE OF	MAIN :	EXISTING DN125 PE	
:	DATE COMMEN	ICED :	DATE COMPLETED :	
	SIGNATU	RE :		
	L			
		DRAWING STATUS		

